DEPARTMENT OF MINES AND ENERGY SOUTH AUSTRALIA

Rept. Bk. No. 89/89

NEPABUNNA - PROPOSED ARTIFICIAL RECHARGE SCHEME

GEOLOGICAL SURVEY

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GROUNDWATER AND ENGINEERING

DME 1002/74

<u>CONTENTS</u>		PAGE
ABSTRACT		1
INTRODUCTION		2
HISTORY		2
HYDROGEOLOGI	CAL SETTING	3
WATER DEMAND		5
PROPOSED RECI	HARGE SCHEME	5
DRILLING		5
DISCUSSION		6
CONCLUSIONS		7
RECOMMENDATIO	DNS	7
REFERENCES		8
	TABLES	
TABLE 1 TABLE 2	Water Analyses Wells Drilled 1989	4 6
	<u>APPENDICES</u>	
Appendix B Appendix C	Geological Logs Modelling of proposed recharge scheme Backhoe pitting Water consumption at Nepabunna	

<u>FIGURES</u>		PLAN NO.
Figure 1 Figure 2 Figure 3 Figure 4 Figure 5 Figure 6	Locality Plan Geological Map Well Location Plan Depth yield graphs Backhoe pit locations Infiltration tests	S21154 S21155 S21156 S21157 S21158 S21163

DEPARTMENT OF MINES AND ENERGY SOUTH AUSTRALIA

Rept. Bk. No. 39/89 D.M.E. No. 1002/74 B00919

NEPABUNNA - PROPOSED ARTIFICIAL RECHARGE SCHEME

ABSTRACT

Nepabunna's water supply has a salinity of 1300 mg/L and high carbonate hardness (550 mg/L). The latter causes severe scaling in pipes.

Artificial recharge was suggested as a means of introducing low salinity floodwaters into the aquifer to lower salinity and hardness.

Backhoe pitting in the stream bed confirmed the presence of thick clayey gravels and clays inhibiting natural recharge, but showed that they were too deep for modification of the stream bed to be economic.

Instead a scheme was proposed to divert some streamflow into an off-stream storage for later injection. A diversion weir and storage were designed by the Department of Agriculture and an injection well was drilled.

At this stage SACON reviewed the project and decided to proceed with water treatment rather than the artificial recharge scheme.

Subject to testing, the injection well may be suitable for equipping as a standby production well.

INTRODUCTION

Nepabunna is an Aboriginal village of 100 people in the north Flinders Ranges, about 65 km east of Copley by road (Fig 1). Aboriginal Works unit of SACON wished to undertake urther drilling to establish a better quality water supply.

SADME advised against this for the following reasons:

- A. SACON had spent a considerable amount running power lines to the existing wells, converting to electric motors and improving the headworks.
- B. Experience showed that it was unlikely that a better quality supply could be develope in this area.

Although a new well might have a lower salinity than the existing well when drilled its salinity and hardness would almost certainly deteriorate under sustained pumping to about that of the existing well.

Instead it was suggested that an artificial recharge scheme might be a better solution.

This report documents SADME's role in the investigation of an artificial recharge scheme.

HISTORY

Up until 1983 Nepabunna had a long history of water rupply problems as successive wells tapping limited local aquifers failed to meet demand (Read 1980 and 1983). At least 12 wells had been dug or drilled in attempts to provide Nepabunna with a reliable water supply.

In 1983 a well was drilled 2 km west of Nepabunna (Fig 2 & 3) to tap an extensive limestone aquifer. This ended the shortages in supply, but salinity, while better than the previous wells, was still poor and increased with time. The major problem is the high carbonate hardness (Table 1) which resulted in extensive scale deposition in pipes.

HYDROGEOLOGICAL SETTING

The production well taps an aquifer in the Parara Limestone, close to Mount McKinlay Creek (Fig. 2). Standing water level is about 45 m, showing that the regional gradient is gentle because of relatively high transmissivities and low recharge rates.

Groundwater in this area generally ranges from 1000 mg/L to 3000 mg/L, the better waters generally occurring close to creeks.

