

GEOLOGICAL SURVEY
ENGINEERING DIVISION

NATURAL GAS PIPELINE, NOOMBA-ADELAIDE
PORT RIVER CROSSING, TAPEHO
GEOLOGICAL INVESTIGATIONS - PROGRESS REPORT NO. 1

DESIGN STAGE

Hundred of Pt. Adelaide

Client: Bechtel Pacific Corporation

by

R.D. STEEL
ASSISTANT SENIOR GEOLOGIST
ENGINEERING GEOLOGY SECTION

SR11/5/238/3

25th April, 1968

NATURAL GAS PIPELINE, MOOMBA-ADELAIDE

FORT RIVER CROSSING, TAPEROD

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G.S. No. 3978
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Rept. Bk. No. 750
G.S. No. 3978
S.R. 11/5/838/5

NATURAL GAS PIPELINE, MOONBA-ADELAIDE

FORT RIVER CROSSING, TAPEROG

GEOLOGICAL INVESTIGATIONS - PROGRESS REPORT NO. 1

DESIGN STAGE

Hundred of Ft. Adelaide

Client: Bechtel Pacific Corporation

INTRODUCTION

It is proposed that the natural gas pipeline from Moonba to Adelaide, will cross the Fort River at a site near Taperog. The crossing line is parallel to and 80ft. south of the E.T.S.A. 60 KV. transmission line (suspended from two 350ft. high towers) and will link the Meter and Regulating Stations at Terrens Island on the eastern side and Taperog on the western side. The 12 in. I.D. steel fabricated pipe will be buried up to 18ft. in sediments beneath the existing river bottom, but future widening and deepening of the shipping channel will decrease this depth to an average of 12ft. Minimum depth of cover ranges between 3 and 5ft. (Fig. 11)

A request for the Department to investigate the crossing area by cable-tool drilling was received in a letter dated 19th December, 1967, from Mr. J.W. Lair, Contracts Manager, Bechtel Pacific Corporation.

Five holes totalling 180ft. have been drilled, four from a pontoon in the river and the fifth on the western river bank (Fig. 11)

Geological work has been confined to the logging of samples from the drillholes (Figs. 6 to 10) and an examination of materials exposed on the western river bank.

The soil horizons have been logged geologically and also classified using the Unified Classification System (Ref. 1) Notes on cable-tool drilling procedures are given in Appendix A and descriptive terms used in the classification of samples are defined in Appendix B.

A continuous tube sample was taken at the top of each of holes CH2, CH4 and CH5. The tubes, 4in. diam. and up to 1ft. long, were sealed and delivered with minimum disturbance to the E. & W.S. Dept. Soils Section laboratories, Wakefield Street. Results of tests carried out on these samples will be presented in a separate report from the E. & W.S. Dept.

OUTLINE OF REGIONAL GEOLOGY

The geology of the region is shown on the ADELAIDE 1 mile to 1 inch geological sheet (Ref. 2) and is described in greater detail by FIRMAN (Ref. 3)

The area lies near the eastern margin of the St. Vincent Graben. Here marine, gulf, estuarine and littoral sediments of Recent Age, overlie progressively Pleistocene alluvial-valley flat deposits and Tertiary sediments of the St. Vincent Basin.

The sequence is outlined in Table 1

TABLE 1

Age	Unit	Depositional Environment	Description
RECENT	SEMAPHORE SAND	Wind-blown	White quartz sand of the coastal dunes.
"	ST. KILDA FORMATION	Shallow sea and shoreline	Finely comminuted shell sand and silty clay, with plant debris, stranded shell banks and shell sand as beach ridges
"	LIPSON FORMATION	Shoreline	Clay, silt and fine sand, with abundant plant fibre and shell

PLEISTOCENE	GLANVILLE FORMATION	Shallow-sea	Sand and clay, with abundant shell - often has lime cemented crust.
"	HINDMARSH CLAY	Alluvial-Valley flat	Interbedded sands, clays and silts. Generally coarsely mottled. Lime pockets and nodules common.

SITE GEOLOGY

Topography

The site area is of generally low relief (Fig. 11, Section

A-D).

Tide levels in the river vary from R.L. 101.1ft. at Indian Springs Low Water to R.L. 103.9ft. at Mean High Water Spring. At these levels the river width is 1080ft. and 1680ft. respectively.

The existing dredged shipping channel is approximately 420ft. wide and 30ft. deep at low water and has a flat bottom. It is proposed in future to widen the channel to 720ft. and deepen it a further 6ft. (Fig. 11)

River bottom gradients are 1:20 (3°) west of the channel and vary between 1:30 (2°) to 1:80 (1°) approaching the mangrove swamps of Torrens Island to the east (Fig. 11)

The western bank has been raised to R.L. 118ft. by the addition of fill material, placed over a number of years. The fill includes cinders, charcoal, sand gravel and calcareous residues from the nearby Imperial Chemical Industries Plant. The mass has become weakly to firmly cemented by lime, giving rise to a surface crust and a steep artificial river bank approximately 9ft. high.

Soil and Rock Types

The five drillholes show wide variations in the depth and nature of materials present, making it difficult to correlate individual soil horizons between successive holes. Table 2 shows the geological sequence revealed by the drilling.

TABLE 2

DEPTH (R.L. in feet)										GEOLOGICAL AGE	UNIT	DESCRIPTION
CH1		CH2		CH3		CH4		CH5				
From	To	From	To	From	To	From	To	From	To			
118	105	-	-	-	-	-	-	-	-	(50 years)	FILL	SAND, gravel, charcoal, slag and I.C.I. residues.
105	81	86	74.5	-	-	95	91	94	79	RECENT	ST. KILDA FORMATION (shallow sea)	Mainly SAND, fine grained, shelly, light grey. Pockets of organic clay and silt near top. Contains patches of decomposing plant matter (strong smell of H ₂ S)
81	76	74.5	73.5	-	-	-	-	-	-	"	LIPSON FORMATION (Shoreline)	CLAY SOIL, high plasticity. Dark grey to blue grey. Contains up to 30% SAND, fine grained clayey, also numerous shell fragments and limestone lumps (Reworked Glanville Formation)
76	53	73.5	50	71.5	49	91	55	-	-	PLEISTOCENE	HINDMARSH CLAY (Alluvial-valley flat)	CLAY and SILT SOILS and SANDS, fine to coarse grained. Interbedded, possibly lenticular. Mottled grey, brown, yellow-brown and red-brown. Contains generally 10 to 20% GRAVEL (mainly lime nodules) and pockets of whitish earthy lime.

Groundwater

Below river level all sands are saturated and all clays and silts are generally at or greater than the plastic limit.

Water level beneath the western bank was measured at R.L. 106.5ft. in Hole CH1, but this is probably subject to tidal influence.

DISCUSSION

Details of the position and depth of burial of the pipeline taken from the drawing of Bechtel Pacific Corporation (Fig. 3.7 Job. No. 5699) are shown on Fig. 11. The nature of the sediments in which the pipeline would be founded over various parts of the crossing axis are shown below.

WESTERN BANK

FILL MATERIAL

RIVER SECTION	- West of Channel	- Sands and clays of ST. KILDA and LIPSON FORMATIONS.
"	"	- Shipping Channel
"	"	- East of Channel
		- Clays, silts and fine to coarse sands of HINDMARSH CLAY
		- Rising through HINDMARSH CLAY into shelly sands of ST. KILDA FORMATION.

The fill material on the western bank has been weakly to firmly cemented by lime from I.C.I. Alkali residues. This material is highly corrosive.

The fine grained shelly sands of the ST. KILDA FORMATION are mainly very loose and form a very weak surface layer 3 to 11ft. deep both east and west of the river channel. The cable tool sampling assembly and tube (total weight 600 lbs.) penetrated this material almost under its own weight. Values of N, where recorded, averaged only 2 to 3.

The clay soils of the LIPSON FORMATION are only present west of the shipping channel and attain a maximum thickness of 5ft. These were mainly firm to stiff at moisture contents generally higher than plastic limit.

