

DEPARTMENT OF MINES  
SOUTH AUSTRALIA

GEOLOGICAL SURVEY  
ENGINEERING DIVISION

GEOLOGICAL INVESTIGATIONS  
TARCOOLA ALICE SPRINGS RAILWAY  
ROBIN RISE TO MARLA BORE SECTION

by

J. SELBY  
SENIOR GEOLOGIST

and

G.H. McNALLY  
GEOLOGIST

R.F. JEUNE  
GEOLOGIST

ENGINEERING GEOLOGY SECTION

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TARCOOLA ALICE SPRINGS RAILWAY

ROBIN RISE TO MARLA BORE SECTION

SUMMARY AND CONCLUSIONS

Geological mapping for engineering purposes along the Robin Rise to Marla Bore Section has revealed a predominant surface material of silty residual soil with areas of drift and dune sand, underlain by kaolinised Bulldog Shale bedrock. A zone of calcified soil, varying from powder calcrete through weak gravelly to strong sheet calcrete occurs immediately above bedrock. Minor areas of strong jasper silcrete, gravelly alluvium, silcrete ferricrete and powder gypsum also occur. Engineering properties of these materials have been estimated from field examination and physical testing, and may be summarised as follows:-

- ..... Most soil materials are suitable for common fill, except for the powder gypsum associated with scattered playas and possibly some of the kaolinised Bulldog Shale.
- ..... Calcrete, alluvial gravel and possibly jasper silcrete will provide sources of selected fill up to the Murloocoppie area (chainage B60.0 km). Thereafter selected fill will be more difficult to obtain and the variable calcified soil horizon will have to be utilised.
- ..... No significant foundation problems are likely to occur provided undercutting is carried out in playas and compaction in areas of powder calcrete.
- ..... Excavation can be readily carried out by earthmoving equipment although heavy ripping and blasting may prove necessary in sheet calcrete, silcrete and in portions of the Bulldog Shale.
- ..... Gravels marginally suited for concrete aggregate occur at Long Creek and Mabel Creek. These materials are predominantly silcrete and reactive with cement. No other suitable sources of aggregate have been located and the use of ballast material for concrete is recommended.
- ..... Groundwater investigations continue along this section of the line. So far sufficient quantities for compaction are indicated although most water encountered is saline ranging from 3 000 to 66 000 mg/l. No potable water has so far been encountered.

## INTRODUCTION

The first report on the Engineering Geological investigations for the Tarcoola Alice Springs Railway covered the Section from Tarcoola to Robin Rise (Jeune: Rept.Bk.No. 72/157). This report continues the investigations for a further 250 km north to Marla Bore Airstrip. Purpose of the investigations are to:-

- ..... Examine and describe the rocks and soils to determine their engineering properties.
- ..... Locate suitable sources of concrete aggregate.
- ..... Locate sources of water for camp and construction use.
- ..... Examine foundation conditions for bridges at stream crossing sites.

This report presents the results of geological investigations along the Robin Rise to Marla Bore Section of the proposed line, together with a summary of the results of Hydrogeological Drilling to the end of 1973. It has been compiled by J. Selby and G.H. McNally from notes and maps prepared by R.F. Jeune.

Bridge sites along this section at Long's Creek and Mabel Creek, have been investigated and are covered by a separate report (Jeune, Rept.Bk.No. 72/192). Drilling at other minor bridge sites is in progress and results will be presented at completion of the programme (McNally, 1975, in preparation).

## METHODS

Base maps covering this Section of the proposed line (from A160 km to B196 km) at a scale of 1:24 000 were supplied by Commonwealth Railways, together with working plans and sections.

Soils and rocks were examined in natural exposures, pits and in a series of trenches excavated by bulldozer. These trenches have been left open for inspection, and it is requested that they be filled in during construction.

Selected samples of materials were submitted to South Australian Highways Department and to the Engineering and Water Supply Department (Appendix III).

Geological boundaries were plotted from air photo interpretation, supplemented by field checks.

### TOPOGRAPHY

Topography over this part of the railway varies and may be summarised as follows:-

<u>Section</u>	<u>Topography</u>
Robin Rise to Mabel Creek	Flat stony desert of lag gravel*
Mabel Creek to Murloocoppie	Low undulating hills with sharp rock outcrops
Murloocoppie to Mt. Willoughby	Flat sandy desert with claypan "crab-holes"*
Mt. Willoughby to Wintinna Hill	Low undulating hills with sharp rock outcrops
Wintinna Hill to Marla Bore	Flat sandy desert

Two major water courses, Long Creek and Mabel Creek, cross the line. "Crab-holes" occur in the floor of many of the shallow claypans and this country (Qpr) becomes very difficult to negotiate in wet weather.

### GEOLOGY AND ENGINEERING PROPERTIES

The regional stratigraphic sequence is given in Table 1 and detailed surface geology is shown on the maps. Dual symbols have been used on the maps in which the upper symbol indicates surface material and the lower, probable subsurface material. This system has been devised because the presence of subsurface calcrete is important in the planning of construction techniques.

A detailed discussion of each stratigraphic unit follows. Samples tested from each unit are listed in Table 3. *and a summary of anticipated conditions in Appendix I.*

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\*Lag gravel: Residual accumulation of coarser particles from which the finer material has been blown away.

\*Crab-hole: A cavity usually occurring in the centre of a shallow claypan caused by the infiltration of runoff into deeper, more permeable layers.

### RECENT ALLUVIUM (Qra)

Unconsolidated, generally clean, sands and silty sands of the present stream courses.

Minor deposits of locally derived sands and silty sands partially fill minor watercourses. Such deposits are thin (15-30 cm), of extremely limited extent and are probably mobile during periods of runoff. Large quantities of fine gravel occupy the channels of Long Creek and Mabel Creek. These deposits are discussed in greater detail in Jeune, 1972 (Rept.Bk.No. 72/192).

#### Engineering Properties

Deposits in local watercourses would be suitable for low strength concrete but quantities are too small for practical exploitation. Since these deposits are mobile, culverts must be designed to accept high bed loads. Culvert sites should be undercut to relatively impervious materials to prevent undermining and collapse.

Alluvial materials are suitable for use as fill but their poor grading may cause difficulties in compaction. Normal earth moving methods should prove adequate.

### PLAYA DEPOSITS (Qr1)

Saline, gypseous, clayey and silty deposits of lakes, claypans and swamps (usually dry).

Depressions forming local base level for closed drainage systems contain fine grained alluvial deposits, commonly mixed with variable proportions of gypsum and soluble salts. A porous, low density surface crust of evaporite is common in larger playas. Although these deposits are normally dry they become soft and wet after rain. A bed of compact crystalline gypsum encountered in playas on the Tarcoola-Robin Rise Section appears to form an impervious seal limiting the downward percolation of surface water but there is no certainty that this seal is ubiquitous. Total thickness of playa deposits is unknown.

## Engineering Properties

Softening of playa deposits following rain is a potential cause of foundation failure, and since drainage is virtually impossible, design should allow for the possibility of such softening occurring during service. Removal of the thinner deposits and replacement with moisture-insensitive material (e.g. drift sand) is recommended. Where thicker deposits are encountered, it is suggested that design should provide for the correction of any settlement by packing and that the toes of potential shear failures should be loaded by berms of wide based fills. Excessive undercutting (say greater than 0.5 m) in thick deposits and replacement with sand is not recommended as this would provide a percolation path to lower levels.

Provision of culverts through fills is recommended to prevent the build up of hydraulic gradients.

Normal earth moving excavation methods, possibly with light ripping should prove adequate when materials are dry. However, should these materials become wet they become impassable to wheeled traffic, and tracked machinery or possibly a dragline may prove necessary. Heavy ripping may be required if excavation of the compact crystalline gypsum bed becomes necessary.

Use of Playa Deposits as fill is not recommended due to the presence of soluble salts. However, their use as a non load-bearing water-bound protective skin may prevent excessive wind erosion of sand fill.

### DRIFT SANDS (Qrs) AND DUNE SAND (Qrd)

Sands and silty sands forming drift sheets and dunes.

These sand deposits are normally red-brown in colour, loose to medium dense, poorly graded and contain only minor clay binder.

Dunes are confused in form, and partially fixed by vegetation but areas of blow-out are common and sand movement is active. The major period of sand movement was probably Late Pleistocene - Early Holocene and present sand movement could be due to the removal of binding vegetation by pastoral activity.



### Engineering Properties

Bearing capacities are adequate and no significant settlement should occur provided loose surface material is removed or compacted prior to emplacement of fill. Normal earth moving methods, possibly with minimal light ripping, should prove adequate.

Although binder content is low, these materials are suitable for fill. Relative compaction at natural moisture contents will be low, and problems with dust and "live" materials will almost certainly arise. Deflation will almost certainly occur during service life. It is recommended that wherever possible at least the outer layers of fills should be compacted damp and protected by silcrete gravel, calcified soil, or Residual Soil.

Batter angles in fill and long term cut batter angles should approach the angle of repose of loose dry sand ( $33^{\circ}$ ). Accordingly, both cut and fill batters of  $35^{\circ}$  are recommended.

### OLDER PLEISTOCENE ALLUVIUM (Qpa)

Dense to medium dense dirty gravel, gravelly sand, and silty sand with occasional silt lenses.

These deposits occupy older channels of major streams and grade laterally into Residual Soil (Qpr) and Tertiary Silcrete (Tsi).

### Engineering Properties

Bearing capacity is adequate for fills. Normal earth moving methods, possibly with minimal light ripping, should prove adequate. All materials examined should prove adequate as fill and no protection against deflation should be necessary. Batter angles of  $35^{\circ}$  are recommended for both cuts and fills.

Care should be taken to ensure that stream channels are not unduly restricted as, although these materials appear to resist scour under present conditions, it is likely that scour could occur if stream velocities are increased significantly.

### RESIDUAL SOIL (Qpr and Qps)

Medium-dense red brown silty clay, commonly with scattered sub-rounded pebbles up to 5 cm diameter. A lag gravel ("gibber") surface skin is common, and much of the area mapped as Residual Soil forms "Crab-hole country" - a series of scattered circular hollows up to 5 metres in diameter and up to 1 metre deep. When wet, residual soil becomes extremely soft and slippery (impassable to 4 wheel drive vehicles). Gravelly slope wash material of the same age has been distinguished as (Qps). The unit referred to as the "Doonbara Formation equivalent" in bridge site logs (McNally, 1975) is largely a Residual Soil in origin, though it may have been reworked in places by stream action.

#### Engineering Properties

Although these materials soften when wet, bearing capacity should prove sufficient to support the formation. Adequate drainage should be provided. These materials should provide excellent fill if compacted damp, no protection against deflation being necessary. Fills should be shaped to shed water to prevent undue softening by rain. Minimal light ripping may be necessary in excavations. Batter angles of 45° are recommended.

Laboratory sieve analysis tests (Figs. 9, 14, 18, 19, 21, 22 and 33) indicate that this material is moderately well-graded with a variable silt/clay content.

### CALCRETE (Qca) AND CALCIFIED SOIL (Qc1)

Nodular and strong sheet calcrete (Qca) and zone of sub-soil lime accumulation (Qc1).

The following classification has been adopted from a more comprehensive scheme proposed by Netterberg (1967).

Nodular calcrete - Discrete, medium strong to strong concretions of carbonate-cemented soil in a loose calcareous soil matrix.

Sheet calcrete - Medium strong to strong near horizontal, continuous near continuous or broken sheets of carbonate-cemented soil, underlain by loose fine grained carbonate powder calcrete.

Calcified soil - Soil weakly cemented by carbonate.

These categories intergrade but have been found useful as descriptive terms. Although sheet calcrete and nodular calcrete have slightly different engineering properties, it was not found practical to map them separately, as their natural surface expressions are similar.

Netterberg states that sheet calcrete is invariably underlain by loose powder calcrete, and that the stronger the sheet calcrete the weaker and more useless for road building purposes will be the underlying powder calcrete. Although sheet calcrete could rarely be penetrated during this present survey no evidence was found to contradict Netterberg's generalisation, and loose powder calcrete may be expected to underly sheet calcrete.

During this survey, weak ironpan was almost invariably found in intimate mixtures with calcified soil and has been mapped with the calcified soil.

Sheet and nodular calcrete are almost invariably overlain by a thin zone of calcified soil.

Calcrete and calcified soil are zones of subsoil lime accumulation thought to result from concentration of windblown calcareous dust blown from the Nullarbor Plain. Although probably still forming, the main period of formation was probably contemporaneous with the period of main sand movement, i.e. Late Pleistocene - Early Holocene.

#### Engineering Properties

Sheet calcrete should have reasonable bearing capacity provided there remains a thickness of not less than about 0.8 m over the underlying powder calcrete. It is recommended that, wherever calcrete is encountered in excavation its thickness should be checked with a jack hammer hole prior to emplacement of

fill. Every effort should be made to avoid excessive excavation of calcrete into the underlying weaker material. Calcified soil by the nature of its wind blown deposition in an arid climate, commonly consists of sand grains weakly cemented by lime and is of low density. These materials are of low bearing capacity and "collapse" readily under load particularly when wet. It is difficult to tell in the field which limy sands are "collapsing" and which are "non-collapsing" as density is the main indicator. Thus it is recommended that all limy soils (including nodular calcrete) should be compacted, preferably with the addition of water, prior to placement of fill. This should result in a dense strong foundation material, with no likelihood of differential settlement. Densities achieved by dry compaction are likely to be low.

Excavation of calcified soil and nodular calcrete required only light ripping by the rubber tyred Le Torneau LW16 dozer used in investigations. However this machine proved incapable of ripping strong sheet calcrete and heavier tracked machinery will be necessary. Blasting may be necessary if particularly strong calcrete is encountered.

All the limy materials except powder calcrete would make good fill material, as any reworking results in the breakage of the weak lime cement bond between sand grains and thus, in placing, a reasonably dense fill result. Compaction will be improved by addition of water.

Some wind erosion of the lime may occur on exposed surfaces, but the severity would depend on the presence and quantity of gravel sized calcrete particles. Use of calcrete, preferably water bound, as a protective outer zone is recommended.

Due to the lime cement, batters may stand at a steep angle in the dry condition, but for long term stability batters of  $35^{\circ}$  are recommended.

### YOUNGER SILCRETE (Qsi) - "JASPER"

Medium strong to strong, red, pink and white silcrete and silicified kaolin clay, forming ridge cappings and scarps.

This material apparently formed during a later phase of silicification than Tertiary Silcrete (Tsi) from which it varies mainly in colour, lower density, closer jointing (2-5 cm) and lower strength. In most exposures examined it grades down through slightly silicified white and red kaolinitic clay to unsilicified white kaolin clay (bleached Bulldog Shale). Silicification is thought to post date kaolinization, and is probably early Pleistocene in age.

#### Engineering Properties

Bearing capacity is adequate to support the formation. Excavation will require medium to heavy ripping with some blasting. These materials would form acceptable fill. A waterbound mixture of younger silcrete and Residual Soil would form a good protective blanket to protect underlying materials against deflation or scour. Since scarps stand vertically, cut batters could be expected also to stand vertically.

### FERRICRETE (Tfe)

Strong sheet ferricrete (ironstone) and coarse lag gravel resulting from the physical disintegration of such sheets.

Intact and near intact ferricrete sheets cover large areas adjacent to the Long's Creek Crossing. Thickness of sheet ferricrete could not be accurately measured but is estimated at 30-50 cm.

Sheet ferricrete appears to be closely related to Tertiary Silcrete (Tsi) and marks the position of the same mid-Tertiary land surface.

#### Engineering Properties

Intact and disintegrated sheet ferricrete should provide adequate foundations for earthworks. Excavation will require medium to heavy ripping, though only minor blasting should be necessary. Ferricrete should form

acceptable fill, particularly if mixed with suitable fines to act as binder. Its use as a protective blanket over sand fill is recommended.

### TERTIARY SILCRETE (Tsi)

Strong sheet silcrete, and coarse lag gravel resulting from the physical disintegration of such sheets.

Small areas of intact or near intact sheet silcrete occur as ridge cappings and as isolated patches in topographic lows. Thickness of sheet silcrete could not be measured but is estimated at 30-50 cm.

Sheet silcrete marks the position of an old mid-Tertiary land surface that has been almost completely destroyed by more recent erosion.

#### Engineering Properties.

Sheet silcrete should provide adequate foundations for earthworks and no surface preparation should be necessary before emplacement of formation. Excavation will probably require light to medium ripping and minor blasting. The rubber tyred Le Torneau LW16 tractor used in investigations proved incapable of ripping sheet silcrete but heavier tracked machines should prove suitable and only minor blasting should be necessary. Use of material as a protective blanket over sand fills is recommended.

Silcrete contains silica as opal or chalcedony and is potentially reactive with cement. The extremely sharp fragments that are liable to fly when silcrete is crushed or struck could cause serious injury and construction staff should be advised to protect their eyesight when working with this material.

The life of rubber tyres is likely to be low.

### BULLDOG SHALE (K1b)

Stiff white and red/white mottled kaolinitic clay and weak siltstone, in part weakly cemented by secondary silicification.

### Stratigraphic Relations

Kaolinitic clay mapped in this report as Bulldog Shale represents the upper part of the unit that has been substantially modified by Tertiary and Quaternary weathering processes. Silicification of this weathered profile, presumably during early Quaternary times formed a silcrete profile, mapped separately in this report as Younger Silcrete (Qsi).

Disintegration of this silcrete profile and further weathering apparently formed the soil profile mapped as Residual Soil (Qpr).

Kaolinitic clay mapped as Bulldog Shale (K1b) probably underlies most of the area covered by this report and can thus be expected in deep excavations. It occurs at surface or near surface only where overlying materials have been stripped by comparatively recent erosion.

### Engineering Properties

Special laboratory tests were devised to cope with the blocky nature of this material:-

..... samples were crushed to pass BS 3/4" (19 mm) sieve and sieve analyses were carried out.

..... Representative samples of these were sieved on a BS No. 7 (2.36 mm) sieve, the portions retained being washed and oven dried. The oven-dried samples were supported on sieves under a fine garden sprinkler and the water turned on. Moisture contents were taken at ten minute intervals for one hour. Moisture content versus time graphs are shown in Fig. 36.

..... The crushed samples MWBP 1 and B49.200 km (passing 19 mm sieve) were wetted to the moisture content corresponding to ten minutes sprinkling, and sample B139.100 km wetted to the moisture content corresponding to twenty minutes sprinkling. These samples were compacted using the Modified AASHO Test and then the whole compacted sample was oven-dried to obtain moisture content and dry density.