4
TABLE 1 WATER ANALYSES

Well	Date	Ca²+	Mg²+	Na+	K+	HCO ₂ ~	20.2				•
6636/101	0.7 (2.15		-		141	nco ₃ ~	SO ₄ 2~	C1-	Conductivity ECU	TDS mg/L	Comment
0026/101	25/1/86	320	60	214	4	495	335	554	3 200	_	
6636/149	13/5/83	142	76	85	5	472			3 200	1 740	Production Well
6636/149	26/10/84	7.00			•	4/2	259	153	1 547	954	Production Well
	20/10/04	188	74	109	4	569	295	177	1 790	000	- Foundation Hell
6636/149	23/1/86	196	77	130	5	591	320	210		990	
6636/149	5/6/87	210	86	105	_		320	218	2 100	1 190	
55354346	•	210	86	125	3	612	365	193	1 950	1 083	
6636/149	28/2/89								1 020		
6636/209	13/5/89	152	69	130	4	400			1 930	1 066	
				2.70	4	423	310	179	1 740	982	Proposed Injection Well

WATER DEMAND

SACON have estimated water demand at 12 kL/day (approx 4 ML/year).

However estimates based on hours of pumping suggest an average of 57 kL/day (21 ML/year) (Appendix D). An annual consumption in the range 10 to 20 ML/year is considered likely.

PROPOSED RECHARGE SCHEME

Several options for artificial recharge were considered:

- A. Modification of the stream bed to permit more rapid infiltration of stream flow.
- B. Construction of filte intakes under the stream bed to conduct water to an injection well during stream flows.
- C. Construction of a diversion weir and off stream storage for later injection by injection well.

Backhoe pitting showed (Appendix C) that permeabilities of the stream gravels and underlying weathered limestone were too low for 'A' or 'B' to be successful.

However preliminary investigations by Mr Moore of Department of Agriculture indicated that an off stream storage and diversion weir could be economically constructed near the production wells.

DRILLING

An injection well was drilled 45 m from the production well (Fig. 3). The geological logs are shown in Appendix A and the depth yield graph in Figure 4.

The first attempt was abandoned at 65 m when a hammer bit shanked off. It was left open as an observation well. The second attempt was successful. Details are in Table 2.

Although the well airlifted 20 L/s the aquifer is deep and apparently a finely fissured fault zone rather than the solution joints hoped for.

This makes the well more prone to clogging by larger particles. While most of the coarser suspended solids will settle in the dam, organic matter (algae, shield-shrimps) would develop and could rapidly clog the injection well.

TABLE 2 WELLS DRILLED 1989

<u>Permit</u> <u>No</u>	<u>Unit</u> <u>No</u>	<u>Depth</u> (m)	<u>Yield</u> <u>L/s</u>	Casing	<u>Comments</u>
252707	6636/165	65	0.3	4.5	Observation Well
952707	6636/209	101	20	98m of 125mm PVC slotted 14 to 98m	Injection well

DISCUSSION

After the injection well had been drilled SACON decided to review water supply options for Nepabunna.

The review concluded that the diversion and storage works needed to be much more substantial (and hence more costly) and that water treatment was more attractive.

Either reverse osmosis or ion-exchange softening could be used.

The former would require increased groundwater pumping. The latter would produce a high sodium water unsuitable for garden use.

CONCLUSIONS

- The injection well as drilled should be capable of accepting about 10 L/s.
- The injection well is liable to clogging.
- 3. As the recharge scheme is not to proceed the well drilled as an injection well may be suitable for use as a standby production well.

RECOMMENDATIONS

- If the desalination option is proceeded with the production well should be test-pumped to confirm that the present well and pump-setting are adequate for the resulting increased demand.
- If it is desired to equip the new well for production a pumping test should be carried out to determine the amount of interference.
- Because of the location of the new well it must be sealed to prevent entry of floods.

R E READ

SENIOR GEOLOGIST

REFERENCES

- Read, R.E., 1981. Nepabunna Water Supply, 1980 Drilling. SADME unpubl. report 81/13.
- Read, R.E., 1984. Nepabunna Water Supply 1983 Drilling, SADME unpubl. report 84/99.

APPENDIX A GEOLOGICAL LOGS

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APPENDIX B

MODELLING OF PROPOSED RECHARGE SCHEME

An attempt was made to model the behaviour of the proposed scheme using the following assumptions:

- 1. Storage volume of off stream storage is 10 ML.
- Daily creek flow at the proposed site is 1/40 of flow at the gauging station at Wertaloona, when this is over 40 ML/day.
- 3. Injection rate is 500 kL/day.