In the river channel both ST. KILDA and LIPSON FORMATIONS are absent and the HINDMARSH CLAY is present at the river bottom. The

materials comprising the HINDMARSH CLAY range from high plasticity clay soils to coarse grained sands. The clay soils mainly proved stiff to very stiff (readings by Soiltest Penetrometer from 3 to more than 4.5) the silt soils mainly compact and the sands dense to medium dense.



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RDS:SMA:OB
25.4.68

REFERENCES

1. UNITED STATES DEPARTMENT OF THE INTERIOR, BUREAU OF RECLAMATION
1966, Earth Manual, 2nd Edition
2. SPRIGG, R.C., WHITTLE, A.W.G., 1951 Geological Atlas of South
Australia, Sheet ADELAIDE (1:63,360)
3. FIRMAN, J.B. - "Stratigraphic Units of Late Cainozoic Age in the
Adelaide Plains Basin, South Australia" Geol.Surv. S.Aust.
Quart. Geol. Notes. No. 17

APPENDIX A

LOGS OF CABLE-TOOL HOLES AND EXPLANATORY NOTES

APPENDIX A

LOGS OF CABLE-TOOL HOLES AND EXPLANATORY NOTES

NOTES ON DRILLING PROCEDURES

Equipment

The drilling is carried out with a Cable-tool drilling plant using "S" series sampling tools (Figs. 1 and 2)

Sampling Procedure

SA Samples

SA samples are obtained by driving an "S" tube, fitted with a Mark A shoe, into the material to be sampled. The number of blows per foot of penetration of the sampling tube are recorded by the driller. The sample, or core, is extruded from the sampling tube using an hydraulic ram. The extruded core is sealed in a plastic bag and stored in a core box, with a label showing the depth of the sample and the number of blows per foot of penetration. The hole is reamed out after each sample with a Mark B shoe.

SAL Samples

Sealed tube samples for laboratory testing (SAL) are taken at various intervals during drilling.

The drilling procedure is similar to that for obtaining SA samples, but in order to completely fill the sampling tube, it is driven 1.5ft. into the material to be sampled. On removal from the drill hole, both ends of the sampling tube are sealed with paraffin wax and screwed caps are fitted to the tube. The sealed tubes are labelled as in Fig. 3.

Standard Penetration Test

The standard Penetration Test (Ref. 1) is used to test the in situ density of sands and to give an indication of the consistency of clays and compactness of silts.

The equipment is illustrated in Fig. 4 and consists of a 2in. diameter sampling spoon (tube) and a hammer of standard weight (140lbs.)

With the equipment assembled as in Fig. 4 the hammer is allowed to fall on to the drill rods until the sampling shoe has penetrated 6 in. into the soil. The Standard Penetration Test is the number of blows (N) required to produce the next foot of penetration.

NOTES ON DRILL LOG SHEETS

The logs are plotted on a standard Cable-tool log form.

Near the centre of the form a graphic log of the materials encountered is shown.

In the column to the right of the graphic log, the soils are classified and described according to the Unified Soil Classification (Fig. 5)

To the left of the graphic log is a geological description of the materials sampled. This includes:-

.....Geological age)	
.....Soil unit name)	printed vertically
.....Type of material)	
.....Mineral composition		
.....Grain shape		
.....Cementation		
.....Organic materials		

Water levels are indicated by a small arrow with the date at which the observation was made.

In the blows per foot column, a continuous histogram is made of the number of blows required to drive the sampling tube through each foot of material. A hatching code is used to distinguish various types of sample. This code is reproduced at the bottom of each log sheet.

In the column on the far right of the log sheet, readings made with a Soiltest Penetrometer are plotted as a histogram. The Soiltest Penetrometer only gives true values of q_u (unconfined compressive strength) when used in clays in which $\phi = 0$

REFERENCE

1. TERZAGHI, K. and PECK, R.B. 1948. Soil Mechanics in Engineering Practice. John Wiley and Sons.
2. UNITED STATES DEPARTMENT OF THE INTERIOR, BUREAU OF RECLAMATION. 1966. Earth Manual, 2nd Edition.

APPENDIX B

DESCRIPTIVE TERMS

CLAY SOILS

CONSISTENCY

CONSISTENCY	SYMBOL	UNCONFINED COMPRESSIVE STRENGTH (kg/sq. cm)	FIELD TEST	N
Very soft	V.S.	less than 0.25	Easily penetrated several inches by fist.	2
Soft	S	0.25 to 0.5	Easily penetrated several inches by thumb	2 to 4
Firm	F	0.5 to 1.0	Can be penetrated several inches by thumb with moderate effort	4 to 8
Stiff	St	1.0 to 2.0	Readily indented by thumb but penetrated only with great effort	8 to 15
Very Stiff	V. St	2.0 to 4.0	Readily indented by thumb nail	15 to 30
Hard (Extremely stiff)	H	over 4.0	Indented with diffi- culty by thumb nail	30 and over

Based partly on Terzaghi, K. and Peck, R.B. 1966
Soil Mechanics in Engineering Practice,
Wiley - New York

MOISTURE CONTENT

Abbreviation	Meaning
MC \approx LL	Moisture Content near liquid limit
MC < LL	" " less than " "
MC > PL	" " greater than plastic limit
MC \approx PL	" " Near " "
MC \leq PL	" " less or equal to " "
MC < PL	" " less than " "
MC \ll PL	" " much less than " "

2. SILT SOILS

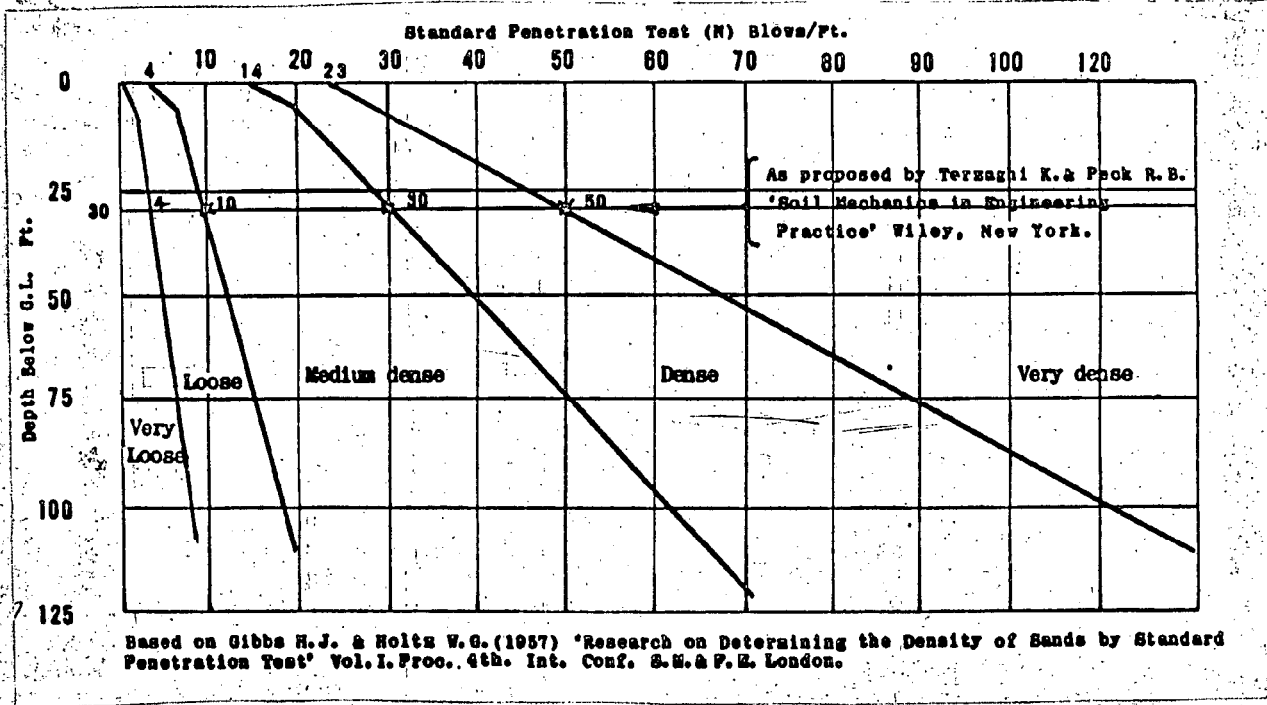
COMPACTNESS	SYMBOL	N
Loose	L	0 to 5
Moderately compact	MC	5 to 15
Compact	C	15 to 20
Very Compact	VC	greater than 20

3. SANDS

CLASSIFICATION OF SANDS BY STANDARD PENETRATION TEST

The relative density of granular soils has been judged from the results of Standard Penetration Tests carried out by the procedure described by Terzaghi and Peck (Ref. 1) bearing in mind the limitations of the method as discussed by Gibbs and Holtz (Ref. 2). At all times the water in the drill hole was kept at the level of surrounding groundwater.