..... Sieve analysis was carried out on each sample to ascertain breakdown during compaction.

Results of all of these tests are given in Figs. 10, 16, 20, 26, 27 and 34.

It may be noted that the effect of wetting and compaction tends to produce fairly uniform, well-graded curves for each sample. Bulk densities, although low, are considered acceptable for embankment fill. Material well-cemented by iron oxide or silica may be expected to compact adequately at low moisture contents. However the whiter, more kaolinitic material such as that found at Mount Willoughby Borrow Pit has performed poorly as unsealed surface course on the Stuart Highway, after excessive wetting due to heavy rain.

It is recommended that compaction be carried out after visual rejection of the most kaolinitic material, at low moisture content and at low loads, preferably with a grid roller. Some of the more cemented material may also prove acceptable for selected fill. Excessive compaction should be avoided.

Normal earthmoving machinery should prove adequate for excavation and it is expected that bearing capacity of the dry in situ material will prove adequate.

Further comments on the compaction of this and other fill materials are included in Appendix III.

#### CADNA-OWIE FORMATION (K1c)

Weak to very weak white quartz sandstone (or compact sand) with kaolinitic matrix. Irregular secondary silica cementation forms zones of strong rock.

No Cadna-owie Formation was identified in outcrop but was encountered in drilling at Long's Creek bridge sites (Jeune 1972, RB. 72/192, discussed as Algebuckina Sandstone). Cadna-owie Formation is believed to underly small areas adjacent to Long Creek.



### Engineering Properties

Bearing capacity is high. Excavation may require ripping and could require blasting should strong silicification be encountered. Sandstone should make excellent fill. Cut batter angles of  $45^{\circ}$  and fill batters of  $25^{\circ}$  are suggested.

### WATER SUPPLIES

Compaction water requirements have been estimated at  $230 \text{ m}^3/\text{day}$ . Saline water is acceptable for compaction but small quantities of potable water are required for mixing concrete and for camp purposes.

Rainfall is intermittent and permanent water supplies are absent.

### Drilling Programme

By completion of the 1973 drilling programme in October 1973 a total of 7 successful bores had been completed along this section of the railway. Results of drilling are summarised in Table 2 and successful bores are shown on the detailed geological plans.

Drilling continued during 1974 and 1975.

### Potable water

Groundwater at Murloocoppie (Water well 29, at B56.5 km) was found to be comparatively fresh although yield was low. This water may be suitable for general use.

Fresh water also occurs at Mt. Willoughby and this will be further investigated during the 1974 season.

### Water for Concrete

Generally water most suitable for concrete making has a Total Dissolved Solids (TDS) content of less than  $5\,000 \text{ mg/l}$  and an Na ion content of under  $1\,000 \text{ mg/l}$ . If these TDS values are exceeded, tests for reduced strength and length change should be carried out before using the water. A high Na/K ion content is significant if an alkali-reactive aggregate is to be used. In

In addition a Cl ion content of 1500 mg/l is generally accepted as the maximum permissible figure for concrete in contact with reinforcing steel.

Adopting the above criteria an assessment of the suitability for concrete from all water wells so far drilled on this section of the line has been given in Table 2. This information is based on full analyses given by Smith (Rept.Bk.74/12).

#### CONCRETE AGGREGATE

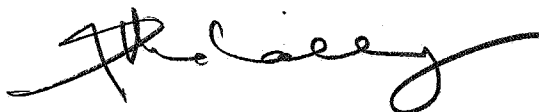
Gravel deposits suitable as sources of low quality aggregate have been located in the river beds of Long Creek and Mabel Creek. The gravel is composed predominantly of silcrete which is reactive with cement, although this material has been used for concrete in engineering projects at Coober Pedy. Available volume of this material has been estimated by Jeune (Rept.Bk.72/192).

Tertiary silcrete could be crushed and screened to provide aggregate but quantities available at any site are low. This silcrete contains opal or chalcedony and so is potentially reactive with cement, though it has been used as a source of concrete aggregate in engineering projects at Coober Pedy.

Good quality sheet and nodular calcrete is used for concrete aggregate in some areas of the State. Its quality is however variable and much of this material has a high dust content after crushing.

The most reliable source of concrete would appear to be ballast material, which for this Section of the line will probably consist of porphyry from Perfection Well.

13th March, 1975  
JS:RFJ:IA



G.H. McNALLY  
GEOLOGIST



J. SELBY  
SENIOR GEOLOGIST



R.F. JEUNE  
GEOLOGIST

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TABLE 1  
PROBABLE REGIONAL STRATIGRAPHIC SEQUENCE  
(ROBIN RISE TO MARLA BORE SECTION)

Age	Mapping Symbol	Unit	Lithology
QUATERNARY	Qra	RECENT ALLUVIUM	Unconsolidated, generally clean sands and gravels of the present stream courses.
	Qr1	PLAYA DEPOSITS	Saline, gypseous, clayey, and silty deposits of lakes, clay pans and swamps (usually dry).
	Qrs	DRIFT SAND	Sands and silty sands forming drift sheets.
	Qrd	DUNE SAND	Sands and silty sands forming dunes.
	Qpa	OLDER ALLUVIUM	Dirty gravel and sand of former river channels.
	Qpr/Qps	RESIDUAL SOIL	Silty clay with a skin of lag gravel.
	Qcl	CALCIFIED SOIL	Zone of subsoil lime accumulation.
	Qca	CALCRETE	Nodular, and strong sheet calcrete.
	Qsi	YOUNGER SILCRETE	"Jasper" silcrete forming ridge cappings and scarps.
TERTIARY	Tfe	FERRICRETE	Sheet "ironstone" and coarse lag gravel.
	Tsi	TERTIARY SILCRETE	Sheet silcrete and coarse lag gravel.
MESOZOIC	K1b	BULLDOG SHALE	Mottled kaolinitic clay partly silicified.
	K1c	CADNA-OWIE FORMATION	White Quartz sandstone with kaolinitic matrix.

TABLE 2,  
GROUNDWATER SUMMARY  
(ROBIN RISE TO MARLA BORE SECTION)

Well	Location km	Depth m	Static W.L. m	Yield l/sec (gph)	Salinity mg/l	Main Aquifer	Suitability for Concrete
17	162A	58.5	13.4	7.8 (6200)	66 900	Jua	Probably unsuitable
15	170A	39	20	5.3 (4200)	42 200	Jua	Probably unsuitable
26	202A	100	73	2.0* (1600)	8 200	Jua	Not suitable for reinforced. Strength and shrinkage tests recommended.
27	1B	94	65	2.5 (2000)	2 500	Jua	Suitable
28	17B	132	105	0.5* ( 400)	3 500	Jua	Borderline for reinforced
29	56½B	50	30	1.0 ( 800)	1 735	K1b	Suitable
30	To be completed in 1974 season						

\*Bore is capable of greater yield

Jua Algebuckina Sandstone

K1b Bulldog Shale

NOTE: Water well drilling is to be continued along this section during 1975

TABLE 3

## SUMMARY OF LABORATORY SOIL TEST RESULTS

## Compaction Tests

Sample Chainage (km)	Mapping Unit	Consistency Limits				Est. C.B.R.	Field M.C.%*	Standard AASHO**		Modified	AASHO
		L.L.	P.L.	P.I.	L.S.			M.D.D. (p.c.f.)	O.M.C. %	M.D.D. (p.c.f.)	O.M.C. %
164.7B	Dune Sand (Qrd)	-	-	N.P.	-	13.7	1	-	-	116	10
179.0B	" "	-	-	N.P.	0	19	0.5	114	11	-	-
160.0B	Drift Sand on Residual Soil (Qrs/Qpr)	20	12	8	-	14.5	3	-	-	130	8
27.9B	Residual Soil (Qpr)	36	32	4	2	28	6.3	95	23	-	-
97.0B	" "	43	17	26	12	7.3	6.8	105	19	-	-
100.0B	" "	-	-	N.P.	-	42	9.5	-	-	95	21.5
130.0B	" "	48	22	26	-	8.2	7	-	-	113	14
190.0B	" "	20	9	11	5	19	3.5	137	9	-	-
170.0A	Residual Soil + Silcrete (Qpr + Qsi)	29	17	12	-	17	-	-	-	127	9.5
200.0A	Residual Soil on Calcrete (Qpr/Qca)	-	-	N.P.	-	72	-	-	-	-	-
177.4A	Bulldog Shale (Klb)	51	25	26	-	7.6	-	-	-	110	17
49.2B	" "	-	-	-	-	-	-	-	-	89	11
139.1B	" "	-	-	-	-	-	-	-	-	82	29
144.0B	" "	62	24	38	16	4	16.3	92	26	-	-
Mt. Willoughby Borrow Pit	" "	-	-	-	-	-	-	-	-	103	12
161.5A	Playa Deposits (Qrl)	35	16	19	-	9.6	-	-	-	117	14.5

\* After transportation of unsealed samples to Adelaide

\*\* Compaction curves not available

## APPENDIX I -

ROBIN RISE TO MARLA BORE SECTION  
SUMMARY OF ANTICIPATED CONDITIONS

Chainage (km)	Predominant Material	Suggested Fill Material		Comments
		(a) Common Fill	(b) Selected Fill	
A159.5 to 162.0	Sand dunes on calcrete	Dune sand but protection against deflation may be necessary. Fill batters <del>35</del> <sup>50</sup> recommended.	Calcrete	Blasting may be necessary to remove calcrete.
A162.0	Small playa	Dune sand	Calcrete	Undercut upper powder gypsum layer. Fill should accept settlement and resist shear failure.
A162.2 to 169.0	Variable silty residual soil with patches of drift sand and ferricrete lag gravel.	Drift sand and residual soil	Long's Creek gravel but addition of binder may be necessary	Heavy ripping may be required in ferricrete patches.
A169.0 to 169.8	Long Creek Alluvium			See Jeune (Rept.Bk.72/192) for Bridge site investigation.
A169.8 to 176.0	Silty residual soil grading to calcified soil	Residual soil	Long's Creek gravel	
A176.0 to 178.0	Jasper silcrete on Bulldog Shale	Silcrete	Long's Creek gravel	Silcrete may require heavy ripping and blasting
A178.0 to B0.0	Silty residual soil on calcrete with scattered small playas	Residual soil	Calcrete or Mabel Creek gravel	Blasting may be necessary to remove calcrete
B0.0 to 1.0	Mabel Creek Alluvium			See Jeune (Rept.Bk.72/192) for Bridge site investigation

# APPENDIX I (contd.)

B1.0 to 8.7	Silty residual soil on calcrete	Residual soil	Calcrete or Mabel Creek gravel	Blasting may be necessary in calcrete
B8.7 to 17.5	Silty residual soil grading to calcified soil	Residual soil	Calcified soil may be suitable	
B17.5 to 21.0	Silty residual soil on clayey Bulldog Shale	Residual soil	Calcified soil may be suitable from next section or local alluvium if sufficient.	Bulldog Shale will need visual checking for fill suitability.
B21.0 to 35.0	Silty residual soil grading to calcified soil with jasper silcrete	Residual soil and silcrete	Possibly silcrete or local alluvium if sufficient	Blasting may be necessary in silcrete
B35.0 to 40.0	Residual and calcified soil on shallow Bulldog Shale. Jasper silcrete also present	Residual soil and silcrete	Calcified soil or possibly silcrete	
B40.0 to 45.0	Residual soil grading to calcified soil	Residual soil	Calcified soil	
B45.0 to 55.0	Residual soil and jasper silcrete on shallow Bulldog Shale	Residual soil and silcrete	Possibly silcrete	
B55.0 to 61.2	Residual soil grading to calcified soil	Residual soil	Possibly silcrete	
B61.2 to 70.0 approx.	Drift sand on residual and calcified soil	Drift sand and residual soil	Suitable selected fill will be difficult to obtain from now on. Calcified soil has been suggested as only alternative	



# APPENDIX I (contd.)

B70.0 approx. to 84.0	Drift sand with scattered dunes on older calcified alluvium	Drift and dune sand	Older alluvium	Protection of sand embankments against deflation may be necessary
B84.0 to 104.3	Residual soil grading to calcified soil with scattered playas after 95.0	Residual soil	Possibly calcified soil	Undercut powder gypsum in playa at 101.8 km
B104.3 to 126.0	Drift sand on residual soil grading to calcified soil	Drift sand and residual soil	Calcified soil	Playa at Cadney Park now bypassed by new alignment
B126.0 to 134.0	Residual soil grading to calcified soil	Residual soil	Calcified soil	
B134.0 to 145.0	Residual and calcified soil with outcrops of Bulldog Shale. Scattered jasper silcrete	Residual soil and silcrete	Silcrete and/or calcified soil	Bulldog shale rippable.
B145.0 to 153.3	Residual soil grading to calcified soil	Residual soil	Calcified soil	
B153.3 to 163.6	Drift sand on residual and calcified soil	Residual soil	Calcified soil	
B163.6 to 196.0	Dune and drift sand on residual and calcified soil	Sand and residual soil	Calcified soil	Protection of sand embankments against deflation may be necessary

# APPENDIX II

## ROBIN RISE TO MARLA BORE SECTION

<u>km</u>	<u>TRENCH LOGS</u>	<u>MAP UNIT</u>
<u>159A</u> Surface 0-60 cm	Gibber (Tsi) to 30 cm diameter, usually 3-4 cm Red-brown silty clay, occasional rounded Tsi pebbles to 5 cm. Basal 15 cm weakly iron cemented	Qpr
Grades to 60-100	Calcified red-brown silty clay occasional Tsi pebbles	Qcl
Grades to 100 +	Silcrete (Tsi) boulders to 60 cm (rare) in weak calcrete, minor Fe cement. Difficult to rip.	Qca
<u>160.4A</u> Surface 0-50+ cm	Dune sand Very loose, red-brown fine sand.	Qrd
<u>165.9A</u> Surface 0-70 cm	Gibber (Tsi, Tfe) to 5 cm, scattered on sand Red-brown silty clay, occasional Tsi (rounded) to 5 cm	Qpr
70-90	Calcified red-brown silty clay, occasional Tsi	Qcl
90-100+	Sandstone, weak, weathered, slightly calcified and silicified	Klc
<u>166.5A</u> Surface 0-60 cm	Gibber to 5 cm on sand Red-brown silty clay, occasional Tsi (rounded) to 5 cm	Qpr
60-100	Calcified red-brown silty clay, occasional Tsi pebbles	Qcl
100-130	Sandstone, weak, weathered, slightly calcified and silicified Rippable with heavy plant	Klc
<u>167.5A</u> Surface 0-75 cm	Ironstone gravel to 30 cm in sand Red-brown silty clay, occasional Tsi pebbles (rounded)	Qpr
75-100	Sandstone, weak, weathered, minor clay, paleosol remnants, minor gypsum crystals	Klc
100+	Weak weathered sandstone	Klc
<u>168.0A</u> Surface 0-60 cm 60-120	Ironstone lag gravel Red-brown silty clay, occasional Tsi pebbles Tsi and Tfe boulders to 15 cm in calcified soil matrix	Qpr Qcl
120+	Weak, weathered sandstone	Klc
<u>170.02A</u> Surface 0-30 cm 30-70 70-110+	Gibber, rounded, Tsi, 3 cm (red) Red-brown silty clay, very weak soil structure Calcified red-brown silty clay Calcified red-brown silty clay with weak ironpan	Qpr Qcl ?Klb

176.2A

Long's Creek overflow channel

0-50 cm	Dense dirty gravel	)	
50-70	Dense gravel, green clay matrix	)	Qpa
70-80+	Dense clean gravel to 2 cm	)	

177.4A

Surface	Top of ridge		
0-70 cm	Gibber to 20 cm usually subrounded (Tsi)		Qsi
	Red-brown silty clay, occasional rounded to sub angular Tsi to 5 cm		
70+	Strong calcified, silicified shale of Mabel Creek.		K1b
	Rippable with heavy machinery.		
	(Note: Calcrete adjacent, undermined by rabbits)		

180.0A

Surface	Gibber, 5 cm, rounded		
0-40 cm	Red-brown clayey silt("bulldust")		Qpr
	weak soil structure		
40-60	Calcified red-brown clayey silt		Qcl
Grading to 60-160	Weak porous calcrete with occasional rounded Tsi pebbles		Qca

181.9A

Surface	Top of ridge		
	Gibber, mainly red. Rare blocks white Tsi in situ nearby (up to 60 cm)		
0-25 cm	Red-brown silty clay. Moderate soil development	)	
25-85	Orange-brown compact clayey silt with 20-30% sub angular silcrete (Tsi) gravel	)	Qpr
85-100+	Calcified orange-brown compact clayey silt with 20-30% subangular (Tsi) gravel		Qcl

184.9A

Surface	Top of ridge		
0-45 cm	Gibber to 10 cm, mainly red silcrete		Qpr
45-110+	Red-brown clayey silt		Qcl
	Calcified red-brown clayey silt, grading to very weak calcrete		Qca
	(Crumbly but probably compacts satisfactorily with water)		

190.0A

Surface	Gibber (round) 3-4 cm		
0-30 cm	Red-brown clayey silt, rare pebbles		Qpr
30+	Strong sheet calcrete		Qca

200.0A

Surface	Rounded gibber to 5 cm		
0-25 cm	Red-brown clayey silt, occasional pebbles		Qpr
Grading to 25-105+	Laminated weak calcrete and ironpan in red-brown clayey silt matrix		Qca
105	Limit of ripping (LW16)		

210.6A

Surface	Gibber to 20 cm (Tsi) red and grey angular sub-rounded		
0-20 cm	Dark brown nutty clay (soil horizon)	)	
20-80	Medium dense orange-brown clayey silt with 20-30% subangular to sub rounded Tsi to 20 cm usually 5cm)	)	Qpr
80+	Silicified, calcified red-white mottled clay (shale)		K1b
	Too strong to rip with LW16 rippable heavy plant		