The design volume of the dam is 13 ML. Allowing for losses by leakage and evaporation this has been assumed + 9 10 ML.

The catchment area is $24~\rm{km^2}$ compared to $1010~\rm{km^2}$ at Wertaloona. Since the catchment upstream from Nepabunna is a better runoff generator than the catchment as a whole the estimate of creek flow as 1/40 of flow at Wertaloona should be conservative.

The streamflow record begins in September 1973. 1974 was a very wet year, and the recorder was out of action from May 1974 to November 1975. In the four months of 1974 for which records are available about 60 ML could have been recharged.

1974 was ignored and the period from February 1976 to December 1987 has been considered. Results indicate that an average of 7 ML/year could have been injected. This is considered a minimum estimate. Minor flows originating from thunderstorms in small catchments could provide sufficient flow to fill a 10 ML dam, but not reach the gauging station.

STREAMFLOW SALINITY

Information from Mr Steer of ETSA is that the salinity of water intake to Aroona Dam is under 200 mg/L.

The Aroona catchment includes some areas of shallow saline groundwater (e.g. Salt Creek) which could be expected to contribute more salt than the overall catchment. No such areas exist in the Nepabunna catchment.

200~mg/L is a reasonable estimate for the salinity of the water likely to be caught in the proposed dam.

QUALITY OF WATER WITHDRAWN

The average salinity of water pumped will be approximated by the following equation:

$$C = IC_1 + (E-I) C_2$$

where C is average salinity of water pumped

C₁ is salinity of injected water

C₂ is salinity of native groundwater

E is annual volume pumped

I is average annual volume injected for values of E greater than I.

 C_1 and C_2 are assumed to be 200 mg/L and 1300 mg/L respectively. Neither the rate of pumping nor the rate of injection are known accurately.

Estimated salinities of extracted water for γ_i rious ratios of injection to extraction are tabulated below.

I>E,	Salinity tends	to	200	mg/L
I=0.5E,	ti		750	mg/L
I=0.25E,	tr		1000	mg/L

APPENDIX C BACKHOE PITTING

Eight backhoe pits were dug in the bed of Mount McKinlay Creek and on the adjacent bank. Locations are shown in Figure 5 and the logs below.

The pits showed that in general 1 to 1.5m of loose relatively permeable gravels overlie about 1m of tight clayey gravels, which overlie clay derived from weathering the limestone.

The gravels ap ear permeability of 1 to 2m/day. The permeability of the gravels has not been estimated but would be low.

Infiltration rates in the weathered amestone were measured at about 0.5m/day, but this could decrease with time.

Infiltration rates in the weathered limestare were estimated by excavating small basins (~ 200 to 300 m lameter) on the floor of backhoe pits, filling them with water and measuring the rate of decline of water level. Results are shown in Figures 6 to 8.

Average infiltration rates are about 0.5m/day, but could decrease with time. To be effective a direct infiltration system would need to be able to infiltrate $1000~\text{m}^3/\text{day}$, that is an area of at least $2000~\text{m}^2$. It would be necessary to move about $6000~\text{m}^3$ of gravels out of the streambed. About $2000~\text{to}~3000~\text{m}^3$ of the more permeable upper gravels would then have to be replaced giving a total of over $8000~\text{m}^3$ to be moved.

Similarly construction of infiltration drains would be impractical using the low permeability river gravels. Permeable trenches backfilled with filter sand would be liable to scouring by floods.

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DEPARTMENT OF MINES SOUTH AUSTRALIA ENGINEERING DIVISION

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DEPARTMENT OF MINES SOUTH AUSTRALIA ENGINEERING DIVISION

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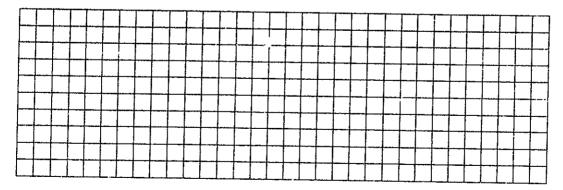
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DEPARTMENT OF MINES SOUTH AUSTRALIA

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DEPARTMENT OF MINES SOUTH AUSTRALIA ENGINEERING DIVISION

EXCAVATION NO.