EFFECT OF OVERBURDEN PRESSURE ON STANDARD PENETRATION TEST



REFERENCES

1. Terzaghi K, and Peck, R.B. 1948 "Soil Mechanics in Engineering Practice". Wiley, New York.
2. Gibbs, H.T. and Holtz W.G. 1957 Research on determining the density of sands by standard penetrations tests. Proc. 4th Int. Conf. SM & FE, London Vol.9

PROJECT **NATURAL GAS PIPELINE LOG OF CABLE TOOL HOLE****MOOMBA - ADELAIDE**

SECTION —

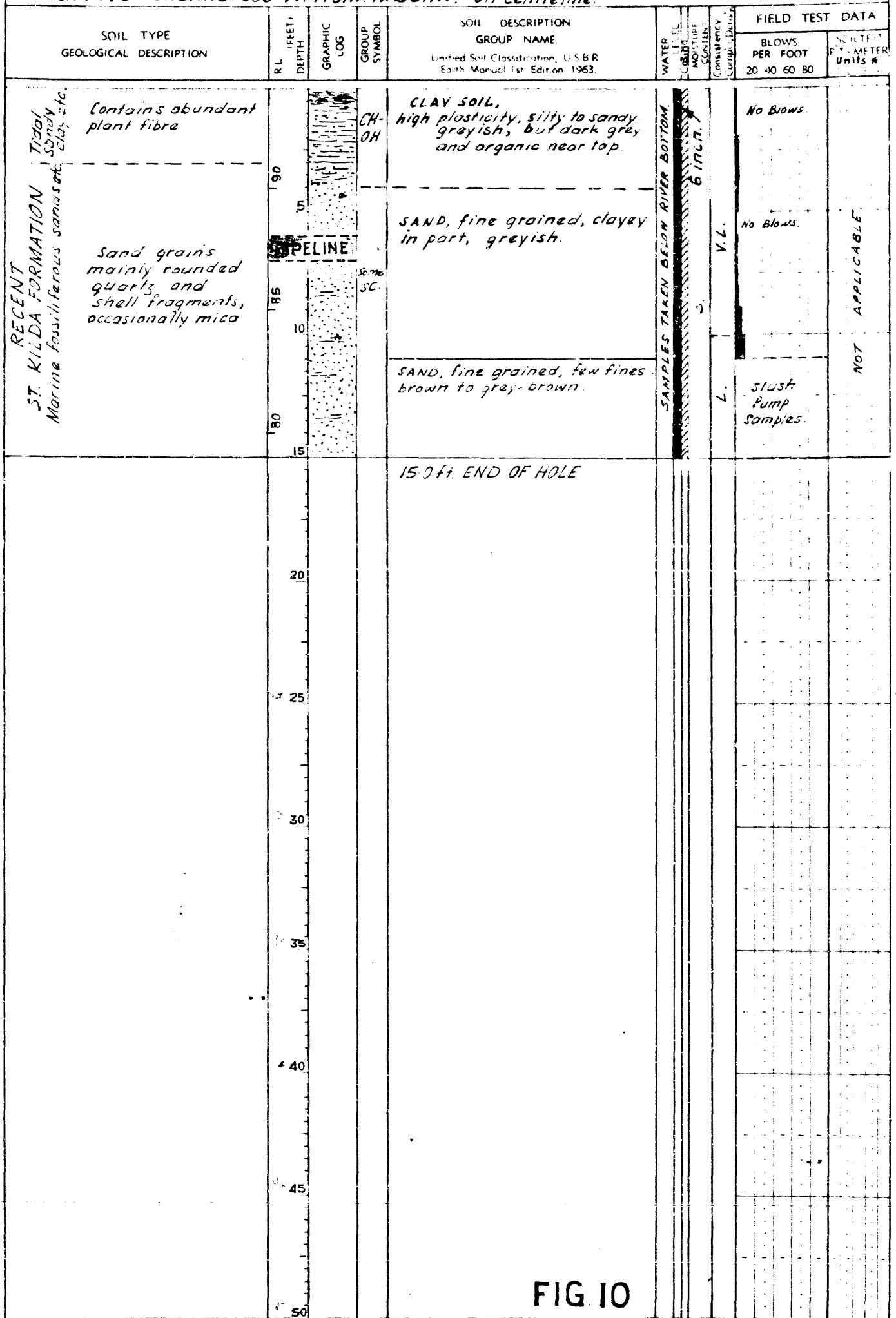
FEATURE **PORT RIVER CROSSING, TAPER00**HUNDRED **PTADELAIDE**LOCATION **River Channel 885 ft from W. Bank. On centreline.**HOLE NO **CH5**SERIAL No **386-68**R.L. River Bottom **94.0**R.L. Collar **111.0 FT**Datum L.W.D. **107.8 FT.**

FIG 10

TYPE OF SAMPLE	CONSISTENCY	COMPACTNESS	RELATIVE DENSITY	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA) D " (SD) E " (SE) G " (SG) Sealed Tube - A Shoe -SAL Standard Pene- tration Test-SPT	VS - Very Soft S - Soft SF - Soft to Firm F - Firm FSt - Firm to Stiff St - Stiff VSt - Very Stiff H - Hard	Ls - Loose MC - Moderately Compact C - Compact VC - Very Compact	VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense	H - Humid D - Damp M - Moist W - Wet S - Saturated LL - Liquid Limit PL - Plastic Limit	DRILL No 8 TYPE Ruston DRILLER D Phillips START 5 Mar 69 FINISH 5 Mar 68	LOGGED BY R.D. Steel DATE Mar 68 TRACED R.D.S. CHECKED M.R.P.B.