<u>10.0B</u>		
Surface	Crabhole with gibber to 10 cm	
0-60 cm	Red-brown silty clay, rare (Tsi) to 1 cm	Qpr
60-110+	Interbedded weak calcrete and ironpan with occasional Tsi rounded to 10 cm	Qc1
<u>13.4B</u>		
Surface	Stream bed	
0-40 cm	Crabholes with gibber to 10 cm	
	Red-brown silty clay, occasional Tsi to 5 cm, sub rounded	Qpr
40-90+	Weak calcrete, ironpan, laminated	Qc1
<u>18.5B</u>		
Surface	Bed of stream	
0-40 cm	Sand	
40-100+	Red-brown silt, trace gravel to 1 cm	Qra
	Dense silty gravel to 5 cm	Qra-Qpa
<u>19.6B</u>		
0-100 cm	Dense silty gravel, (narrow stream bed)	Qpa
<u>20.00B</u>		
Surface	Angular shale fragments, part silicified, to 1-2 cm	
0-80+ cm	Red-brown clayey silt with angular shale fragments	?Qpr ?K1b
<u>20.60B</u>		
Surface	Angular shale chips 1-5%, white, purple	Qpr
0-100+ cm	Medium strong white and purple siltstone (shale)	K1b
	Rippable LW16	
<u>24.1B</u>		
0-40 cm	Bed of stream	
40-120	Loose red-brown gravelly sand, mobile	Qra
	Dense orange brown clayey silt with 30% gravel to 2 cm	Qpa
120+	Gravel, 10 cm, in very stiff silty clay matrix	Qpa
<u>27.85B</u>		
Surface	Gibber to 5 cm	
0-40 cm	Red-brown silty clay	Qpr
40-50+	Weak ironpan?	
<u>30.00B</u>		
Surface	Gibber on crabhole	
0-50 cm	Red-brown silty clay rare gravel (Tsi, Qca) to 5 cm. No lime.	Qpr
50-75+	Tsi (round) to 5 cm in matrix of red sandy silt, calcified, ironpan	Qc1
<u>35.0B</u>		
Surface	Gibber, round, 5 cm on crabholes	
0-50 cm	Red-brown silty clay rare pebbles (round) to 2 cm	Qpr
50-110	Weak laminated calcrete and ironpan	Qc1
<u>40.0B</u>		
Surface	Gibber (round) to 5 cm	
0-30 cm	Orange-brown silt	Qpr
30-50+	Weak ironpan, minor calcrete	

<u>45.4B</u>		
Surface	Flat gibber rounded, 2-3 cm	
0-35 cm	Red-brown clayey silt	Qpr
35-50+	Weak laminated calcrete/ironpan,	Qc1
<u>53.95B</u>		
	Bed of major stream	
	Mixture of boulders to 20 cm, powdery calcrete and green silty clay	Qpr
<u>60.0B</u>		
Surface	Rounded Tsi, white and red 2-3 cm	Qpr
0-30 cm	Orange-brown compact clayey silt	
30-50+	Calcareous cemented orange-brown clayey silt with iron cementing	
<u>69.95B</u>		
Surface	Sandy silt with scattered Tsi and Tfe to 2 cm	
0-40 cm	Orange-brown fine sandy silt with occasional rounded pebbles to 1 cm	Qpr
40-60+	Platy calcrete/ironpan mixture, medium strong. Will compact well if wet	Qc1
<u>87.0B</u>		
Surface	Drift sand with ironstone pan gravel in crabhole terrain	
0-50 cm	Red-brown silty clay	Qpr
50-60+	Iron and lime cemented shale	Qc1/K1b
<u>97.0B</u>		
	Surface crabhole	
0-80 cm	Red-brown silty clay	Qpr
80-100+	Calcified and ferruginised shale	Qc1/K1b
<u>100.0B</u>		
Surface	Rounded gibber to 3 cm in silty matrix	
0-10 cm	Fine red-brown clayey silt	Qpr
10-80+	Calcified and iron cemented silt and silty clay	Qc1
<u>106.0B</u>		
Surface	Drift with scattered rounded gibber to 3 cm	Qrs
0-20 cm	Red-brown clayey silt	Qpr
20-40+	Calcified and ferruginised shale	Qc1/K1b
<u>110.0B</u>		
Surface	Drift, scattered gibber	
0-25 cm	Red-brown fine sandy silt, trace gravel to 1 cm	Qrs
25-35+	Calcified and ferruginised shale	Qc1
<u>114.5B</u>		
0-20 cm	Cadney Park Claypan	
20-30	Loose grey-brown clayey silt	Qr1
30-60	Loose powdery limey silt	
60+	Stiff greenish grey blocky clay, high plasticity	
	Strong ironpan, too strong to rip (LW16)	
<u>128.95B</u>		
Surface	Bed of watercourse	
0-70+ cm	Drift	
	Dense well rounded Tsi gravel to 3 cm in silty sand matrix	Qpa

<u>130.0B</u>		
Surface	Gibber (10 cm) and crabholes	
0-45 cm	Red-brown silty clay	Qpr
45-50+	Strong to medium strong iron cemented shale	K1b
<u>140.0B</u>		
Surface	Gibber to 10 cm	
0-40 cm	Red-brown silty clay, rare pebbles	Qpr
40-100	Calcified silty clay (shale)	K1b
<u>144.0B</u>	Saddle	
Surface	Coarse rounded to subangular Tsi gibber and yellow siltstone chips. Minor crabhole development	
0-80 cm	Red-brown silty clay, scattered Tsi pebbles	Qpr
80-120+	Pinkish white weak siltstone	K1b
<u>150.0B</u>		
Surface	Rounded gibber to 5 cm on crabhole	
0-60 cm	Red-brown silty clay with weak calcrete and ironpan	Qpr
60-70	Weak calcrete/ironpan	Qc1
<u>160.0B</u>		
Surface	Drift sand	
0-100 cm	Drift sand with occasional lenses well rounded gravel to 5 cm	Qrs
100+	Weak ironpan in silt matrix	Qpr
<u>169.2</u>		
Surface	White angular silicified shale chips in silt matrix	
0-20 cm	Angular white silicified shale chips in silt matrix	Qpr
20+	Strong calcrete with shale chips. Too strong for LW16 but could be ripped by D9	Qca
<u>170.0B</u>		
0-80 cm	Dune sand	Qrd
80+	Strong calcrete with minor yellow silicified shale chips	Qca
<u>174.2B</u>	Floodway	
Surface	Sand	Qrs
0-100 cm	Silty sand	Qpr
100+	Calcified silty sand	Qc1
<u>180.0B</u>		
Surface	Dune sand	Qrd
0-50 cm	Red-brown fine sandy silt	Qrs
50-70+	Strong calcrete with minor silicified shale	Qca/K1b
<u>190.0B</u>		
Surface	Sand with scattered ironstone (rounded) to 1 cm	Qrs
0-65 cm	Red-brown silty fine sand with rounded gravel	Qpr
65-95+	Weak calcrete and ironpan	Qc1
<u>192.8B</u>		
Surface	Angular white silicified shale chips in silt matrix	Qpr
0-60 cm	Calcified silt with 20-30% angular white silicified shale chips	Qc1
60-80+	Strong calcrete with shale chips	Qca/K1b

## APPENDIX III

### ROBIN RISE TO MARLA BORE SECTION

#### REPORT ON LABORATORY TESTS ON EMBANKMENT FILL MATERIALS

by

Mr. S. Ronan  
Engineering and Water Supply Department

#### GENERAL

A request was received on 4th September 1974, from the Department of Mines Engineering Geology Section to run a series of laboratory tests on embankment fill materials from locations between chanages B. 49.2 km and B. 164.7 km.

#### SAMPLE DESCRIPTIONS AND LABORATORY TESTS

The descriptions of the samples supplied by the Department of Mines are given below in Table 1.

TABLE 1

<u>Sample No.</u>	<u>Description</u>
B. 49.2 km (K1b)	Yellow-brown KAOLINITE; blocky, partly altered.
B. 100.0 km (Qpr)	Red-brown LATERITE: VERY FRIABLE
B. 130.0 km (Qpr)	Red-brown SILTY CLAY; very friable
B. 139.0 km (K1b)	White KAOLINITE: blocky; hard
B. 160.0 km (Qrs/Qpr)	Red-brown SILTY (CLAYEY) SAND
B. 164.7 km (Qrd)	Red-brown FINE SAND
MWBP.1	White KAOLINITE; blocky, hard

The following laboratory tests were requested:-

Sieve analysis  
Atterberg limits, where applicable  
Modified compaction tests  
Estimated Californian Bearing ratio (CBR)

The test results, together with a description of the test procedures are appended. The South Australian Highways Department method of estimating CBR was used to obtain the CBR values.

#### COMPACTION RECOMMENDATIONS

KAOLINITE: (BULLDOG SHALE)	This should be treated as a weak rock. It should be compacted with a vibrating roller so as to achieve the best packing of this granular material. It only requires enough water to weaken the sharp edges and to aid the packing together of the blocky pieces. Nothing useful will be achieved by trying to grind it into a powder.
LATERITE AND SAND: (RESIDUAL SOIL)	These materials have very flat compaction curves and once a minimum amount of water has been added, little increase in density will be achieved by adding more.

SILTY CLAY:  
(RESIDUAL SOIL)

Probably the best form of compact will be a sheepsfoot roller if a heavy standard of compaction is required. The heavier the roller the lower the optimum moisture content.

SILTY (CLAYEY) SAND:  
(DRIFT SAND OVER  
RESIDUAL SOIL)

It is not certain if this should be treated as a sand or a clay as it has a predominantly sandy grading but appears to have a clays compaction curve. A trial will show how it behaves.

TRIAL FILLS:

These should be run on the various materials to determine the most effective compactive method.

WATER:

Either salt or fresh water can be used for the compaction of these soils. Comparative tests have been run in the laboratory by compacting samples B100, B130, B160 and B164.7 firstly with distilled water and then with salt water. These samples were then "surface-washed" with distilled water (to simulate flooding of an embankment). Samples B130 and B160 showed little loosening of their surfaces. Sample B164 showed some loosening of the surface with the salt water-compacted material being slightly more effected than that compacted with fresh water. Sample B100 had a dramatic surface loosening, this being more pronounced where the sample had been compacted with salt water. It is therefore recommended that this latter material should be faced when it is used in a location which is liable to be flooded or subject to water erosion.

S. RONAN

DESIGNING ENGINEER  
SOILS & FOUNDATIONS

3.10.74



## LABORATORY TEST PROCEDURES

Sieve Analyses were done on all samples as received. Figs. 1-7.

### 1. BULLDOG SHALE SAMPLES (MWB1, B49.200 km & B139.100 km)

- 1.1 These samples were crushed to pass a 19 mm (3/4") sieve and then sieve analyses were done. The results are shown on Figs. 20, 26 and 34.
- 1.2 Representative samples of these were sieved on a 2.36 mm (BS7) sieve, the portions retained being washed and oven dried. The oven dried samples were supported on sieves under a fine garden sprinkler and the water turned on. Moisture contents were taken at ten minute intervals for one hour. A moisture content versus time graph was drawn for each sample. The results are shown on Fig. 36.
- 1.3 The crushed Kaolinite samples MWB1 & B49.200 km (passing 19 mm sieve) were wetted to the moisture content corresponding to ten minutes sprinkling, and sample B139.00 km wetted to the moisture content corresponding to twenty minutes sprinkling. These samples were then compacted using modified compactive effort (as A89 Test 12A), then the whole compacted sample was oven dried to obtain moisture content and dry density.
- 1.4 Sieve analyses were done on the three compacted samples to ascertain breakdown during compaction. The grading curves are also shown on Figs. 20, 26 and 34.

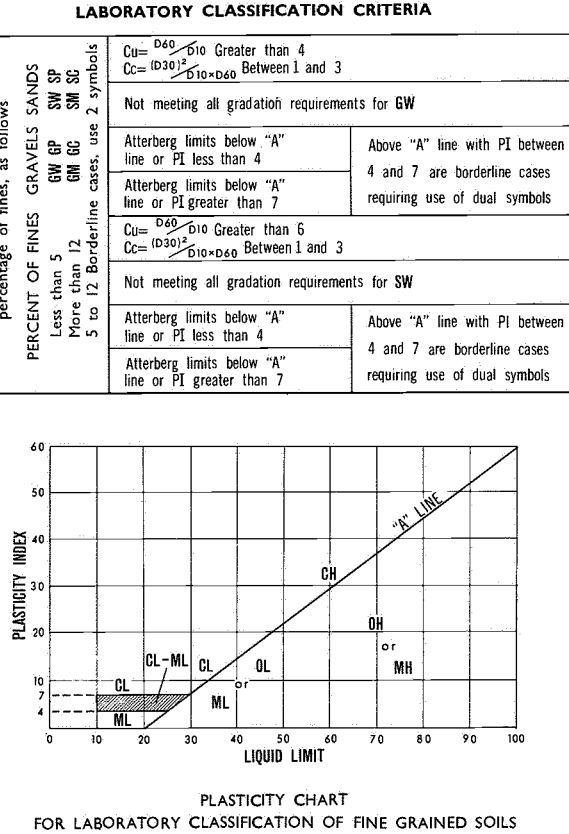
### 2. OTHER SAMPLES (B100.00 km, B130.00 km, B160.00 km & B164.70 km)

- 2.1 Modified compaction tests were done on these four samples, using the saline water provided; the tests starting at the moisture contents of the samples as received. Compaction curves are shown on Figs. 23, 25, 29 and 31.
- 2.2 Atterberg limits were done on the portion of sample passing a 425 micron sieve from samples B130.00 km & B160.00 km. Samples B100.00 km & B164.70 km were found to be non-plastic. Estimated C.B.R.'s were calculated for these four samples. Atterberg limits results and estimated C.B.R. values are shown on the respective compaction test sheets.

**DEPARTMENT OF MINES - SOUTH AUSTRALIA**  
**ENGINEERING CLASSIFICATION OF SOILS**  
The Unified Soil Classification System

COARSE-GRAINED SOILS More than 50% of material is larger than No. 20 B.S. sieve size		FIELD INVESTIGATION PROCEDURES Excluding particles larger than 7.5cm and basing fractions on estimated weights						GROUP SYMBOL	GROUP NAME and typical materials	GRAIN SIZE CURVES to be used to identify soil fractions	LABORATORY CLASSIFICATION CRITERIA	
		GRAVELS More than 50% of the coarse fraction is larger than 2mm. (retained on B.S.7 sieve)	CLEAN GRAVELS Little or no fines	Wide range in grain sizes, and substantial amounts of all intermediate particle sizes				GW	GRAVEL, well graded; gravel sand mixtures, little or no fines		$C_u = \frac{D_{60}}{D_{10}}$ Greater than 4 $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ Between 1 and 3	
FINE-GRAINED SOILS More than 50% of material is smaller than No. 200 B.S. sieve size	DIRTY GRAVELS Appreciable amount of fines		Predominantly one size or a range of sizes, with some intermediate sizes missing				GP	GRAVEL, poorly graded; gravel sand mixtures, little or no fines	Not meeting all gradation requirements for GW	Atterberg limits below "A" line or PI less than 4 Above "A" line with PI between 4 and 7 are borderline cases		
		Non-plastic fines—for identification see ML below				GM	GRAVEL, excess silty fines; poorly graded gravel-sand-silt mixtures	Atterberg limits below "A" line or PI greater than 7 requiring use of dual symbols				
	SANDS More than 50% of the coarse fraction is smaller than 2mm. (passing B.S.7 sieve)	CLEAN SANDS Little or no fines	Wide range in grain sizes, and substantial amounts of all intermediate particle sizes				SW		SAND, well graded; well graded sands, gravelly sands, little or no fines	$C_u = \frac{D_{60}}{D_{10}}$ Greater than 6 $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ Between 1 and 3		
			Predominantly one size or a range of sizes, with some intermediate sizes missing				SP	SAND, poorly graded; poorly graded sands, gravelly sands, little or no fines	Not meeting all gradation requirements for SW			
		DIRTY SANDS Appreciable amount of fines	Non-plastic fines—for identification see ML below				SM	SAND, excess silty fines; poorly graded sand-silt mixtures		Atterberg limits below "A" line or PI less than 4 Above "A" line with PI between 4 and 7 are borderline cases		
			Plastic fines—for identification see CL below				SC	SAND, excess clayey fines; poorly graded sand-clay mixtures	Atterberg limits below "A" line or PI greater than 7 requiring use of dual symbols			
FIELD INVESTIGATION PROCEDURES on fraction smaller than 0.4mm. (passing B.S.36 sieve)								GROUP SYMBOL		GROUP NAME and typical materials	<div>PLASTICITY INDEX</div> <div></div> <div>FOR LABORATORY CLASSIFICATION OF FINE GRAINED SOILS</div>	
SILTS AND CLAYS Liquid limit less than 50	SOIL CAST (soil wet)	SOIL THREAD	SHINE	DILATANCY	ODOUR	DRY STRENGTH	ML	SILT SOIL, low plasticity; inorganic silts and very fine silty or clayey sands, rock flour				
	Forms fragile cast Cracks form when kneaded while moist	Thick crumbly thread; easily broken	None to very dull	Distinct	Not significant	None to slight	CL	CLAY SOIL, low plasticity; inorganic clays of low to medium plasticity, gravelly clay, sand, clays, silty clays, lean clays				
SILTS AND CLAYS Liquid limit more than 50	Cast maybe handled freely without breaking Can be kneaded moist without cracking Material adheres to the hand	Thread can be pointed as fine as a lead pencil but is fragile	Moderate	None to slight	Not significant	Moderate	OL	ORGANIC SOIL, low plasticity; organic silts and silt clays of low plasticity				
	Cast fragile to cohesive material will adhere somewhat to the hand	Soft, weak thread	None to very dull	Slight to distinct	Decayed organic matter	Low	MH	SILT SOIL, high plasticity; inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts				
	Moderately plastic and cohesive Material adheres somewhat to the hand	Weak to medium thread May be crumbly	Dull	None to slight	Not significant	Moderate Powdered soil feels floury	CH	CLAY SOIL, high plasticity; inorganic clays of high plasticity, fat clays				
	Very plastic and cohesive Material very sticky to the hand Greasy to touch	Very tough thread, can be rolled to a pin point	Very glossy	None	Strong earthy	High to very high Cannot be powdered by finger pressure	OH	ORGANIC SOIL, high plasticity; organic clays of medium to high plasticity				
Plastic and cohesive Feels slightly spongy Greasy to touch	Weak to medium thread Often soft and fibrous	Moderate to very glossy	None	Decayed organic matter	Moderate to high Powdered soil may be fibrous							
	Readily identified by colour, odour, spongy feel and frequently by fibrous texture						PI	PEATY SOIL; Peat and other highly organic soils				
NOTE: BOUNDARY CLASSIFICATIONS: Soil possessing characteristics of two groups are shown as a combination of two group symbols, eg. GW-GC, well graded gravel with clay binder.												
Based on "The Unified Soil Classification System" United States Department of the Interior, Bureau of Reclamation "Earth Manual" First Edition, Denver COLORADO 1960.												