LOG OF EXCAVATION

SECTION SURFACE ELEV

DATUM

LOCATION OR CO OFFS

HUNDRED DATE

UATE 28/2/89

TYPE OF EXCAVATION

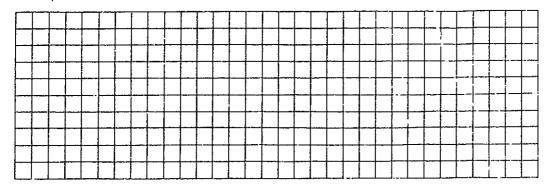
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FORMATION SOIL HORIZOR	ELEV. m DEPTH m	GRAPHIC LGG	UNIFIED	DESCRIPTION AND STRUCTURE !UNIFIED SOIL CLASSIFICATION}	WATER LEVE	A CONTROL	CONSISTEN REL DENS	1631	SAMPLE-
	11111	0 ·		Gravels varying from 0.8 to 1.1m thick.				; ;	-
	1,4,1,4,1	7		Hard yellow-brown limestone interbedded with limestone weathered to soft white silt.				1	1 1. 1.
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PROFILE/REMARKS



Water D. Dry S. Saft L. Laase TRACED	aq	logged R Read	*TEST. SAMPLE DESCRIPTION	RELATIVE DENSITY (ESTIMATED)	CONSISTENCY .	MOISTURE CONTENT	WATER LEVEL
Water LL Liquid limit St Stiff D - Dense SP=Seepage PL - Plastic limit H - Hard F - Firable SHEET OF NO S	9	DATE	•	MD Medium Danse	F Fun St Stiff	M - Moist W - Wer LL - Liquid limit	Vater

APPENDIX D

WATER CONSUMPTION AT NEPABUNNA

The estimate is based on readings of the hour meters on the pumps at Nepabunna and the assumption that the pumps are running at the design rate of 1.2 L/s.

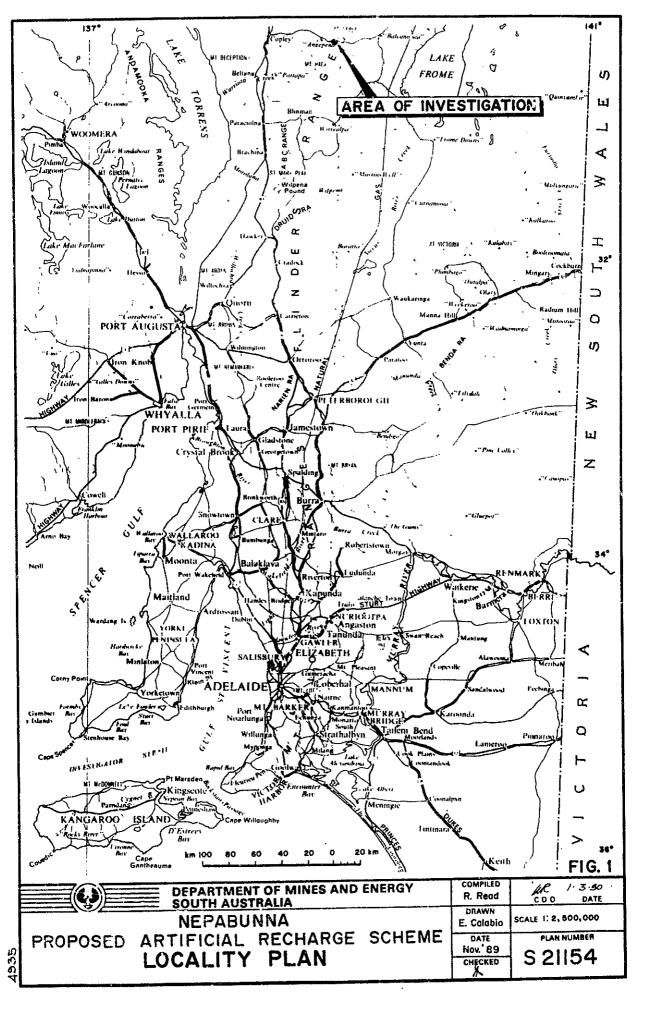
<u>Date</u>	New-Bore 'No 2'	Bore No 1
Sept 88	Pumps electrified	- meters at '0'
11/5/89	2439 hrs	- (Read by R. Read)
22/9/89	3646 hrs	1175 hrs (Read by J. Coulthard)