PROJECT **NATURAL GAS PIPELINE LOG OF CABLE TOOL HOLE**
MOOMBA-ADELAIDE

FEATURE **PORT RIVER CROSSING, TAPER 00**

SECTION —

HUNDRED PT ADELAIDE

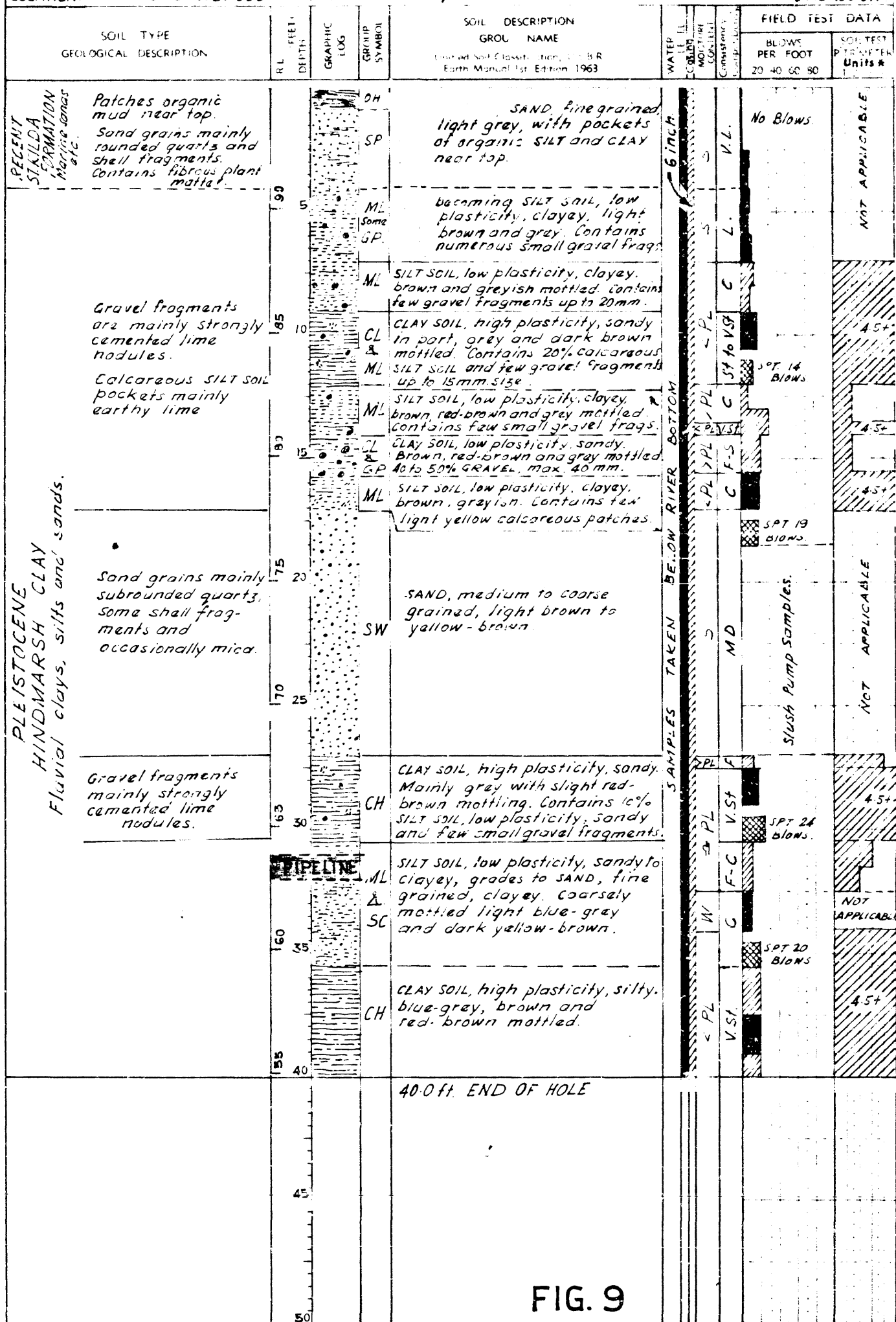
LOCATION: **River Channel 696 ft. from W. Bank, 10 ft. N of centreline**HOLE NO **CH4**SERIAL No. **389-68**RL River Bottom **94.8 FT**RL Corral **113.8 FT**Datum **L.W.L. = 100.8 ft**

FIG. 9

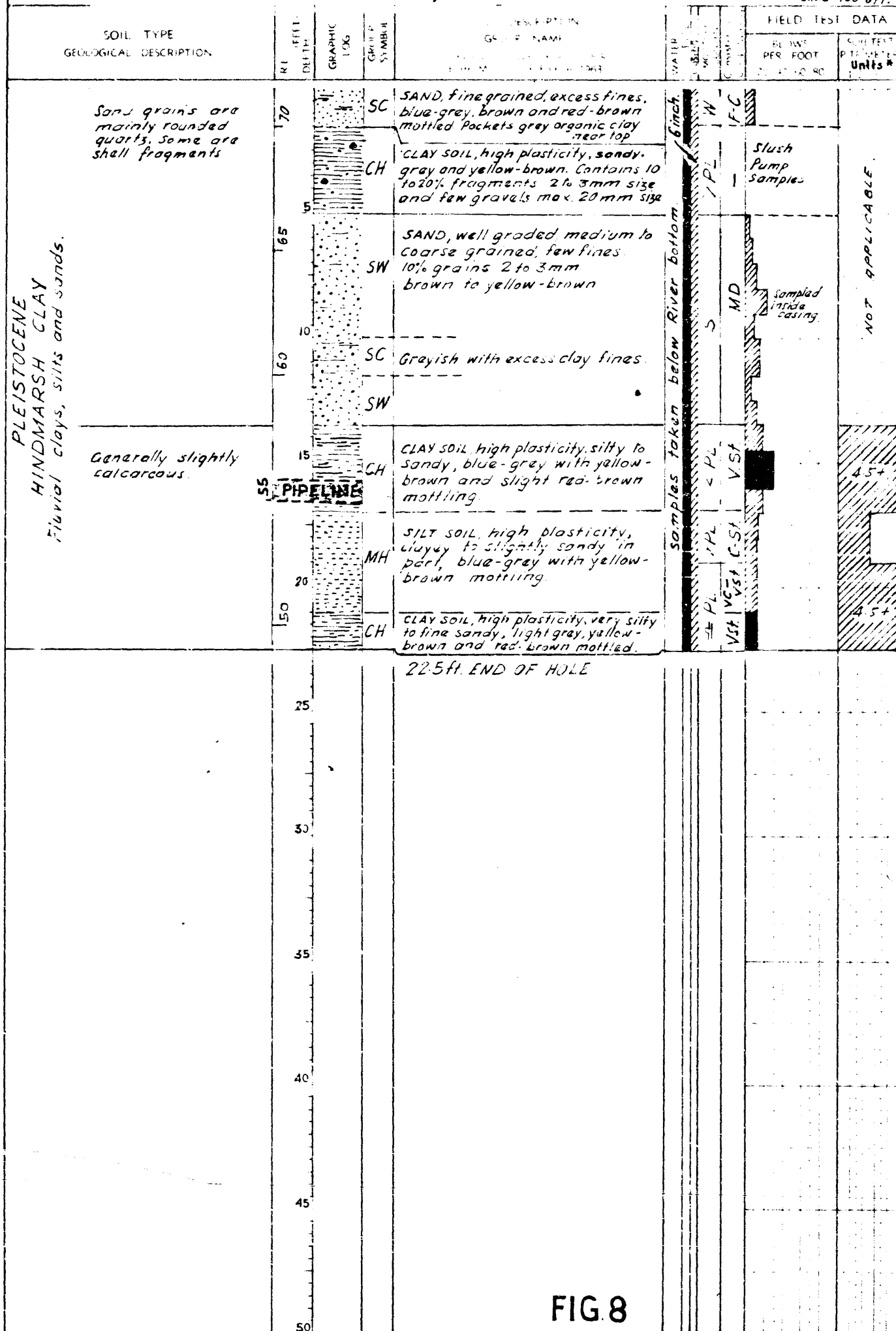
TYPE OF SAMPLE	CONSISTENCY	COMPACTNESS	RELATIVE DENSITY	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA) D " (SD) E " (SE) G " (SG) Sealed Tube - A Shoe - SA1 Standard Penetration Test - SPT	VS — Very Soft S — Soft SF — Soft to Firm F — Firm F St. — Firm to Stiff St — Stiff V St. — Very Stiff H — Hard	Ls — Loose MC — Moderately Compact C — Compact VC — Very Compact	VL — Very Loose L — Loose MD — Medium Dense D — Dense VD — Very Dense	H — Humid D — Damp M — Moist W — Wet S — Saturated LL — Liquid Limit PL — Plastic Limit	DRILL No. 8 TYPE Ruston DRILLER D. Phillips START 11 Mar. 68 FINISH 12 Mar. 68	LOGGED BY R. D. Steel DATE Mar. 68 TRACED R. D. S. CHECKED H. R. P. B.
					SHEET 1 OF 1	DRG No S 6488 Ho. 1.

PROJECT **NATURAL GAS PIPELINE**
MOOMBA - ADELAIDE

LOG OF CABLE TOOL HOLE

FEATURE **PORT RIVER CROSSING, TAPER 00**

SECTION —

HUNDRED **PT ADELAIDE**LOCATION **River Channel 414 ft. from W. Bank, 100 ft. N. of centreline**HOLE NO **CH3**SERIAL No **396-68**R.L. River Bottom: **71.5** FTR.L. Casing: **108.0** FTCasing LWD: **100.6** ft.