70 - 641



NOTE: BOUNDARY CLASSIFICATIONS: Soil possessing characteristics of two groups are shown as a combination of two group symbols, eg. GW-GC, well graded gravel with clay binder.

Based on "The Unified Soil Classification System" United States Department of the Interior, Bureau of Reclamation "Earth Manual" First Edition, Denver COLORADO 1960.

**DEPARTMENT OF MINES - SOUTH AUSTRALIA**  
**ENGINEERING CLASSIFICATION OF SOILS**  
The Unified Soil Classification System

		FIELD INVESTIGATION PROCEDURES						GROUP SYMBOL	GROUP NAME and typical materials	LABORATORY CLASSIFICATION CRITERIA		
		Excluding particles larger than 7.5cm and basing fractions on estimated weights										
COARSE-GRAINED SOILS More than 50% of material is larger than No. 200 B.S. sieve size	GRAVELS More than 50% of the coarse fraction is larger than 2mm. (retained on B.S.7 sieve)	CLEAN GRAVELS Little or no fines	Wide range in grain sizes, and substantial amounts of all intermediate particle sizes				GW	GRAVEL, well graded; gravel sand mixtures, little or no fines	Cu = $\frac{D_{60}}{D_{30}} \geq \frac{D_{10}}{D_{60}}$ Greater than 4 Cc = $\frac{D_{30}}{D_{10}} \geq \frac{D_{10}}{D_{60}}$ Between 1 and 3			
		DIRTY GRAVELS Appreciable amount of fines	Predominantly one size or a range of sizes, with some intermediate sizes missing				GP	GRAVEL, poorly graded; gravel sand mixtures, little or no fines	Not meeting all gradation requirements for GW			
			Non-plastic fines—for identification see ML below				GM	GRAVEL, excess silty fines; poorly graded gravel-sand-silt mixtures	Atterberg limits below "A" line or PI less than 4 Above "A" line with PI between 4 and 7 are borderline cases			
			Plastic fines—for identification see CL below				GC	GRAVEL, excess clayey fines; poorly graded gravel-sand-clay mixtures	Atterberg limits below "A" line or PI greater than 7 requiring use of dual symbols			
	SANDS More than 50% of the coarse fraction is smaller than 2mm. (passing B.S.7 sieve)		CLEAN SANDS Little or no fines	Wide range in grain sizes, and substantial amounts of all intermediate particle sizes				SW	SAND, well graded; well graded sands, gravelly sands, little or no fines	Cu = $\frac{D_{60}}{D_{30}} \geq \frac{D_{10}}{D_{60}}$ Greater than 6 Cc = $\frac{D_{30}}{D_{10}} \geq \frac{D_{10}}{D_{60}}$ Between 1 and 3		
		DIRTY SANDS Appreciable amount of fines	Predominantly one size or a range of sizes, with some intermediate sizes missing				SP	SAND, poorly graded; poorly graded sands, gravelly sands, little or no fines	Not meeting all gradation requirements for SW			
	Non-plastic fines—for identification see ML below				SM	SAND, excess silty fines; poorly graded sand-silt mixtures	Atterberg limits below "A" line or PI less than 4 Above "A" line with PI between 4 and 7 are borderline cases					
		Plastic fines—for identification see CL below				SC	SAND, excess clayey fines; poorly graded sand-clay mixtures	Atterberg limits below "A" line or PI greater than 7 requiring use of dual symbols				
FIELD INVESTIGATION PROCEDURES on fraction smaller than 0.4mm. (passing B.S. 36 sieve)								GROUP SYMBOL	GROUP NAME and typical materials			
FINE-GRAINED SOILS More than 50% of material is smaller than No. 200 B.S. sieve size	SILTS AND CLAYS Liquid limit less than 50	SOIL CAST (soil wet)	SOIL THREAD	SHINE	DILATANCY	ODOUR	DRY STRENGTH			ML	SILT SOIL, low plasticity; inorganic silts and very fine silty or clayey sands, rock flour	
		Forms fragile cast Cracks form when kneaded while moist	Thick crumbly thread; easily broken	None to very dull	Distinct	Not significant	None to slight	CL	CLAY SOIL, low plasticity; inorganic clays of low to medium plasticity, gravelly clay, sand, clays, silty clays, lean clays			
		Cast maybe handled freely without breaking Can be kneaded moist without cracking Material adheres to the hand	Thread can be pointed as fine as a lead pencil but is fragile	Moderate	None to slight	Not significant	Moderate	OL	ORGANIC SOIL, low plasticity; organic silts and silt clays of low plasticity			
		Cast fragile to cohesive material will adhere somewhat to the hand	Soft, weak thread	None to very dull	Slight to distinct	Decayed organic matter	Low	MH	SILT SOIL, high plasticity; inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts			
	SILTS AND CLAYS Liquid limit more than 50	Moderately plastic and cohesive Material adheres somewhat to the hand	Weak to medium thread May be crumbly	Dull	None to slight	Not significant	Moderate Powdered soil feels floury	CH	CLAY SOIL, high plasticity; inorganic clays of high plasticity, fat clays			
		Very plastic and cohesive Material very sticky to the hand Greasy to touch	Very tough thread, can be rolled to a pin point	Very glossy	None	Strong earthy	High to very high Cannot be powdered by finger pressure	OH	ORGANIC SOIL, high plasticity; organic clays of medium to high plasticity			
		Plastic and cohesive Feels slightly spongy Greasy to touch	Weak to medium thread Often soft and fibrous	Moderate to very glossy	None	Decayed organic matter	Moderate to high Powdered soil may be fibrous	Pt	PEATY SOIL; Peat and other highly organic soils			
		Readily identified by colour, odour, spongy feel and frequently by fibrous texture										
	NOTE: BOUNDARY CLASSIFICATIONS: Soil possessing characteristics of two groups are shown as a combination of two group symbols, eg. GW-GC, well graded gravel with clay binder.											

Coarse-grained soil classified on basis of percentage of fines, as follows

PERCENT OF FINES GRAVELS SANDS

Less than 5 GW GP SW SP

More than 12 GM GC SM SC

5 to 12 Borderline cases, use 2 symbols

GRAIN SIZE CURVES to be used to identify soil fractions

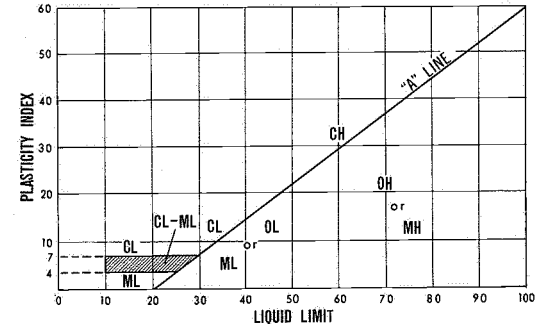
Based on "The Unified Soil Classification System" United States Department of the Interior, Bureau of Reclamation "Earth Manual" First Edition, Denver COLORADO 1960.

70 - 641

Coarse-grained soil classified on basis of percentage of fines, as follows

PERCENT OF FINES GRAVELS SANDS  
Less than 5 GW GP  
More than 12 GM GC  
5 to 12 Borderline cases, use 2 symbols

GRAIN SIZE CURVES to be used to identify soil fractions



PLASTICITY CHART  
FOR LABORATORY CLASSIFICATION OF FINE GRAINED SOILS

NOTE: BOUNDARY CLASSIFICATIONS: Soil possessing characteristics of two groups are shown as a combination of two group symbols, eg. GW-GC, well graded gravel with clay binder.

Based on "The Unified Soil Classification System" United States Department of the Interior,  
Bureau of Reclamation "Earth Manual" First Edition, Denver COLORADO 1960.

LIST OF DEPARTMENT OF MINES ENGINEERING  
SECTION REPORTS COMPLETED BY JULY '74

<u>ENGINEERING GEOLOGY</u>		D.M. REPORT BK.
Jeune R.F. - Tarcoola/Robin Rise Section.		72/157
Jeune R.F. - Long Creek/Mabel Creek Bridge Crossings		72/192
Selby J & Jeune R.F. - Robin Rise/Marla Bore Section		This report
<u>HYDROGEOLOGY</u>		
Carosone F. - Tarcoola/Mabel Creek Section Completion Report.		72/156
Smith P.C. - Robin Rise/Wintinna Hill Section Progress Report N° 5		74/12

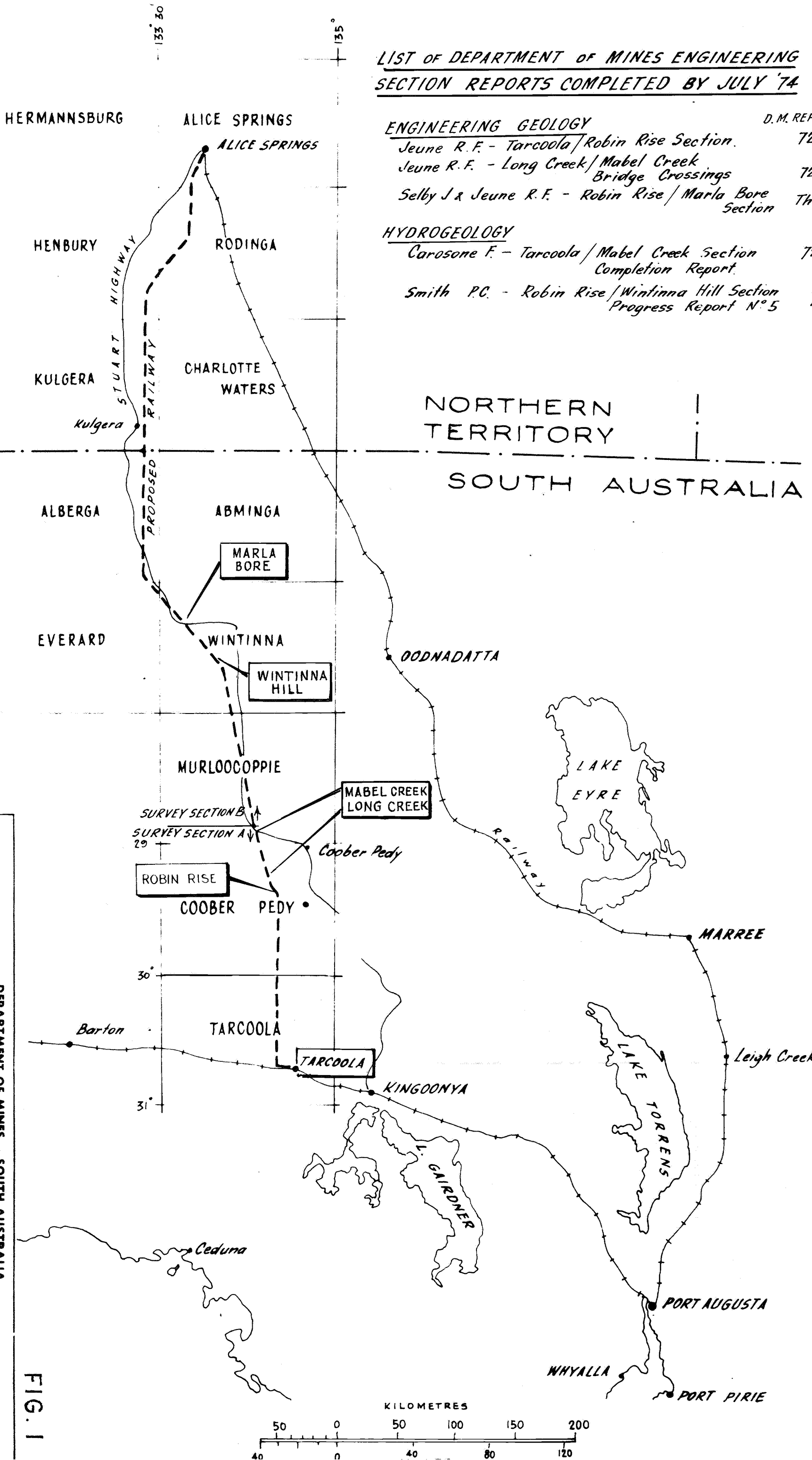
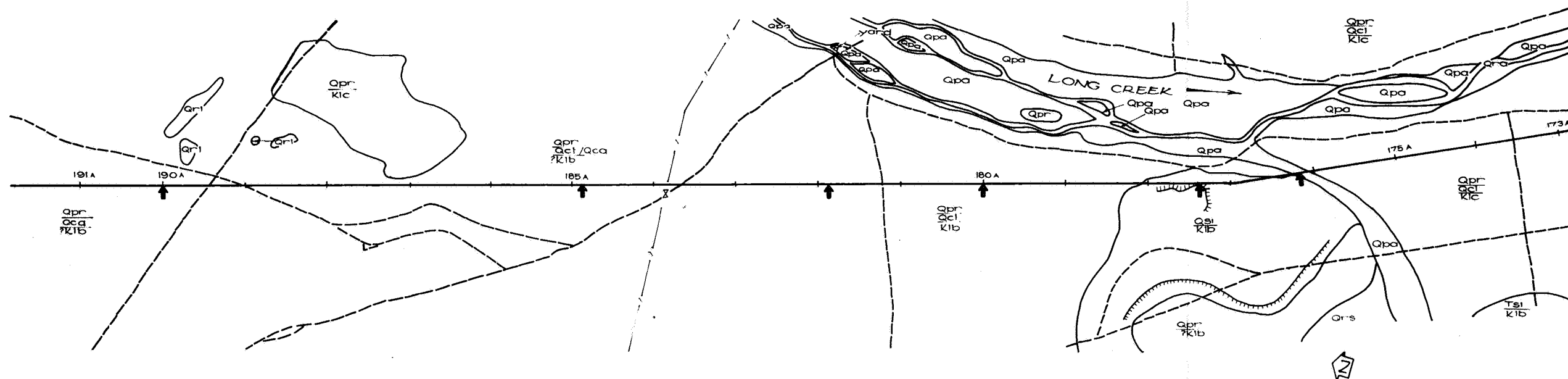


FIG. 1

DEPARTMENT OF MINES - SOUTH AUSTRALIA  
TARCOOLA - ALICE SPRINGS RAILWAY  
LOCALITY MAP  
Date 5<sup>th</sup> August 74  
Drg. No. 72-145 b  
9942+81



SCALE IN KILOMETRES

SCALE IN MILES

FIG. 2

**TARCOOLA - ALICE SPRINGS RAILWAY**  
156 KILOMETRE TO 191 KILOMETRE SECTION

ENGINEERING GEOLOGY SECTION	GEOLOGIST	Completed <i>R.F.</i>	Scale: 1" = 2400'(OR)(G) Date: 11 July 197
Director of Mines	SUP GEOLOGIST	Drn. <i>R.B.</i> Ckd. <i>A.F.</i>	Drp. No. <i>74-514</i> <i>B2</i>

Qrd	DUNE SAND sand and silty sand forming dunes
-----	---

Q5:      40% NIGRA    60% WHITE    Red, pink and white \* sugar?    100%    100%

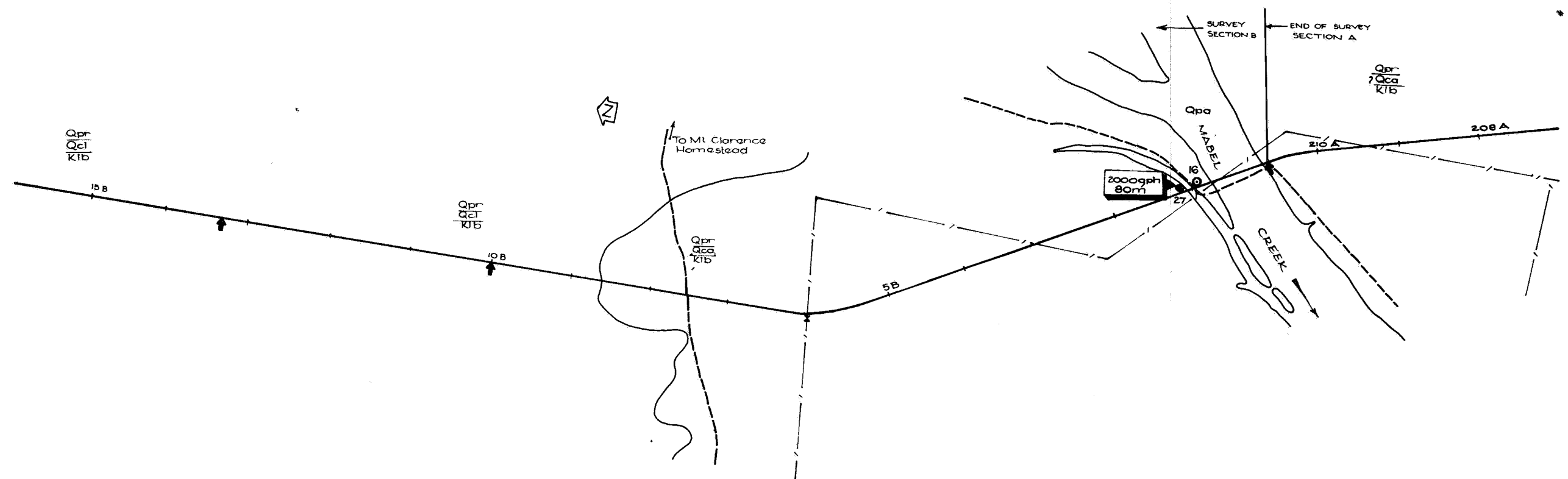
KL ADNA TWB INFORMATION Weak white guinea pig  
with agouti spots

```

    unsuccessful