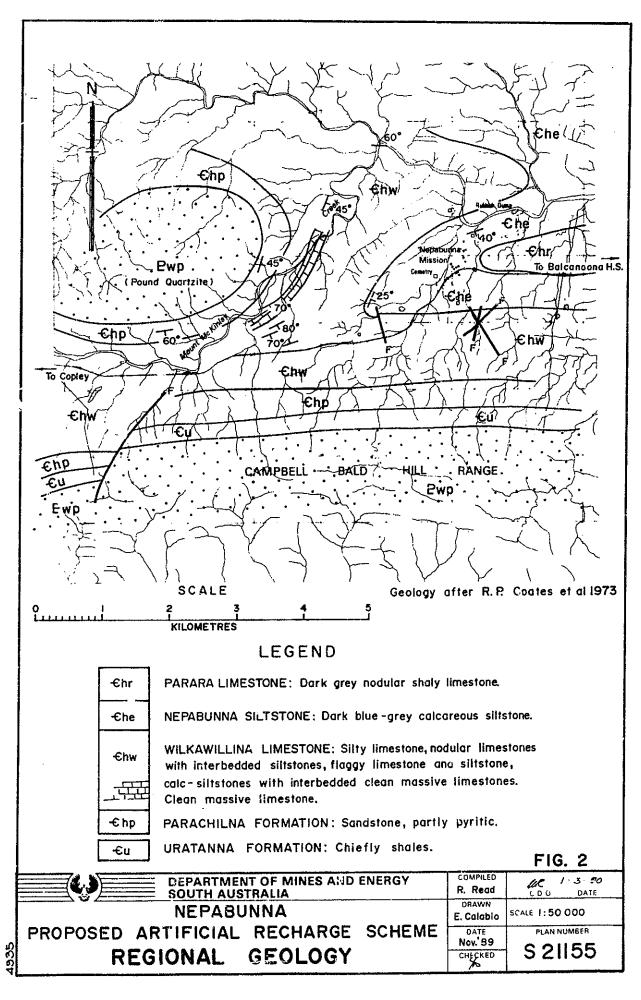
From the above it appears that 20 800 kl have been pumped in 12 months, an average of 57 kL/day.

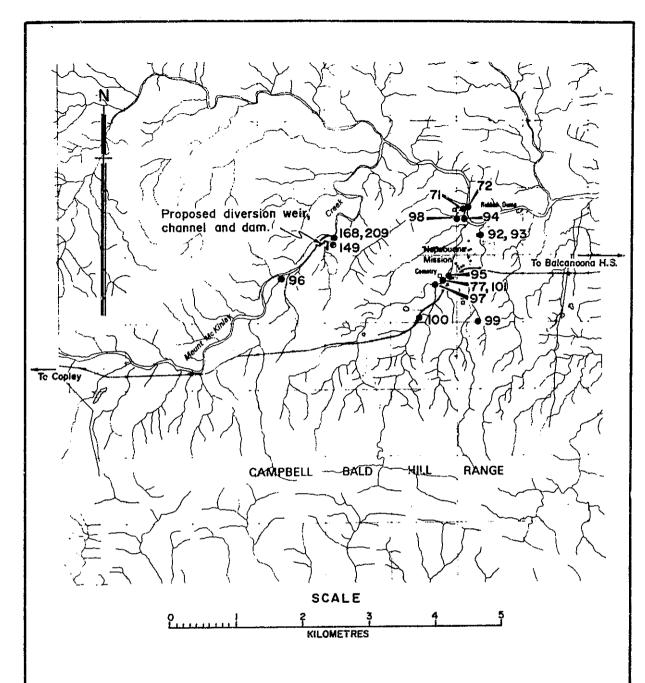
This is an average daily consumption of about 570 L per person, which is reasonable compared with 780 L per person at Leigh Creek.

The peak weekly demand is not known. Using the ratio between average and peak use at Leigh Creek gives about 90 kL/day.

A reasonable guess is that a system should be designed for a peak demand of about 75 kL/day, and an annual consumption of 21 ML.