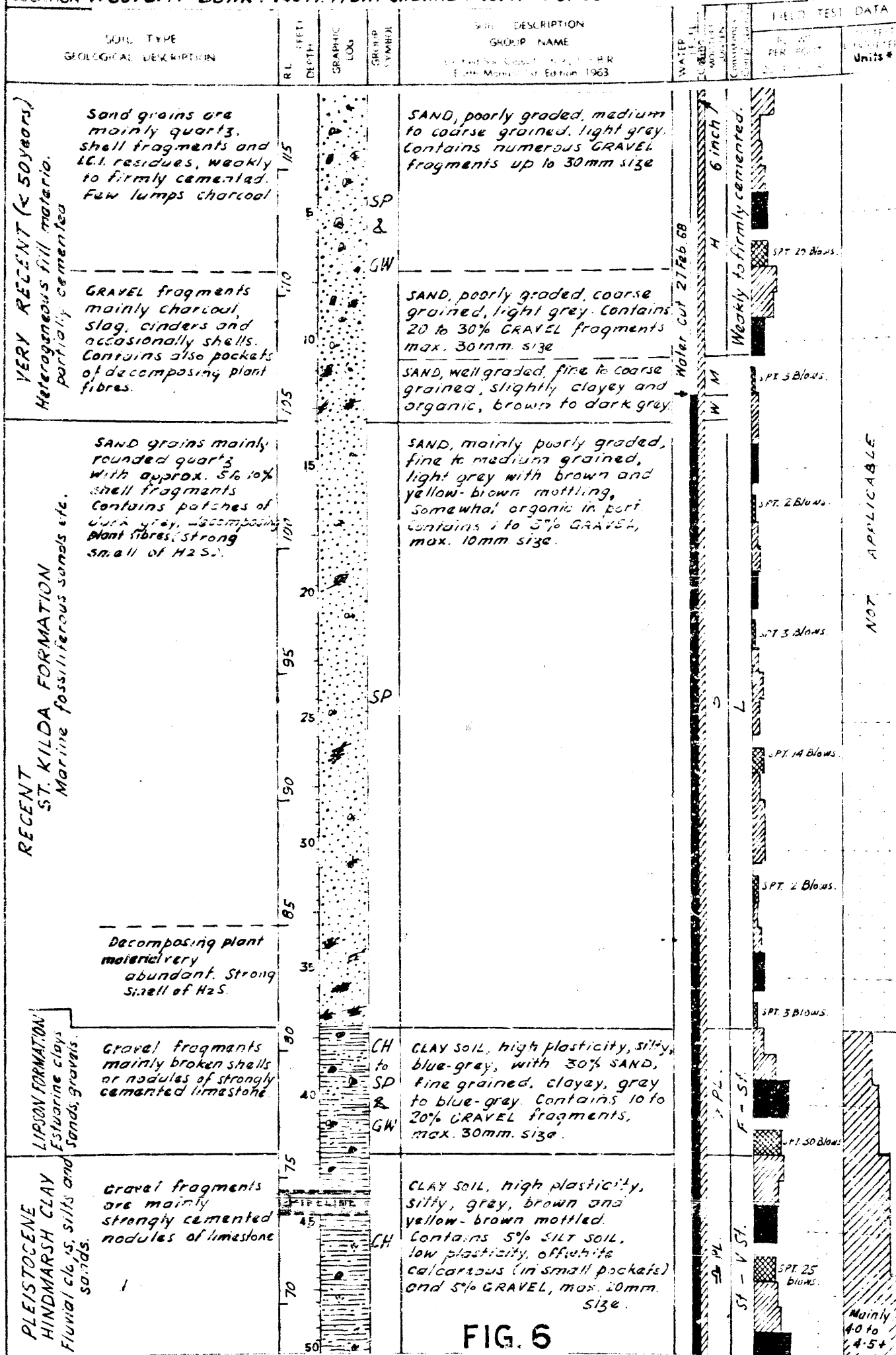
TYPE OF SAMPLE	CONSISTENCY	COMPACTNESS	RELATIVE DENSITY	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS — Very Soft	LS — Loose	VL — Very Loose	H — Humid	DRILL No 8	LOGGED BY
D " (SD)	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	TYPE Ruston	R.D. Steel
E " (SE)	SF — Soft to Firm	C — Compact	MD — Medium Dense	M — Moist	DRILLER D. Phillips	DATE Mar. 68
G " (SG)	F — Firm	VC — Very Compact	D — Dense	W — Wet	START 17 Mar. 68	TRACED R.D.S.
Sealed Tube — A Shoe — SAL	FSt — Firm to Stiff	VC — Very Compact	VD — Very Dense	S — Saturated	FINISH 17 Mar. 68	CHECKED W.R.P.B.
Standard Penetration Test — SPT	St — Stiff			— Liquid Limit	SHEET 1 OF 1	DRG No S6487 Ha 1.
	VSt — Very Stiff			— Plastic Limit		
	H — Hard					

HOLE NO **CHI**
385-03PROJECT **NATURAL GAS PIPELINE**

LOG OF CABLE TOOL HOLE

MOOMBA-ADELAIDESECTION **Harbors Board Block 3**FEATURE **PORT RIVER CROSSING, TAPEROD**HUNDRED **PT ADELAIDE**LOCATION **Western Bank, 140ft from channel, 160ft N. of centreline**REMARKS **118.0****114.5**

LWD: 100.9ft



NOT APPLICABLE

Mainly
40 to
4.5+

TYPE OF SAMPLE		CONSISTENCY	COMPACTNESS	RELATIVE DENSITY	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	<div><div>Dec. 36</div><div>Coating</div><div>Water level (date)</div><div>W.C.P.</div><div>Water cut</div></div>	VS — Very Soft	LS — Loose	VL — Very Loose	H — Humid	DRILL No 8	LOGGED BY
D " (SD)		S — Soft	MC — Moderately Compact	I — Loose	D — Comp	TYPE Ruston	R.D. Steel
E " (SE)		SF — Soft to Firm	F — Firm	MD — Medium Dense	M — Moist	DRILLER D. Phillips	DATE Mar 68
G " (SG)		FSt — Firm to Stiff	C — Compact	D — Dense	W — Wet	START 27 Feb. 68	TRACE R.D.S.
Sealed Tube - A Shoe - SAL		St — Stiff	VC — very Compact	VD — Very Dense	S — Saturated	FINISH 29 Feb. 68	CHECKED M.R.P.B.
Standard Penetration Test - SPT		VSt — Very Stiff	* These values refer to clay soils only and provide an indication of their consistency		LL — Liquid Limit	SHEET 1 OF 2	DRG No
		H — Hard			P — Plastic Limit		S6495 Hal

HOLE NO
SERIAL No 385,68

CHI

PROJECT **NATURAL GAS PIPELINE**
MOOMBA-ADELAIDE

LOG OF CABLE TOOL HOLE

SECTION Harbors Board Block 3

FEATURE **PORT RIVER CROSSING, TAPER00**HUNDRED **PT. ADELAIDE**LOCATION **Western Bank, 140ft. from channel. 160ft. N. of centraline.**

R.L. Surface 118.7

R.L. Center 114.5

Depth L.W.D. 100.8ft.