```

 Gate



SCALE IN KILOMETRES


SCALE IN MILES

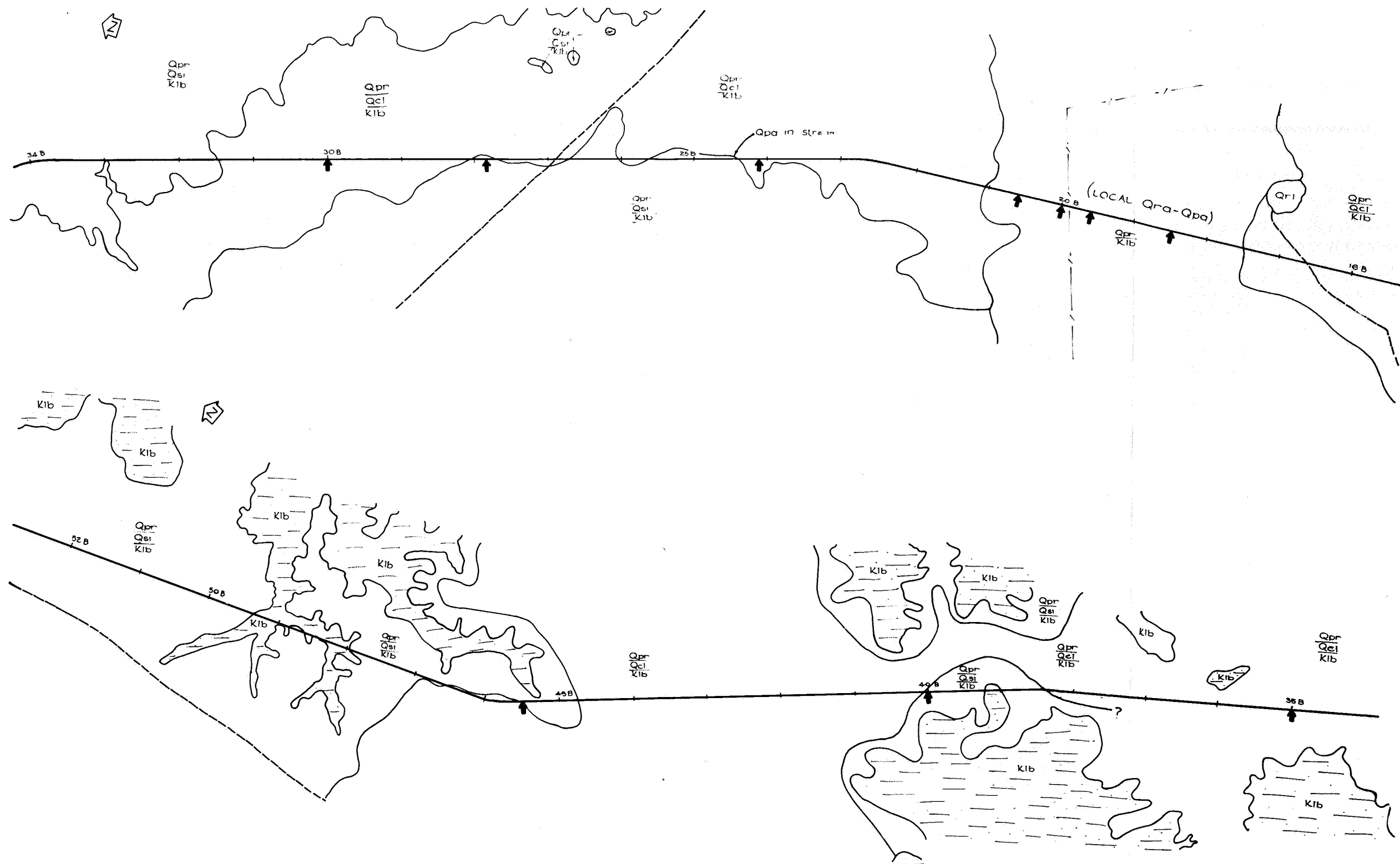
**FIG. 3**

**TARCOOLA - ALICE SPRINGS RAILWAY**  
**190AKILOMETRE TO 15B KILOMETRE SECTION**

ENGINEERING GEOLOGY SECTION	GEOLOGIST	Completed R.F./	Date: 11-24-68 (HRS)
Director of Mines	SUP. GEOLOGIST	Dir. R.B. Chd. A.F.	Date: 13th July 1974 Org. No. 74-515 Pg

- |     |  |
|-----|--|
| Tfe | FERRICRETE Strong sheet ferrocrete ("ironstone") and coarse lag gravel resulting from disintegration of the sheets |
| Tsl | OLDER SILCRETE Strong sheet silcrete and coarse lag gravel resulting from disintegration of the sheets.            |
| Klb | BULLDOG SHALE Buff white and red mottled kaolinitic clay, in part weakly cemented by secondary silicification      |
| Klc | GADNA - OWIE FORMATION Weak white quartz sandstone with kaolinitic matrix  |