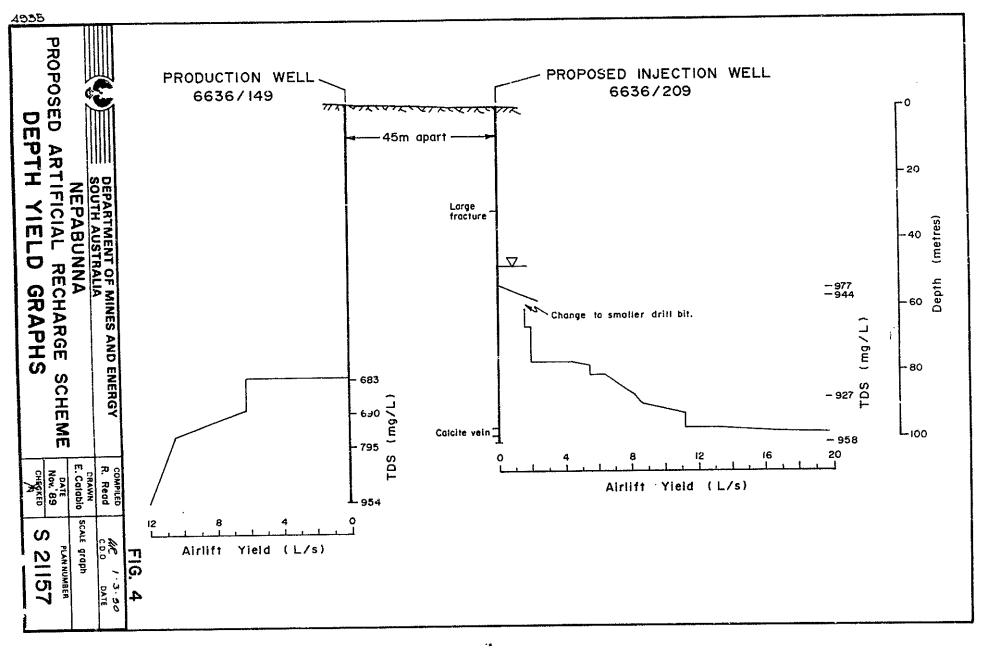
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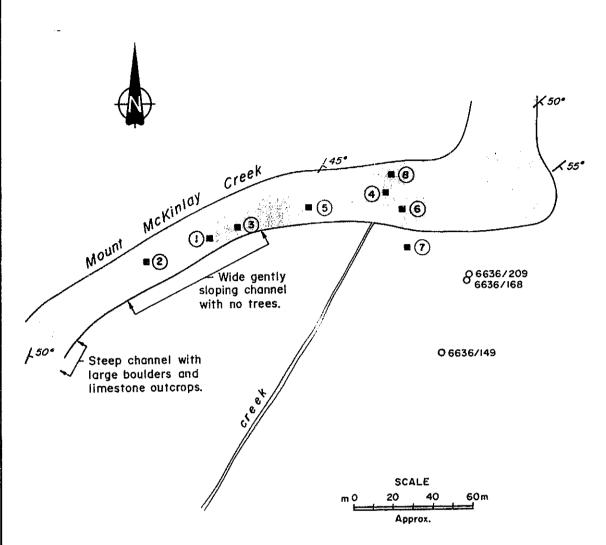
Water well location with:

• 100—Number (to obtain Unit No. prefix with 6636001WW00___)

FIG. 3

DEPARTMENT OF MINES AND ENERGY SOUTH AUSTRALIA	COMPILED R. Read	UR 1.3.30 CDO DATE
NEPABUNNA	ORAWN E. Calabio	SCALE 1:50 000
PROPOSED ARTIFICIAL RECHARGE SCHEME WELL LOCATION PLAN	DATE Nov. 89 CHECKED	PLAN NUMBER S 21156





Note: Pace and compass survey.

Water well location and number...... ... O 6636/168

		FIG. 5
DEPARTMENT OF MINES AND ENERGY SOUTH AUSTRALIA	COMPILED R. Read	UC 1-3-90 C.D.O. DATE
NEPABUNNA	DRAWN E. Calabio	SCALE As shown
PROPOSED ARTIFICIAL RECHARGE SCHEME	DATE Nov. 89	PLAN NUMBER S 21158
BACKHOE PIT LOCATIONS	CHECKED	3 21100