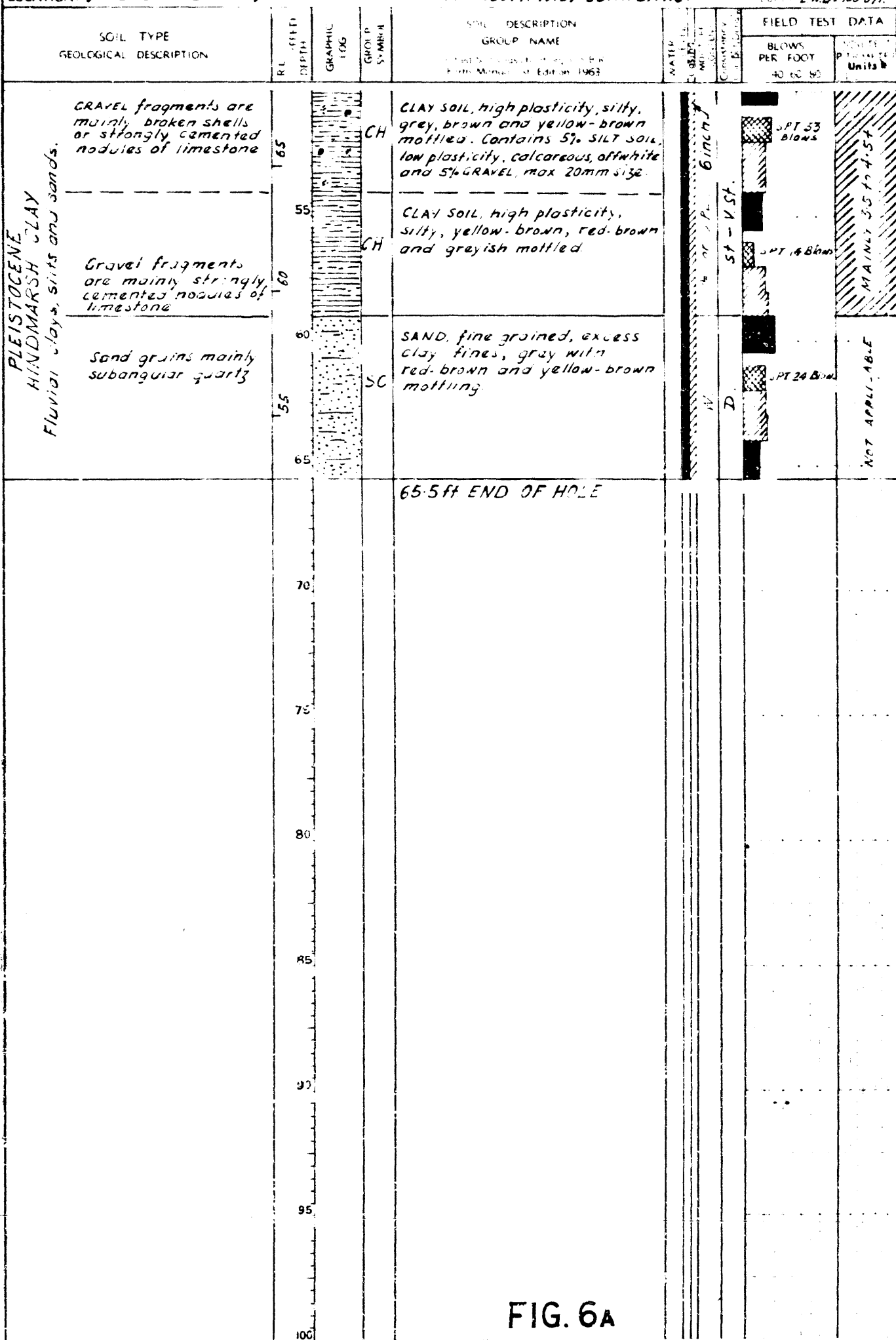
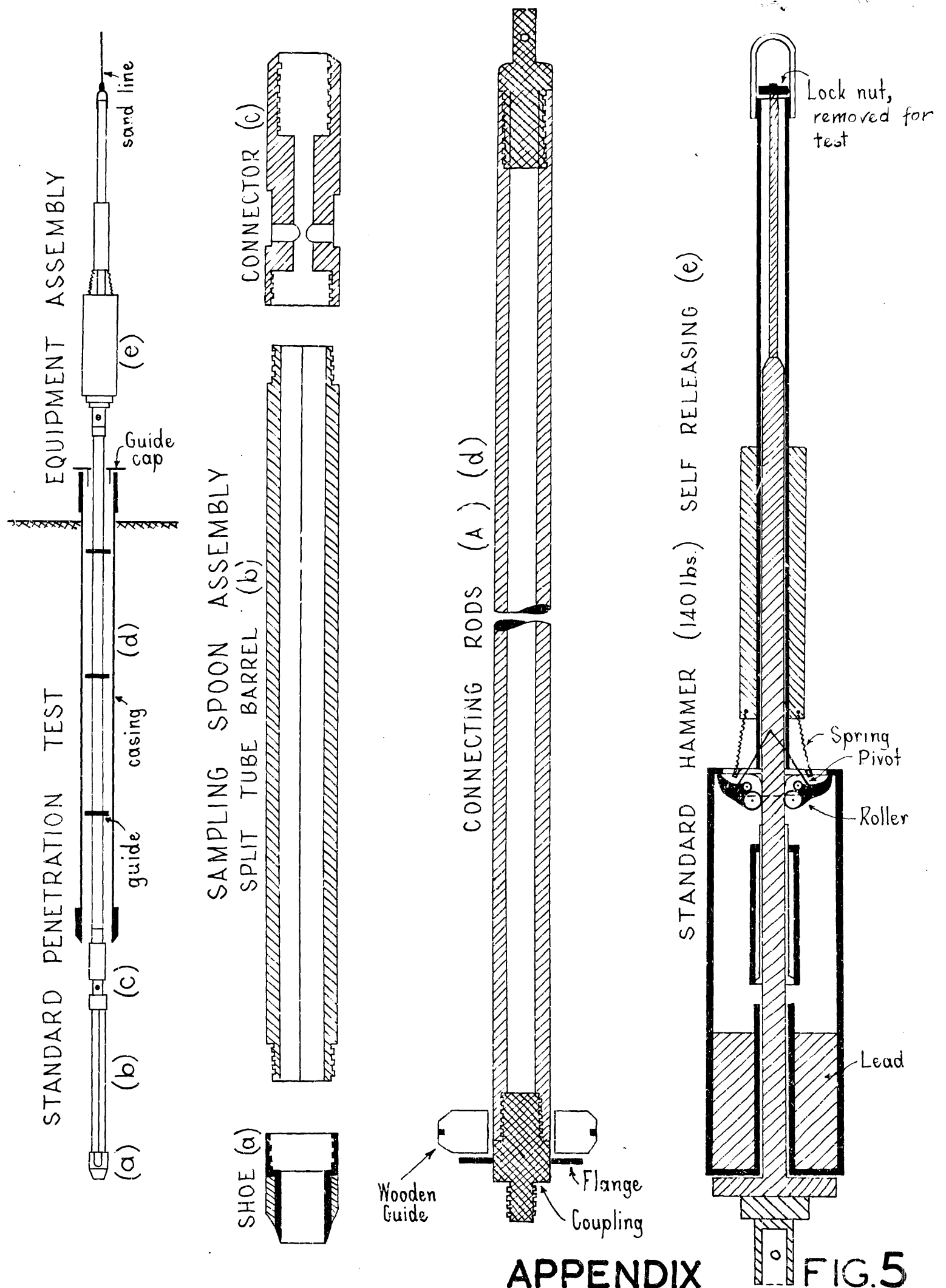


FIG. 6A

TYPE OF SAMPLE	CONSISTENCY	COMPACTNESS	RELATIVE DENSITY	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS - Very Soft	LS - Loose	VL - Very Loose	H - Humid	DRILL No 8	LOGGED BY
D " (SD)	S - Soft	MC - Moderately Compact	L - Loose	D - Damp	TYPE Ruston	R.D. Staal
E " (SE)	SF - Soft to Firm	C - Compact	MD - Medium Dense	M - Moist	DRILLER D. Phillips	DATE Mar. 68
G " (SG)	F - Firm	VC - Very Compact	D - Dense	W - Wet	START 27 Feb. 68	TRACED R.D.S.
Sealed Tube - A Shoe - SAL	FSt - Firm to Stiff		VD - Very Dense	S - Saturated	FINISH 29 Feb. 68	CHECKED W.R.P.B.
Standard Penetration Test - SPT	St - Stiff			LL - Liquid Limit	SHEET 2 OF 2	
	VSt - Very Stiff			P - Plastic Limit	DRG No	S6485a. Hal
	H - Hard					