- 
- X 84      Soil sample location  
      Soil Investigation Trench  
 205      Surveyed rail centre line with km peg  
 ●      Successful  
 ○      Unsuccessful  
 — / —      Fences  
 — — — — —      Dry weather tracks  
 — — — — —      Gate  
 — — — — —      Windmill  
 ~~~~~      Geological boundary  
 — — — — —      Scarp

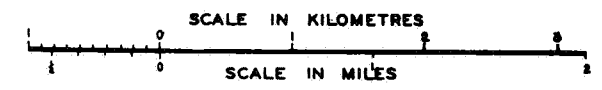


# LEGEND (NOT IN STRATIGRAPHIC ORDER)

- Qca** CALCRETE: Nodular and strong sheet calcretes
- Qol** CALCIFIED SOIL: Zone of subsol lime accumulation
- Qpa** OLDER ALLUVIUM: Dirty gravel and silty sand deposits of former river channels
- Qpr** RESIDUAL SOIL: Red-brown silty clay with a skin of lag gravel. Forms "crab hole country". Qpa is gravelly slope wash material.
- Qrd** DUNE SAND: Sand and silty sand forming dunes
- Qrs** DRIFT SAND: Sand and silty sand forming drift sheets. Qrs is thin drift generally <30 cms. Qrs is thick >30 cms.
- Qri** PLAYA DEPOSITS: Saline, gypsaceous, clays and silts of lakes, claypans and swamps, usually dry
- Qra** RECENT ALLUVIUM: Unconsolidated, generally clean, sands and gravels of the present stream courses
- Qrg** GYPSEOUS DUNES: Powdery gypsum and gypsum/sand mixtures forming dunes
- Qsi** YOUNGER SILCRETE: Red, pink and white "Jasper" silcrete forming ridge cappings and scarps

- Tfe** FERRICRETE: Strong sheet ferricrete ("ironstone") and coarse lag gravel resulting from disintegration of the sheets
- Tsi** OLDER SILCRETE: Strong sheet silcrete and coarse lag gravel resulting from disintegration of the sheets
- Kib** BULLDOG SHALE: Shiny white and red mottled kaolinitic clay, in part weakly cemented by secondary silicification
- Kic** GADNA-OWIE FORMATION: Weak white quartz sandstone with kaolinitic matrix

- X 04** Soil sample location
- Soil investigation Trench
- 200** Surveyed rail centre line with km peg
- HYDROGEOLOGICAL INVESTIGATION BORES: Successful
- Unsuccessful
- 2000** Yield in gallons/hour
- 3000** Pump depth in metres
- Geological boundary
- Scarp
- Dry weather tracks
- Fence
- Gate
- Windmill



NB: THIS PLAN WAS PRODUCED FROM AERIAL PHOTOS WITHOUT THE USE OF GROUND CONTROL

| DEPARTMENT OF MINES - SOUTH AUSTRALIA  |               |                        |                        |
|----------------------------------------|---------------|------------------------|------------------------|
| TARCOOLA - ALICE SPRINGS RAILWAY       |               |                        |                        |
| 16B KILOMETRE TO 52B KILOMETRE SECTION |               |                        |                        |
| ENGINEERING GEOLOGY SECTION            | GEOLOGIST     | Compiled <b>R.F.J.</b> | Scale: 1:24000 (ORIG)  |
|                                        |               | Drawn <b>R.B.</b>      | Date: 12 July 1974     |
| Director of Mines                      | SUP GEOLOGIST | Checked <b>A.F.</b>    | Drp. No. <b>74-516</b> |
|                                        |               |                        | <b>Ba</b>              |

FIG. 4





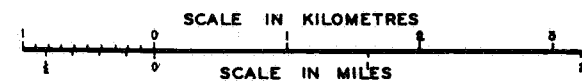
- Qos** CALCRETE: Nodular and strong sheet calcrete
- Qol** CALCIFIED SOIL: Zone of subsoil lime accumulation
- Qpa** OLDER ALLUVIUM: Dirty gravel and silty sand deposits of former river channels
- Qpr** RESIDUAL SOIL: Red-brown silty clay with a skin of lag gravel. Forms "crab hole country". Qpa is gravelly slope wash material.
- Qrd** DUNE SAND: Sand and silty sand forming dunes

- Qrs** DRIFT SAND: Sand and silty sand forming drift sheets. Qrs is thin drift generally <50 cms. Qrs is thick >50 cms.
- Qrl** PLAYA DEPOSITS: Saline, gypsaceous, clays and silts of lakes, claypans and swamps, usually dry
- Qra** RECENT ALLUVIUM: Unconsolidated, generally clean, sands and gravels of the present stream courses
- Qrg** GYPSEOUS DUNES: Powdery gypsum and gypsum/sand mixtures forming dunes
- Qsi** YOUNGER SILCRETE: Red, pink and white "jasper" silcrete forming ridges, cappings and scarps

- Tfs** FERRICRETE: Strong sheet ferruginous ("ironstone") and coarse lag gravel resulting from disintegration of the sheets.
- Tsi** OLDER SILCRETE: Strong sheet silcrete and coarse lag gravel resulting from disintegration of the sheets.
- Kib** BULLDOG SHALE: Shiny white and red mottled kaolinitic clay, in part weakly cemented by secondary silicification
- Kio** CADNA - OWIE FORMATION: Weak white quartz sandstone with kaolinitic matrix

- X 84** Soil sample location
- >** Soil investigation Trench
- 20B** Surveyed rail centre line with km peg
- HYDROGEOLOGICAL INVESTIGATION BORES**
- Successful
- Unsuccessful
- Yield in gallons/hour**
- Pump depth in metres**

- Geological boundary**
- Scarp**
- Dry weather tracks**
- Fence**
- Gate**
- Windmill**



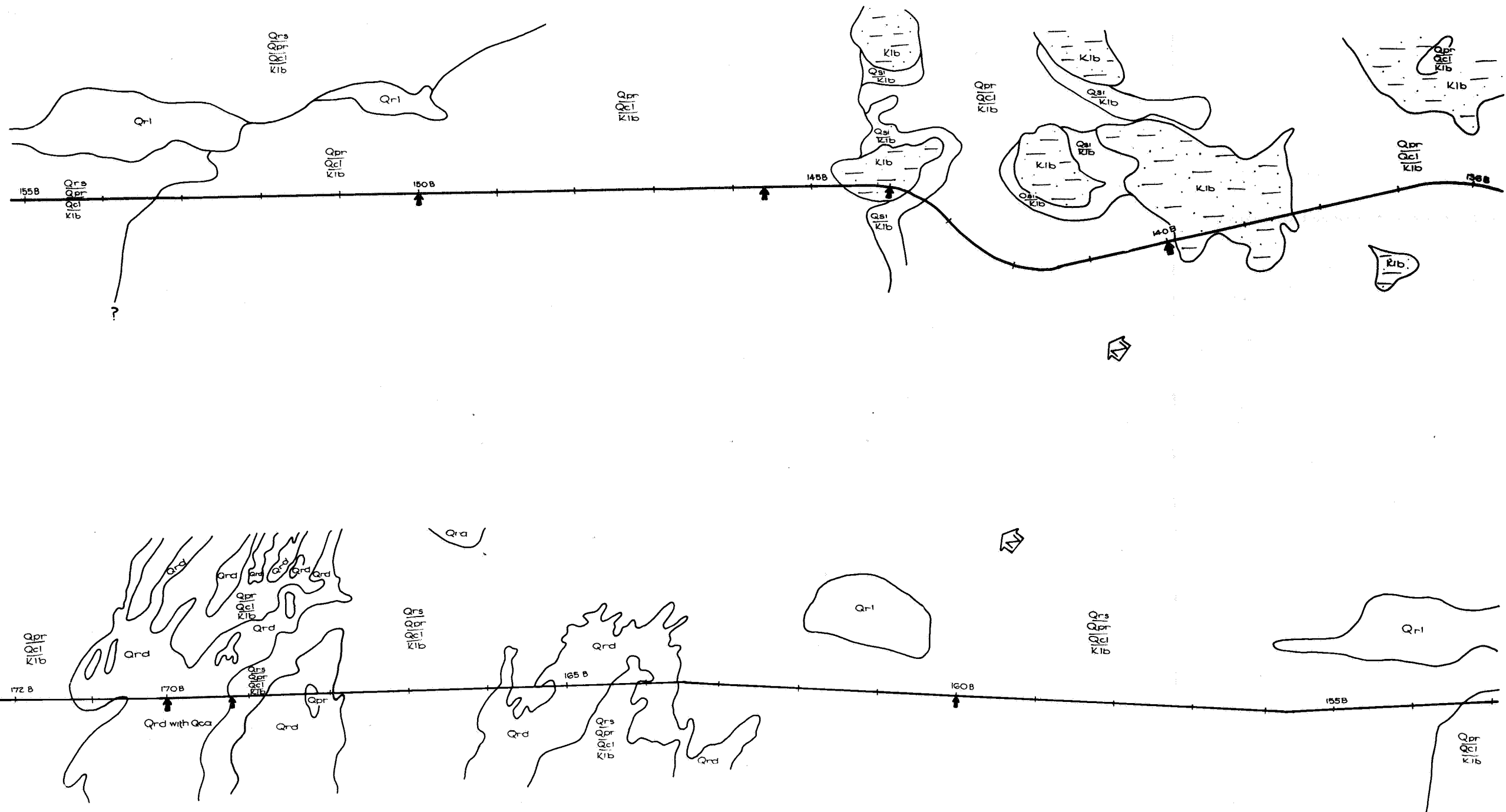
NB: THIS PLAN WAS PRODUCED FROM AERIAL PHOTOS WITHOUT THE USE OF GROUND CONTROL

DEPARTMENT OF MINES - SOUTH AUSTRALIA

**TARCOOLA - ALICE SPRINGS RAILWAY**  
86B KILOMETRE TO 138B KILOMETRE SECTION

|                             |                |                    |                       |
|-----------------------------|----------------|--------------------|-----------------------|
| ENGINEERING GEOLOGY SECTION | GEOLOGIST      | Compiled<br>R.F.J. | Scale: 1:24000 (ORIG) |
|                             |                | Drm. R.B.          | Date: 15th July 1974  |
| Director of Mines           | SUP. GEOLOGIST | Chd. A.F.          | Drg. No. 74-518<br>Ba |

FIG. 6



WINTINNA HILL

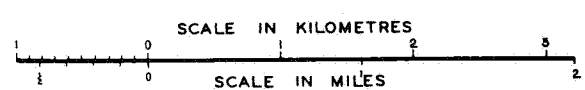
### LEGEND

(NOT IN STRATIGRAPHIC ORDER)

- |                                                                                                                                         |                                                                                                                              |
|-----------------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------------------------------------------------------------------------------|
| <b>Qca</b> CALCRETE. Nodular and strong sheet calcrete.                                                                                 | <b>Qrs</b> CRIST SAND. Sand and silty sand forming drift sheets. Qrs is thin drift generally 200 cms. Qrs is thick 1000 cms. |
| <b>Qcl</b> CALCIFIED LIGN. Cover of subsoil lime accumulation.                                                                          | <b>Qrl</b> PLAYA DEPOSITS. Saline, gypsum, clays and silts of lakes, playans and swamps, usually dry.                        |
| <b>Qpa</b> OLDER ALLUVIUM. Dark gravel and silty sand deposits of former river channels.                                                | <b>Qra</b> RECENT ALLUVIUM. Unconsolidated generally clean sand and gravel of the present stream courses.                    |