APPENDIX

FIG.5

ENGINEERING
DIVISION

DEPARTMENT OF MINES - SOUTH AUSTRALIA

Scale: Not to scale

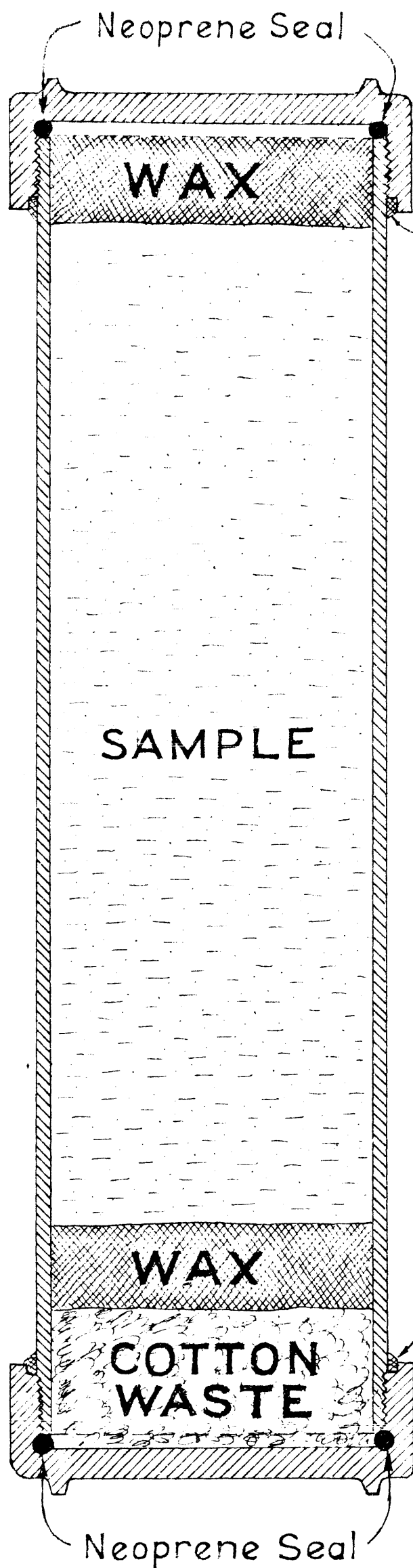
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Date: 22 Dec 1969

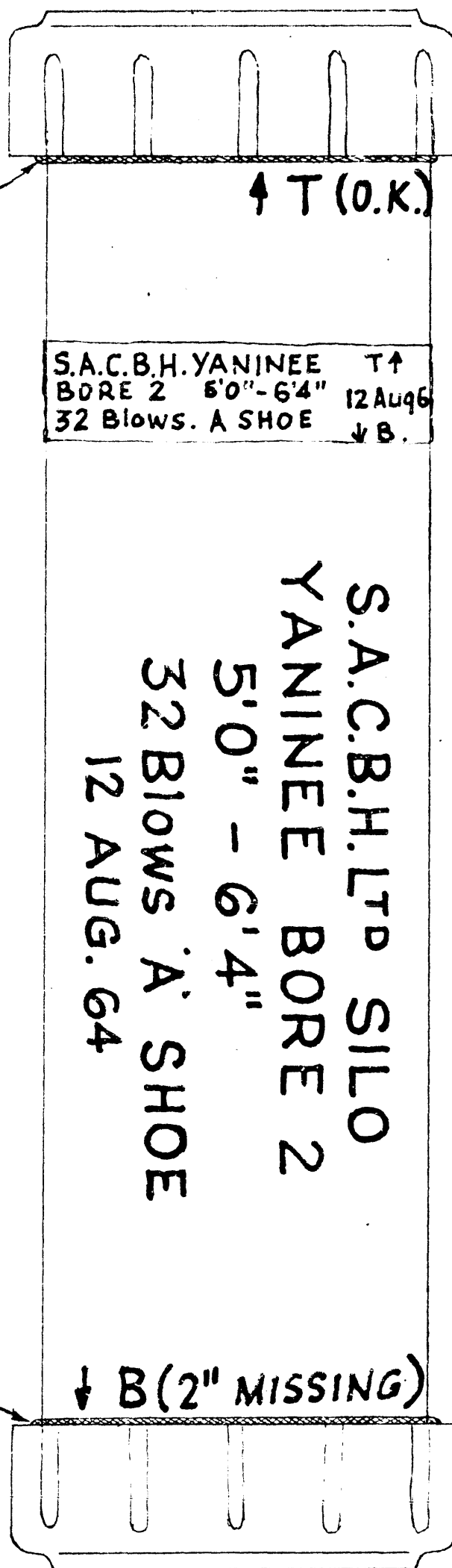
Drn. R.H. Ckd.

STANDARD PENETRATION TEST
EQUIPMENT

Drq. No
S4420 MB



SEALING



LABELLING

APPENDIX FIG. 3

DEPARTMENT OF MINES — SOUTH AUSTRALIA

ENGINEERING
GEOLOGY

Des. R.D.S.

Tcd. R.D.S.

DIRECTOR
OF
MINES

Chd. D.H.S.

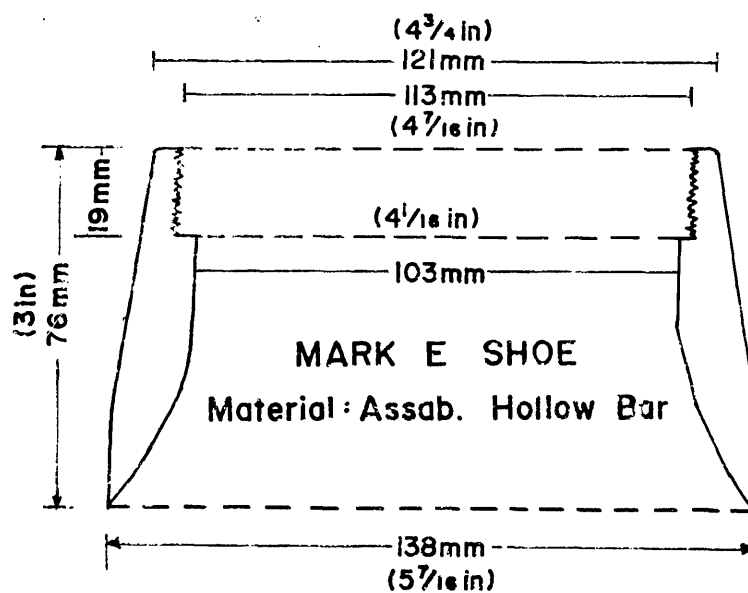
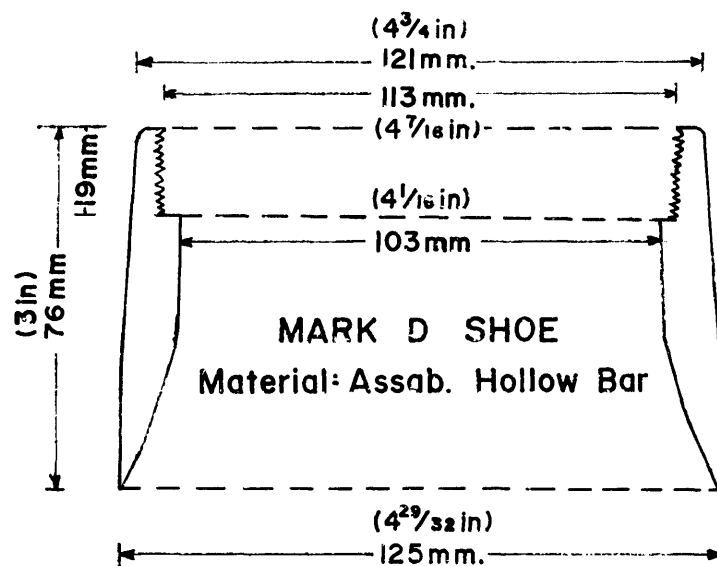
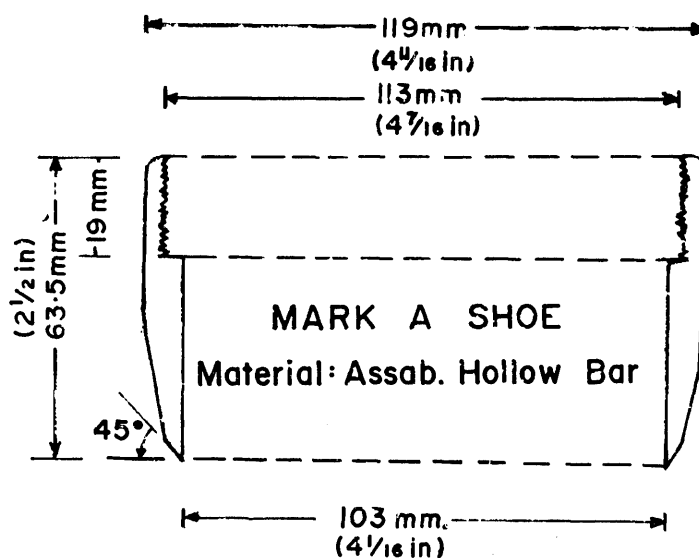
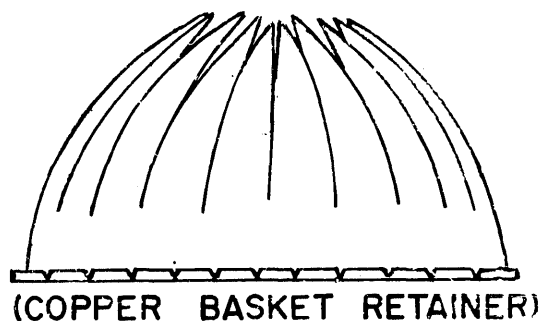
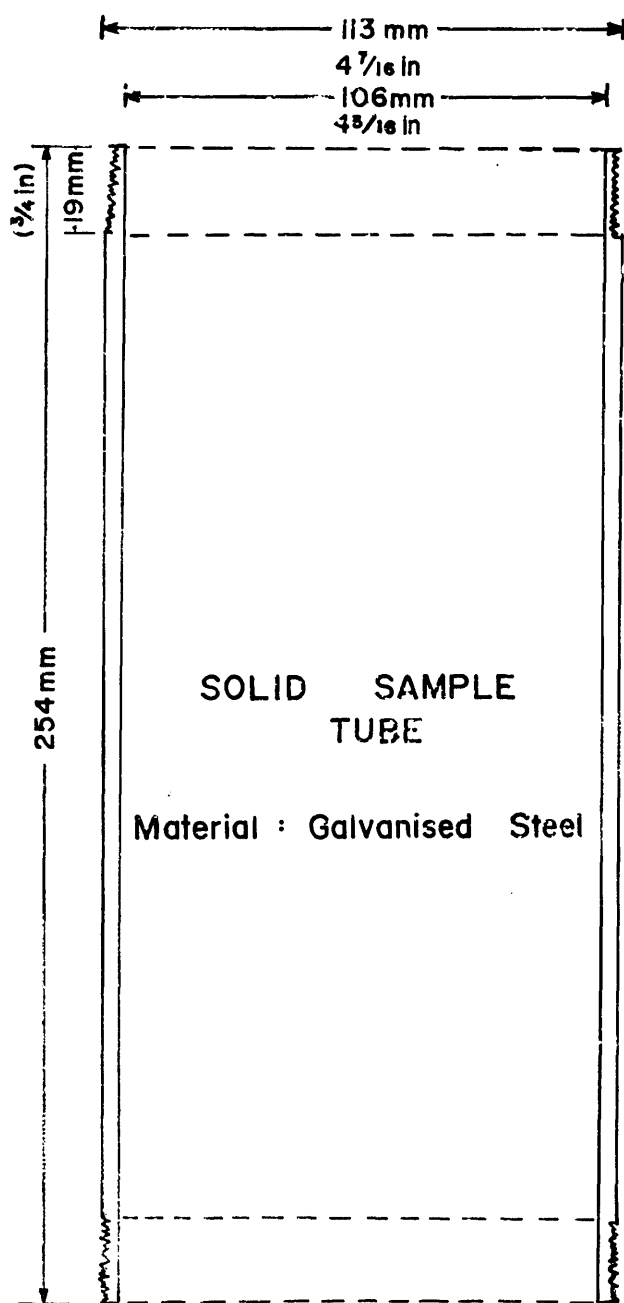
Exd.