| <b>Qpr</b> PEBBLY SAND. Red brown silty clay with a skin of lag gravel. Forms "crab hole country". Qps is gravelly (Qpr wash material). | <b>Qra</b> GYPSUM CLAY. Gypsum, gypsum and gypsum sand mixtures forming dunes.                                               |
| <b>Qrd</b> DUNE SAND. Sand and silty sand forming dunes.                                                                                | <b>Qs</b> QUARTZITE. Gneiss and white (grey) quartzite forming hills, aprons and scarp.                                      |

- |                                                                                                                               |                                                                                                                      |
|-------------------------------------------------------------------------------------------------------------------------------|----------------------------------------------------------------------------------------------------------------------|
| <b>Tfe</b> TERRAZZES. Strong sheet ferruginous (ironstone) and coarse lag gravel resulting from disintegration of the sheets. | <b>Kib</b> BUNTING SHALE. Red and red mottled argillaceous clay, in part weakly cemented by secondary calcification. |
| <b>Qcl</b> OLDER CALCAREOUS. Strong sheet calcareous and coarse lag gravel resulting from disintegration of the sheets.       | <b>Kib</b> LADINA SWELL FORMATION. Weak white quartz and/or with argillaceous matrix.                                |

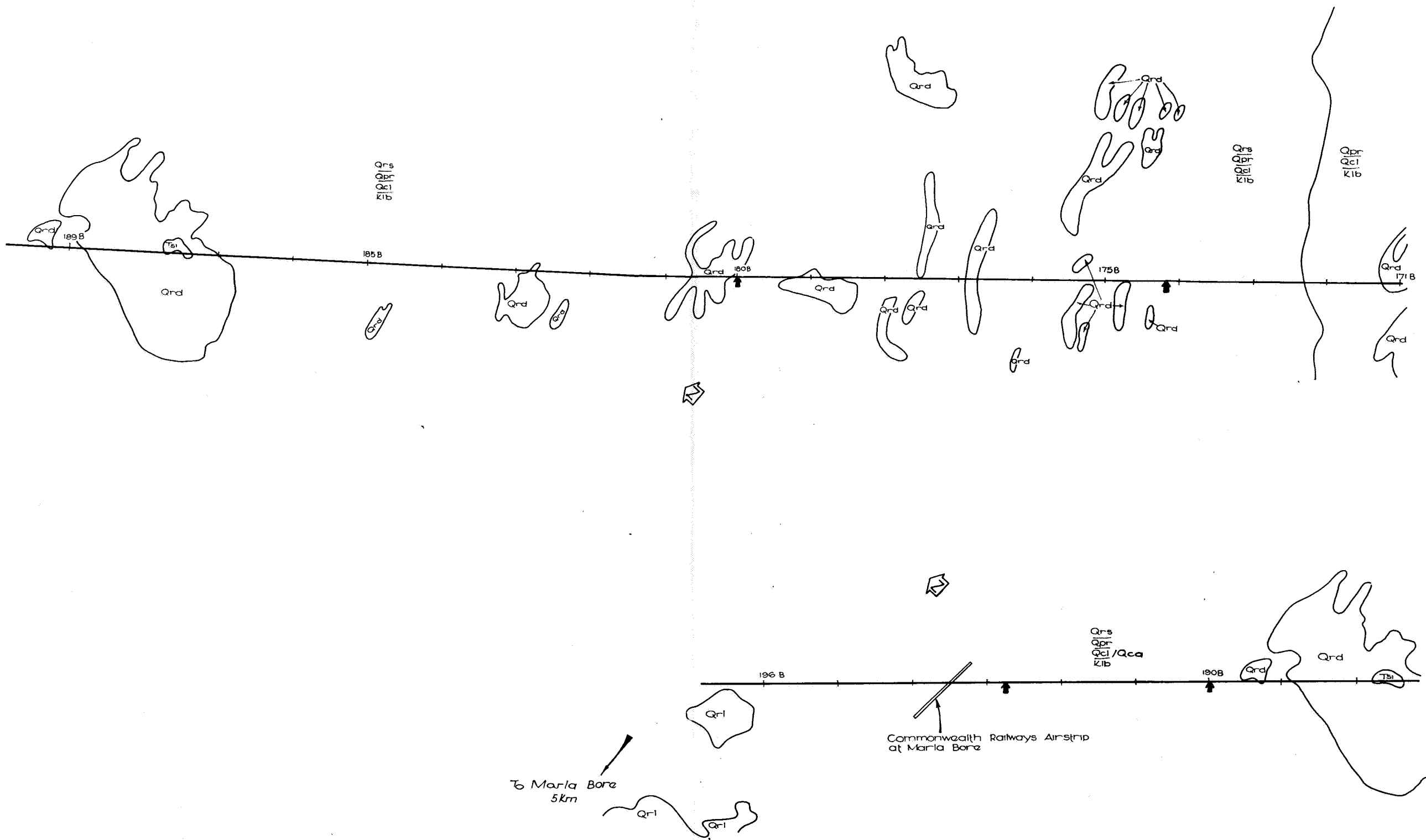
- |                                                   |                            |
|---------------------------------------------------|----------------------------|
| <b>X 84</b> Soil sample location.                 | <b>Geological boundary</b> |
| <b>→</b> Soil investigation trench.               | <b>Scarp</b>               |
| <b>205</b> Surveyed rail centre line with km peg. | <b>Dry weather tracks</b>  |
| <b>HYDROGEOLOGICAL INVESTIGATION BORES</b>        | <b>Fence</b>               |
| • Successful.                                     | <b>Gate</b>                |
| • Field in gallons per hour.                      | <b>Windmill</b>            |
| • Pump depth in metres.                           |                            |



NB: THIS PLAN WAS PRODUCED FROM AERIAL PHOTOS WITHOUT THE USE OF GROUND CONTROL.

| DEPARTMENT OF MINES — SOUTH AUSTRALIA    |                |                        |                      |
|------------------------------------------|----------------|------------------------|----------------------|
| TARCOOLA — ALICE SPRINGS RAILWAY         |                |                        |                      |
| 136B KILOMETRE TO 172B KILOMETRE SECTION |                |                        |                      |
| ENGINEERING GEOLOGY SECTION              | GEOLOGIST      | Compiled <b>R.F.J.</b> | Scale: 1:24000 (ORI) |
|                                          |                | Drawn <b>R.B.</b>      | Date: 15th July 1974 |
| Director of Mines                        | SUP. GEOLOGIST | Chd. A.F.              | Org. No. 74-519      |
|                                          |                |                        | Bq                   |

FIG 7



# LEGEND (NOT IN STRATIGRAPHIC ORDER)

|     |                                                                                                                                |
|-----|--------------------------------------------------------------------------------------------------------------------------------|
| Qca | CALCRETE: Nodular and strong sheet calcrete.                                                                                   |
| Qcl | CALCIFIED SOIL: Zone of subsoil lime accumulation.                                                                             |
| Qpa | OLDER ALLUVIUM: Dirty gravel and silty sand deposits of former river channels.                                                 |
| Qpr | RESIDUAL SOIL: Red-brown silty clay with a skin of lag gravel. Forms "crab hole country". Qps is gravelly slope wash material. |
| Qrd | DUNE SAND: Sand and silty sand forming dunes.                                                                                  |
| Qrs | DRIFT SAND: Sand and silty sand forming drift sheets. Qrs is thin drift generally <50 cms. Qrs is thick >50 cms.               |
| Qri | PLAYA DEPOSITS: Saline, gypsiferous, clays and silts of lakes, claypans and swamps, usually dry.                               |
| Qra | RECENT ALLUVIUM: Unconsolidated, generally clean, sands and gravels of the present stream courses.                             |
| Qrg | GYPSEOUS DUNES: Powdery gypsum and gypsum/sand mixtures forming dunes.                                                         |
| Qsi | YOUNGER SILCRETE: Red, pink and white "jasper" silcrete forming ridge cappings and scarps.                                     |

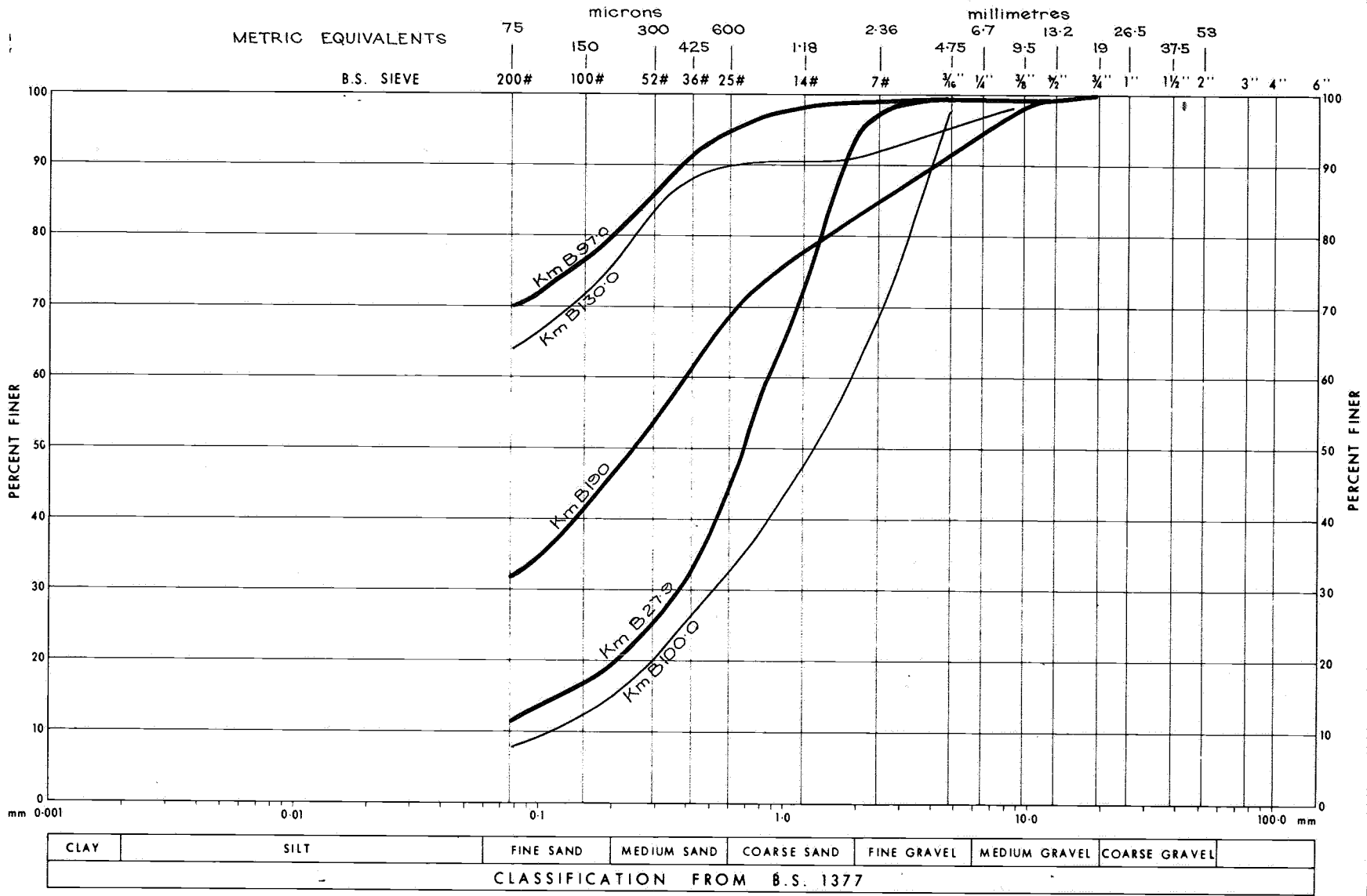
|     |                                                                                                                       |
|-----|-----------------------------------------------------------------------------------------------------------------------|
| Tfo | FERRICRETE: Strong sheet ferruginous ("ironstone") and coarse lag gravel resulting from disintegration of the sheets. |
| Tsi | OLDER SILCRETE: Strong sheet silcrete and coarse lag gravel resulting from disintegration of the sheets.              |
| Kib | BULLDOG SHALE: Buff white and red mottled kaoliniferous clay, in part weakly cemented by secondary silicification.    |
| Kic | GADNA-OWIE FORMATION: Weak white quartz sandstone with kaoliniferous matrix.                                          |

|                       |                                       |
|-----------------------|---------------------------------------|
| X 84                  | Soil sample location                  |
| —                     | Soil Investigation Trench             |
| 200                   | Surveyed rail centre line with km peg |
| ●                     | Successful                            |
| ○                     | Unsuccessful                          |
| Yield in gallons/hour |                                       |
| Pump depth in metres  |                                       |
| —                     | Geological boundary                   |
| —                     | Scarp                                 |
| —                     | Dry weather tracks                    |
| —                     | Fence                                 |
| —                     | Gate                                  |
| —                     | Windmill                              |

SCALE IN KILOMETRES  
SCALE IN MILES

NB: THIS PLAN WAS PRODUCED FROM AERIAL PHOTOS WITHOUT THE USE OF GROUND CONTROL.