CABLE-TOOL DRILLING
SEALED TUBE
SAMPLE

SCALE: NOT TO SCALE

S4419
MB

DATE: 29 JUNE 65



DEPARTMENT OF MINES - SOUTH AUSTRALIA

SCALE Not to Scale

COMPILED R. G. S.

CABLE - TOOL DRILLING

DATE 12 April 1978

REVISED T. E. L. K. D.

S SERIES SAMPLING
TUBE AND SHOES

PLAN NUMBER

S 4418

FIG.9-5

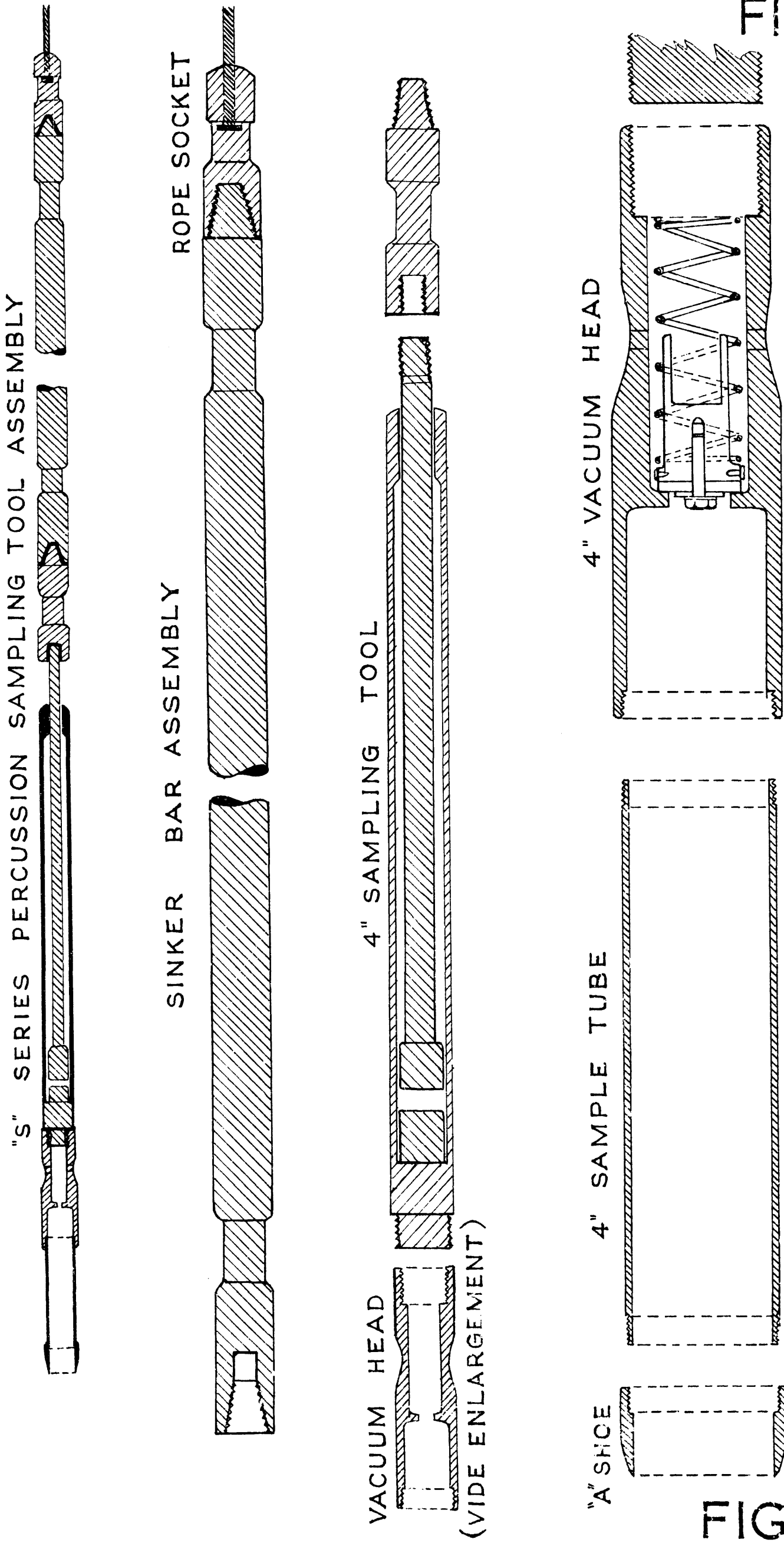


FIG.9-5

DEPARTMENT OF MINES — SOUTH AUSTRALIA

ENGINEERING
SERVICES
SECTION

DIRECTOR
OF
MINES

Drn. R.D.S.
Tcd. R.D.S.
Ckd. D.H.S.
Exd.

PERCUSSION DRILLING
SAMPLING TOOL ASSEMBLY
WITH "S" SERIES SHOES

SCALE: NOT TO SCALE

S4416 a.
MB

DATE: 29 June 65