| DEPARTMENT OF MINES - SOUTH AUSTRALIA   |           |                  |                       |
|-----------------------------------------|-----------|------------------|-----------------------|
| TARCOOLA - ALICE SPRINGS RAILWAY        |           |                  |                       |
| 1718 KILOMETRE TO 196 KILOMETRE SECTION |           |                  |                       |
| ENGINEERING GEOLOGY SECTION             | GEOLOGIST | Compiled R.F. J. | Scale: 1:24000 (ORIG) |
|                                         |           | Drs. R.B.        | Date: 15th July 1974  |
|                                         |           | Chd. A.F.        | Drg. No. 74-520       |
|                                         |           |                  | Ba                    |



NOTE: Sample number is chainage location.

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 24<sup>TH</sup> JULY 1974

Drn: A.R.

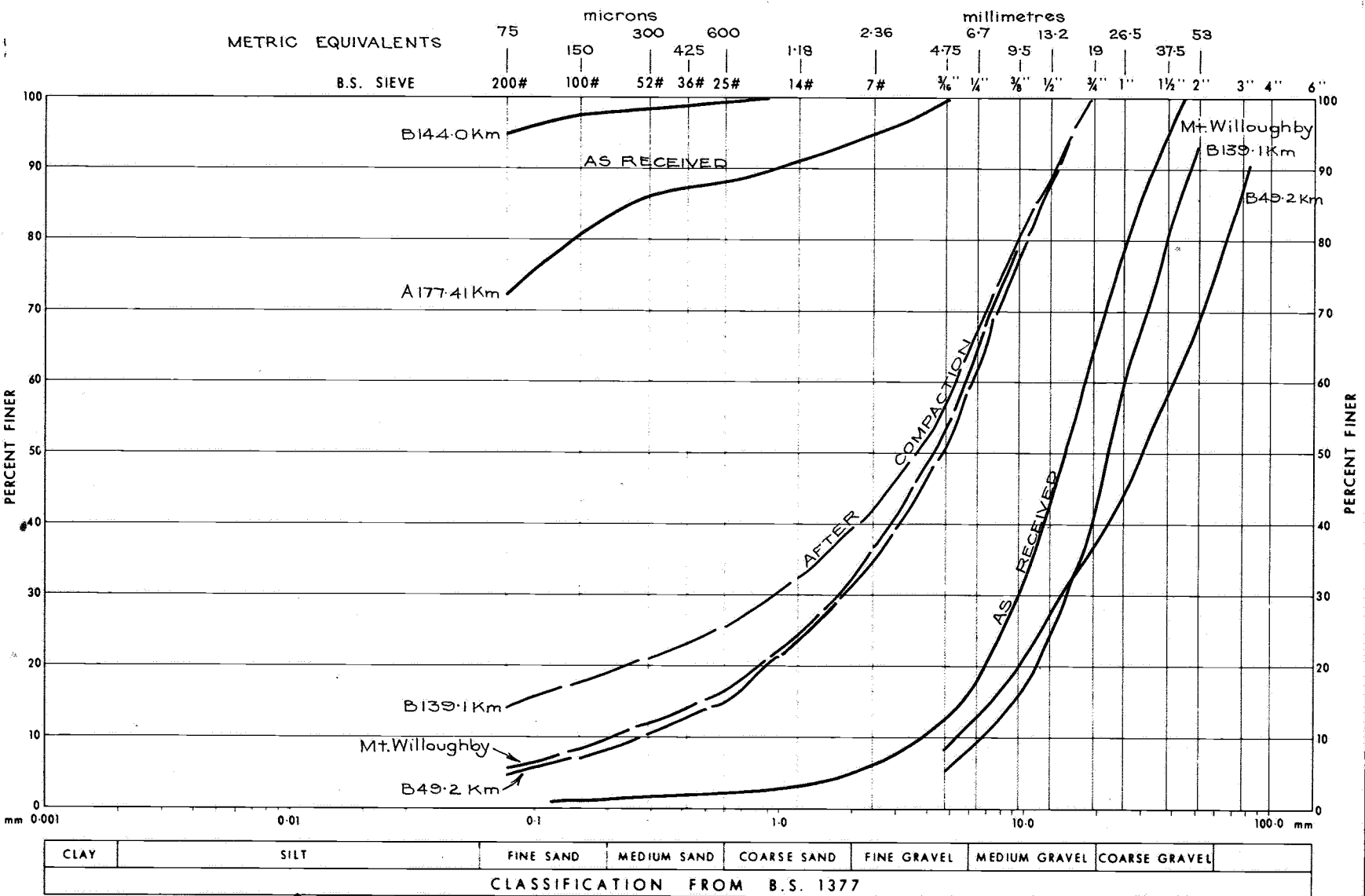
TESTING OF MATERIALS

FIG: 9

Ckd:

ROBIN RISE TO MARLA BORE — SUMMARY OF  
RESIDUAL SOIL (Qpr)

Drp No. S11369



ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: G.H. McN

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 6th March 1975

Drm: A.R.

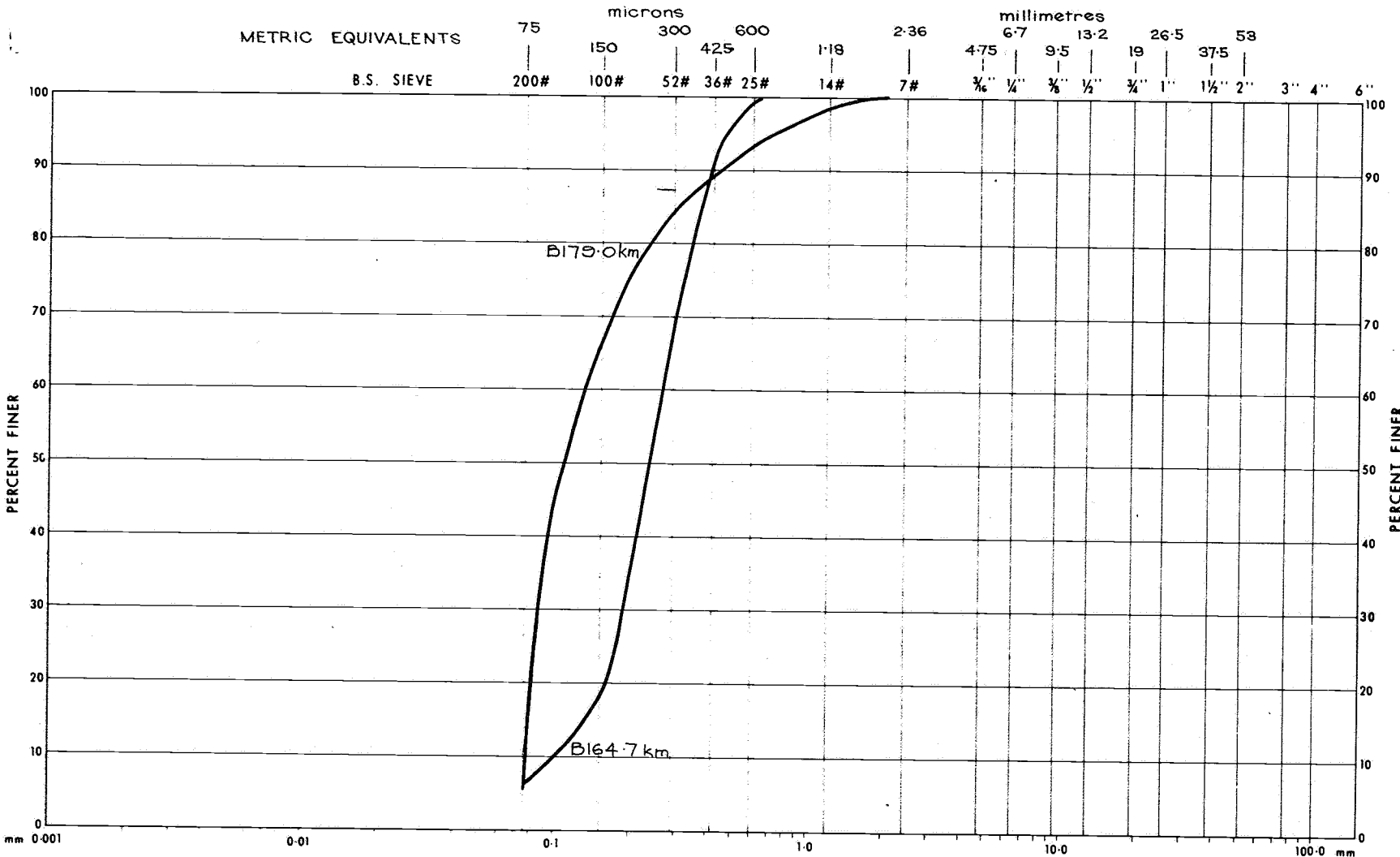
TESTING OF MATERIALS

FIG: 10

Ckd:

ROBIN RISE TO MARLA — BORE — SUMMARY OF  
RESULTS ON BULLDOG SHALE (KIB)

Drg No. S11370



|                               |      |           |             |             |             |               |               |
|-------------------------------|------|-----------|-------------|-------------|-------------|---------------|---------------|
| CLAY                          | SILT | FINE SAND | MEDIUM SAND | COARSE SAND | FINE GRAVEL | MEDIUM GRAVEL | COARSE GRAVEL |
| CLASSIFICATION FROM B.S. 1377 |      |           |             |             |             |               |               |

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Compiled: G.H.McN.

Drn: A.R.

Ckd:

TARCOOLA — ALICE SPRINGS RAILWAY

TESTING OF MATERIALS

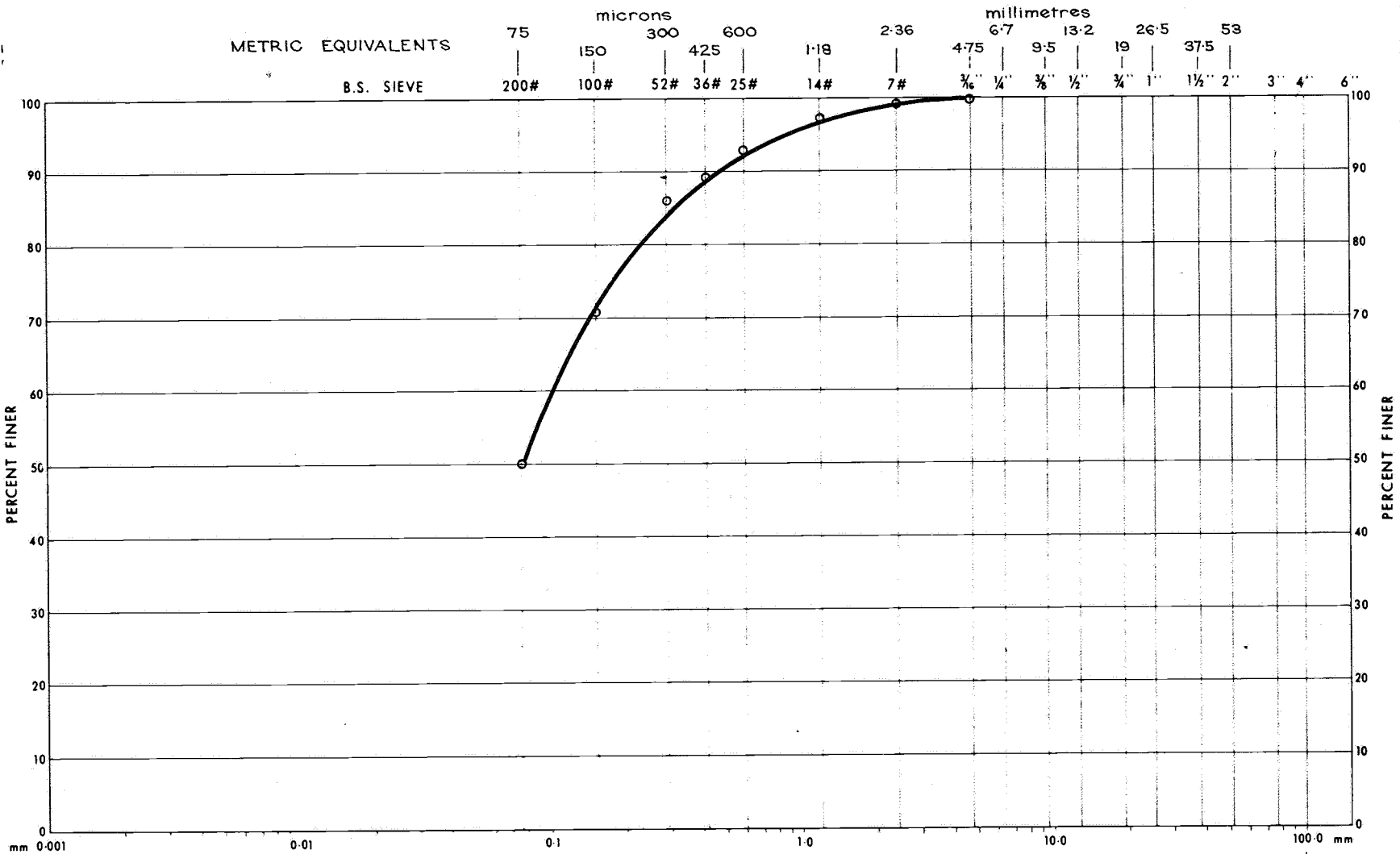
ROBIN RISE TO MARLA BORE — SUMMARY OF RESULTS ON DUNE SAND (Grd)

Date: 6<sup>th</sup> March 1975

FIG: 11

Scale: As shown

Drp No. S11371



|                               |      |           |             |             |             |               |               |
|-------------------------------|------|-----------|-------------|-------------|-------------|---------------|---------------|
| CLAY                          | SILT | FINE SAND | MEDIUM SAND | COARSE SAND | FINE GRAVEL | MEDIUM GRAVEL | COARSE GRAVEL |
| CLASSIFICATION FROM B.S. 1377 |      |           |             |             |             |               |               |

ENGINEERING GEOLOGY  
SECTION

Compiled: GHMCN

Drm:

Ckd:

DEPARTMENT OF MINES — SOUTH AUSTRALIA

TARCOOLA — ALICE SPRINGS RAILWAY  
TESTING OF MATERIALS

PLAYA DEPOSITS (Qr1) - Ch. A161.5

Scale: As shown

Date: 5-3-75

FIG: 12

Drg No. S11372

# MODIFIED A.A.S.H.O. COMPACTION TEST

## ATTERBERG LIMITS:

Liquid Limit 35 % Plasticity Index 19  
Plastic Limit 16 % Linear Shrinkage - %

## COMPACTION:

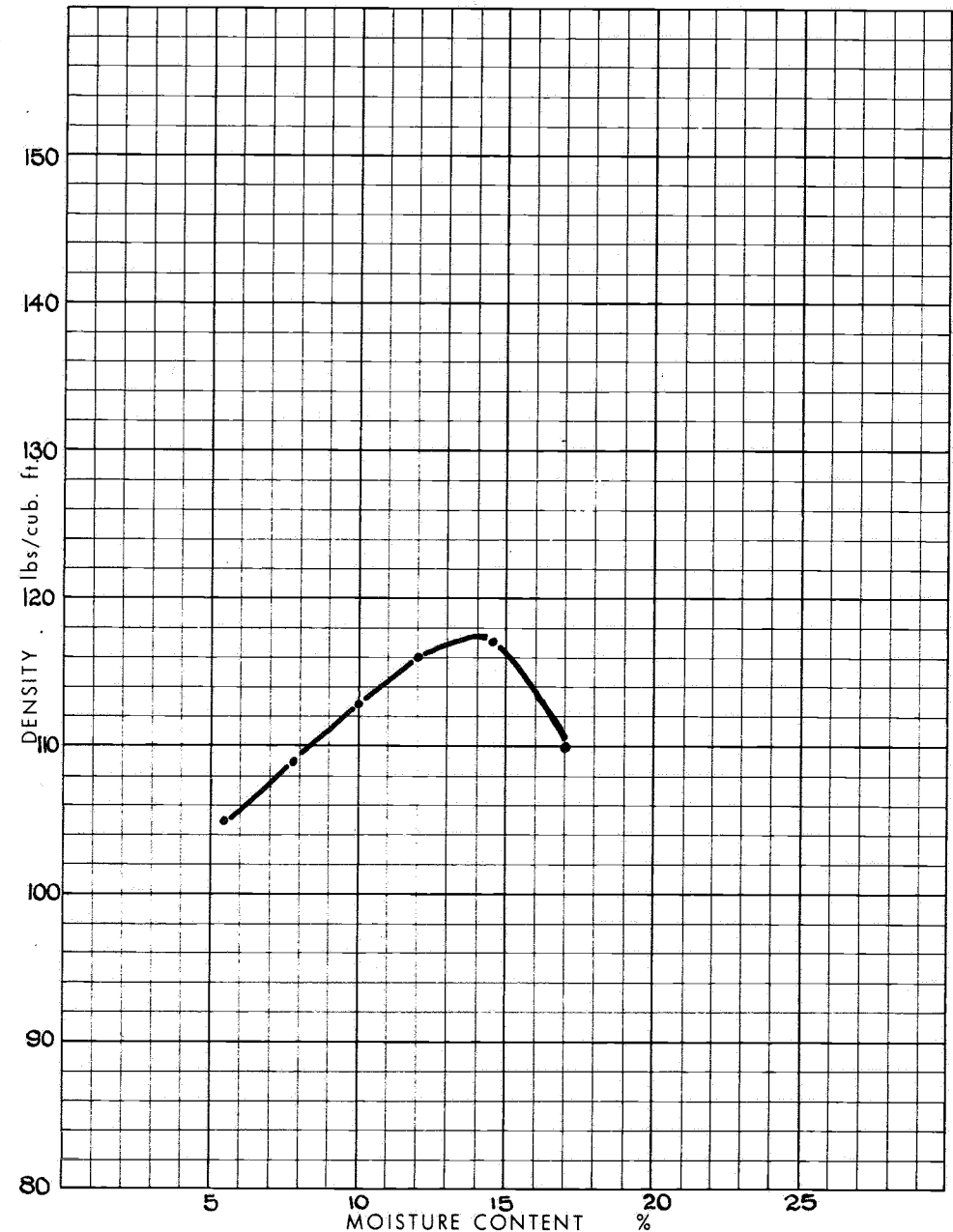
Maximum Dry Density 117 lbs/cub.ft.  
Optimum Moisture Content 14.5 %  
Natural Moisture Content - %  
Compacted Dry Density at Natural Moisture Content  
lbs/cub.ft.  
%AASHO

CALIFORNIAN BEARING RATIO: (estimated)

9.6 %

## COMMENTS:

Saline water used



DESCRIPTION OF MATERIAL

Sandy Clay

SAMPLE No. A161.5km LOCATION

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date

Drm: J.W.

ENGINEERING PROPERTIES OF MATERIALS

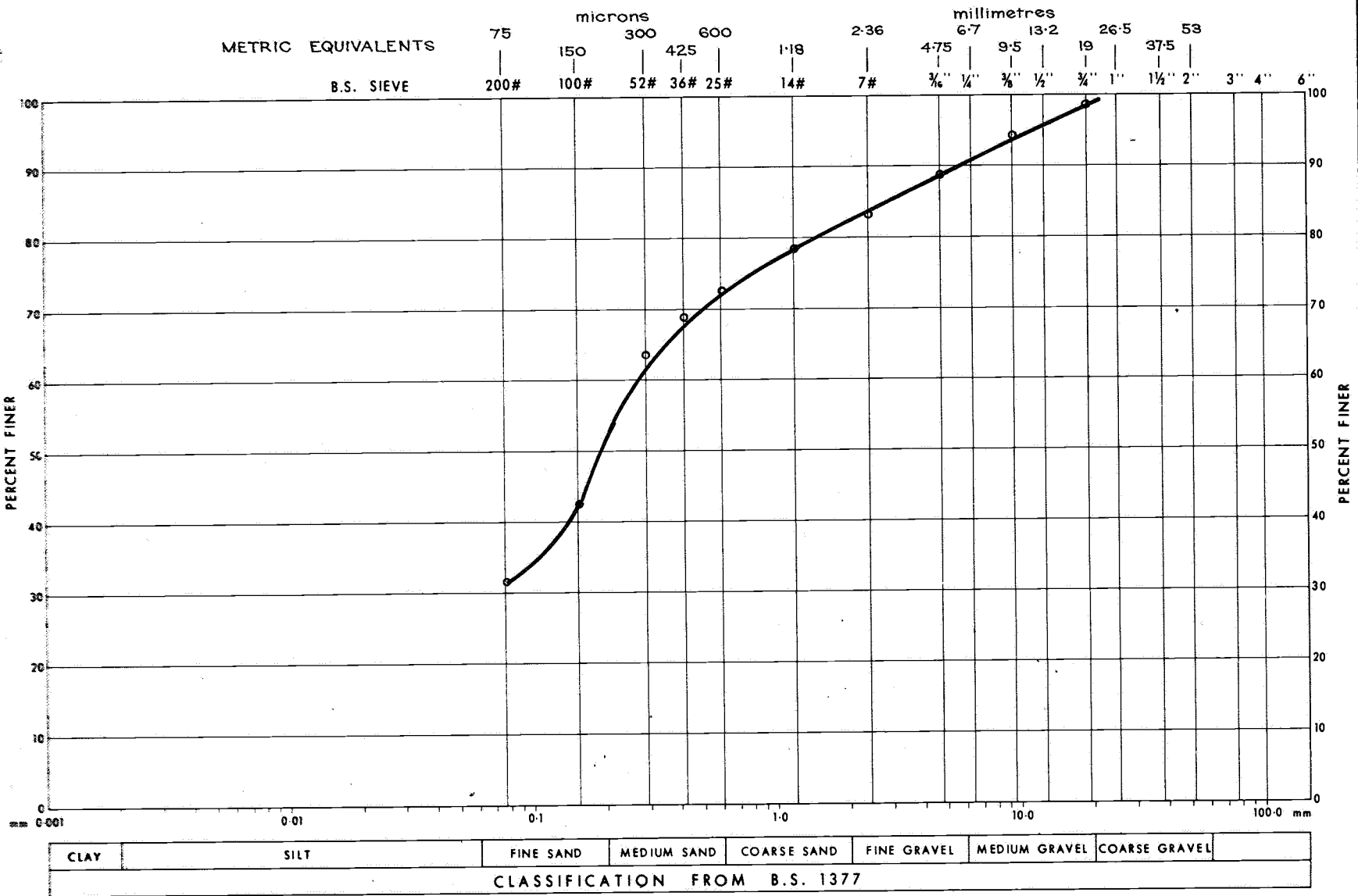
FIG. 13

Ctd:

PLAYA DEPOSITS (Qr1)

Drg No. S 11373





ENGINEERING GEOLOGY  
SECTION

Compiled G.H. McN

Dr. J. Williams

Ckd:

DEPARTMENT OF MINES — SOUTH AUSTRALIA

TARCOOLA — ALICE SPRINGS RAILWAY

TESTING OF MATERIALS

RESIDUAL SOIL & SILCRETE (Qpr+Qsi)CH. A1700

Scale: As shown

Date: 5-3-75

FIG: 14

Drig No. S11374

# MODIFIED A.A.S.H.O. COMPACTION TEST

## ATTERBERG LIMITS:

Liquid Limit 29 % Plasticity Index 12  
Plastic Limit 17 % Linear Shrinkage - %

## COMPACTION:

Maximum Dry Density 127 lbs/cub.ft.  
Optimum Moisture Content 9.5 %  
Natural Moisture Content %  
Compacted Dry Density at Natural Moisture Content  
lbs/cub.ft.  
%AASHO

CALIFORNIAN BEARING RATIO: (estimated)  
17 %

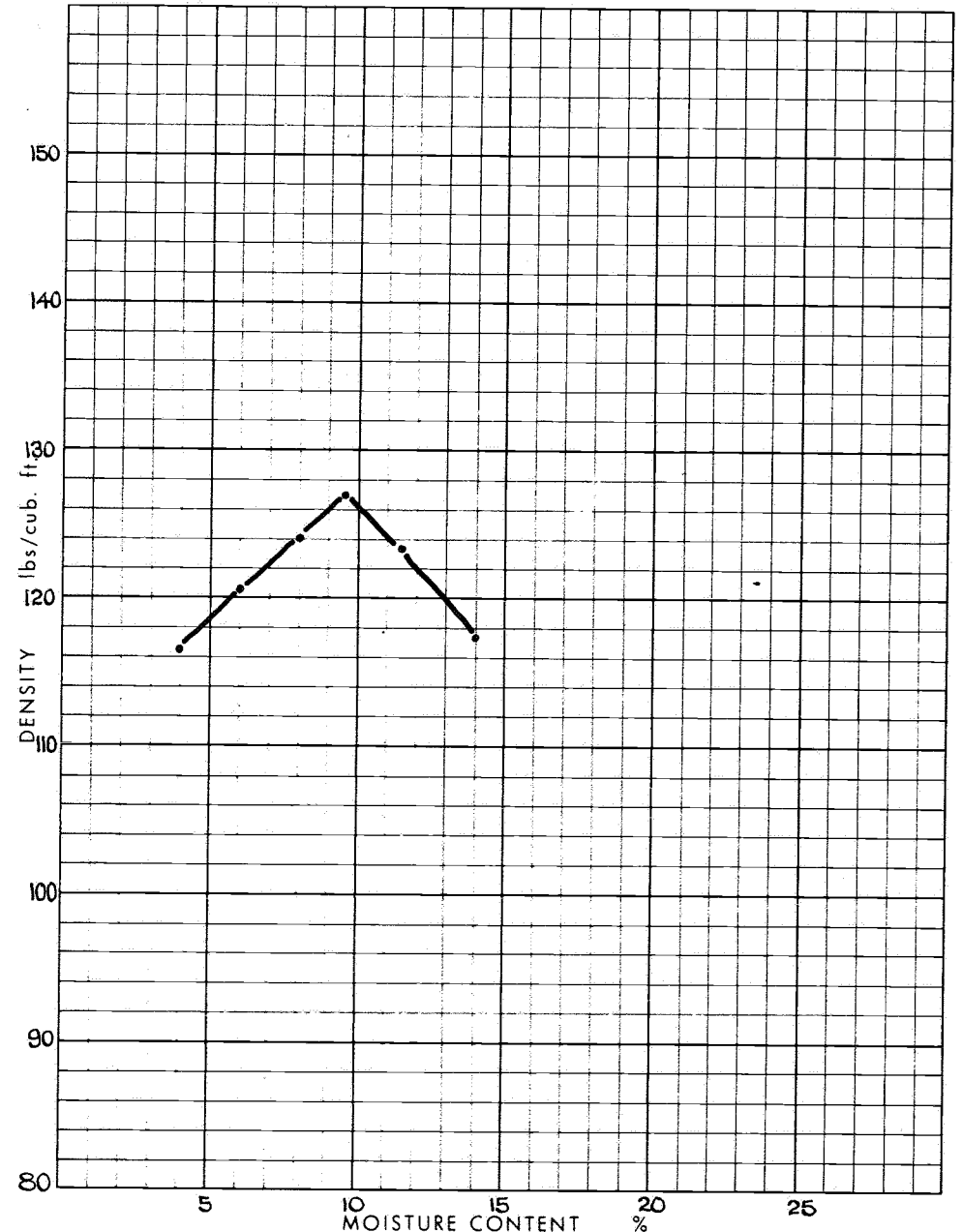
## COMMENTS:

Saline water used

## DESCRIPTION OF MATERIAL

Gravelly and sandy silt

SAMPLE No. A170-0 km LOCATION



ENGINEERING GEOLOGY  
SECTION

Compiled: J. Selby

Drm: J. Williams

Ckd:

DEPARTMENT OF MINES — SOUTH AUSTRALIA

TARCOOLA — ALICE SPRINGS RAILWAY

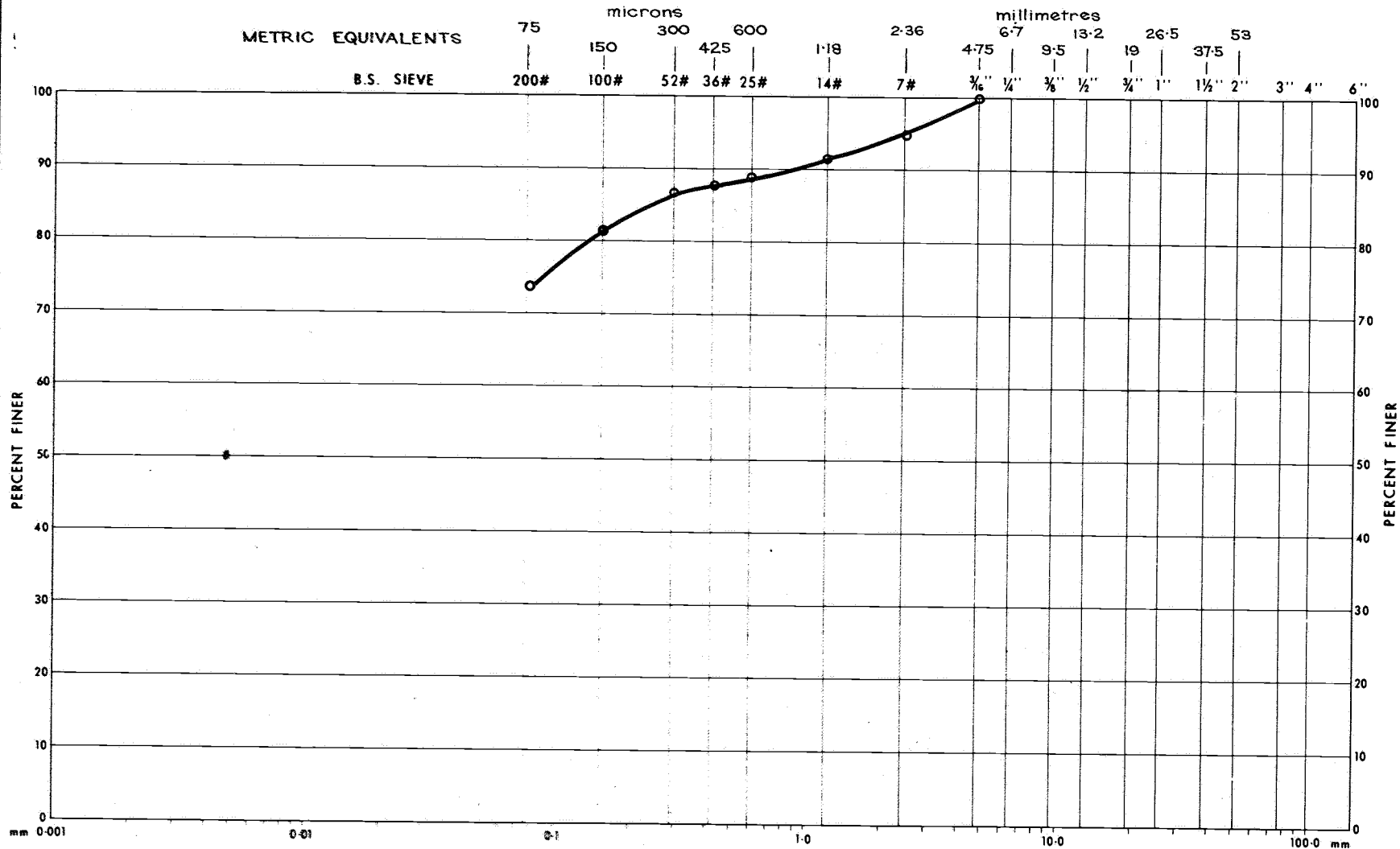
ENGINEERING PROPERTIES OF MATERIALS  
RESIDUAL SOIL & SILCRETE (qpr & qsi)

Scale: As shown

Date: 25-3-75

FIG: 15

Drg No. S11375



|                               |      |           |             |             |             |               |               |
|-------------------------------|------|-----------|-------------|-------------|-------------|---------------|---------------|
| CLAY                          | SILT | FINE SAND | MEDIUM SAND | COARSE SAND | FINE GRAVEL | MEDIUM GRAVEL | COARSE GRAVEL |
| CLASSIFICATION FROM B.S. 1377 |      |           |             |             |             |               |               |

Note - 15% Crystalline Gypsum removed prior to sieving.

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: G.H. McN

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 15-3-75

Drawn: J. Williams

TESTING OF MATERIALS

FIG: 16

Chd:

BULLDOG SHALE (Kib) — Ch. A177.4 km.

Drg No. S11376

# MODIFIED A.A.S.H.O. COMPACTION TEST

## ATTERBERG LIMITS:

Liquid Limit 51 % Plasticity Index 26  
 Plastic Limit 25 % Linear Shrinkage — %

## COMPACTION:

Maximum Dry Density 110 lbs/cub.ft.  
 Optimum Moisture Content 17 %  
 Natural Moisture Content — %  
 Compacted Dry Density at Natural Moisture Content  
 lbs/cub.ft.  
 %AASHO

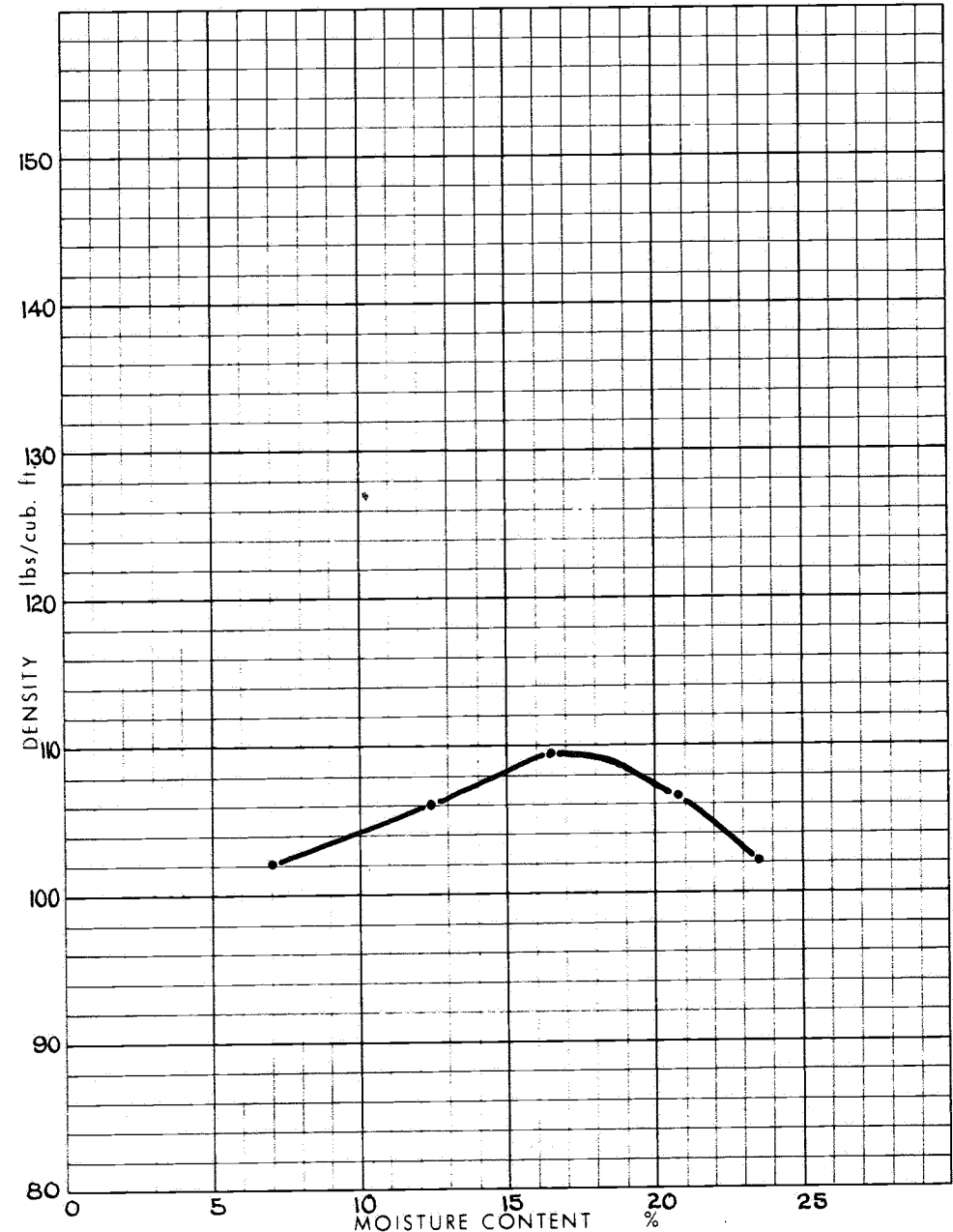
CALIFORNIAN BEARING RATIO: (estimated)  
 %

## COMMENTS:

Saline water used

DESCRIPTION OF MATERIAL  
 Sandy or clayey silt

SAMPLE No. A177-4 km. LOCATION



ENGINEERING GEOLOGY  
 SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date:

Dwn: J.W.

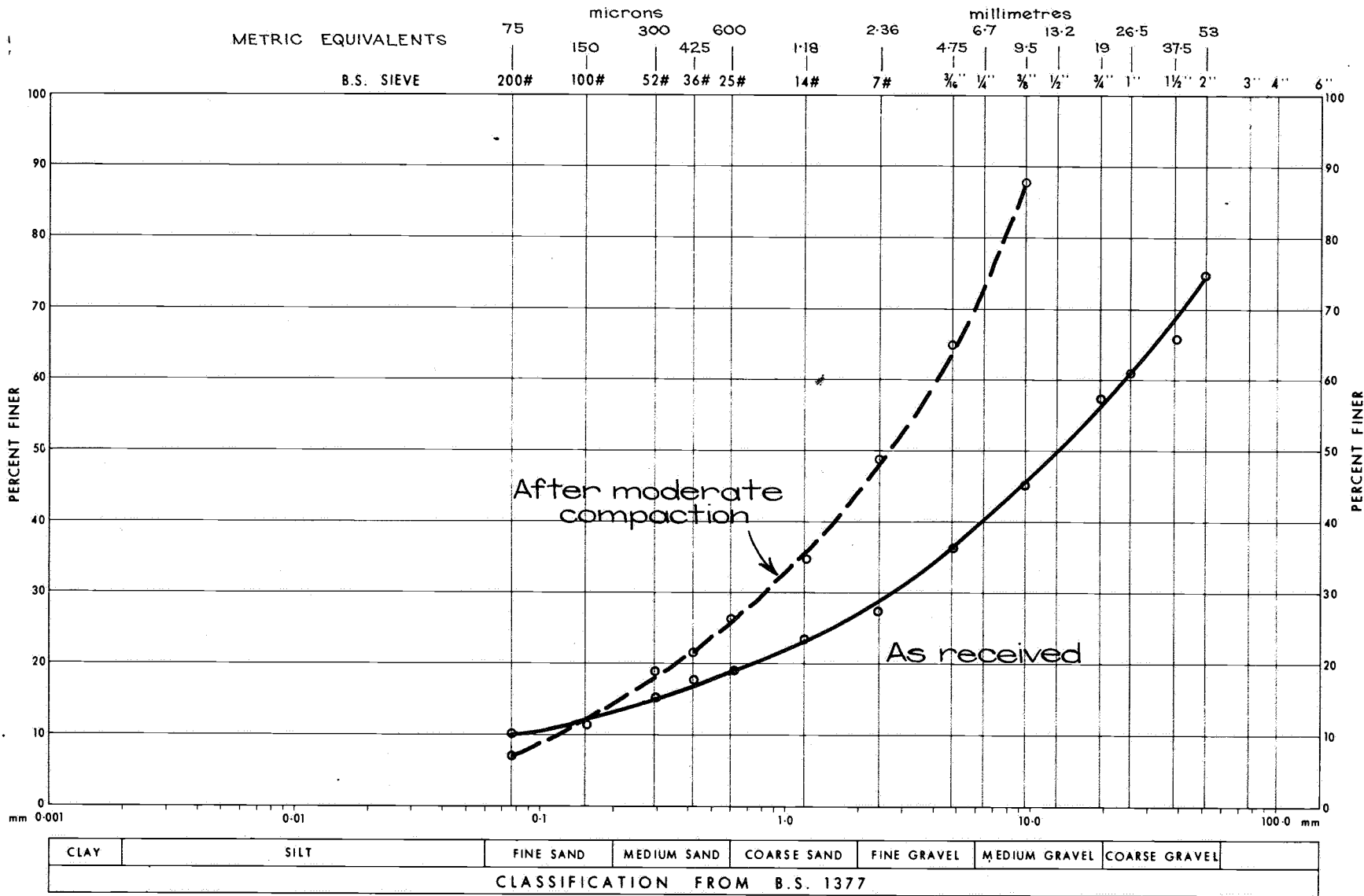
ENGINEERING PROPERTIES OF MATERIALS

FIG: 17

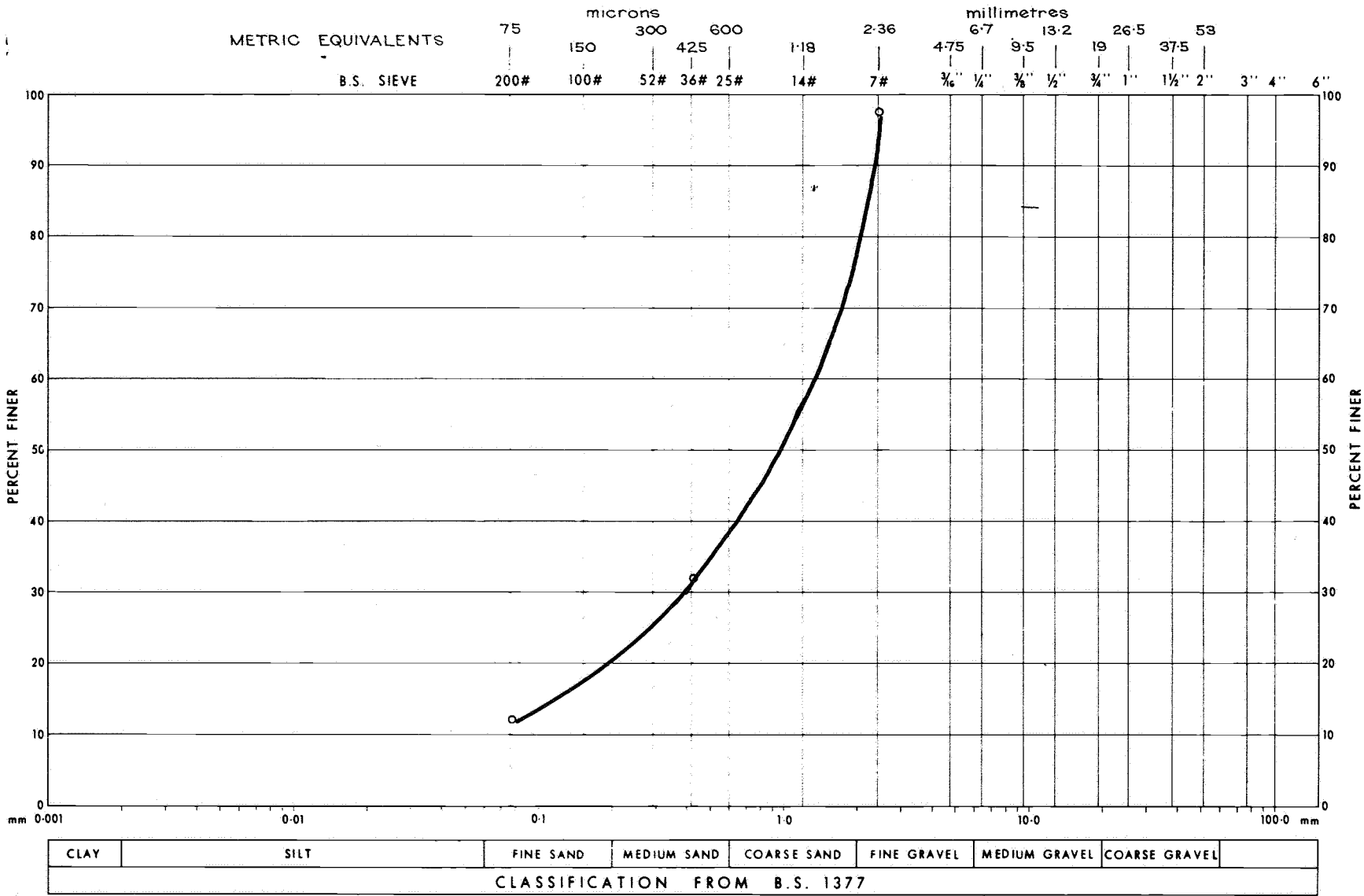
Ckd:

BULLDOG SHALE (Kib)

Dwg No. S11377



Note :- Insufficient material for compaction test.



Note :- Compaction curve not available. Results of Standard AASHO test. M.D.D. 95 p.c.f., O.M.C. 23%.  
 L.L. 36, PL 32, P.I. 4, L.S. 2, est. C.B.R. 28%,  
 field M.C. 6.3%.

ENGINEERING GEOLOGY  
SECTION

Compiled: CH.McN

Dra: J. Williams

Ckd:

DEPARTMENT OF MINES — SOUTH AUSTRALIA

TARCOOLA — ALICE SPRINGS RAILWAY

TESTING OF MATERIALS

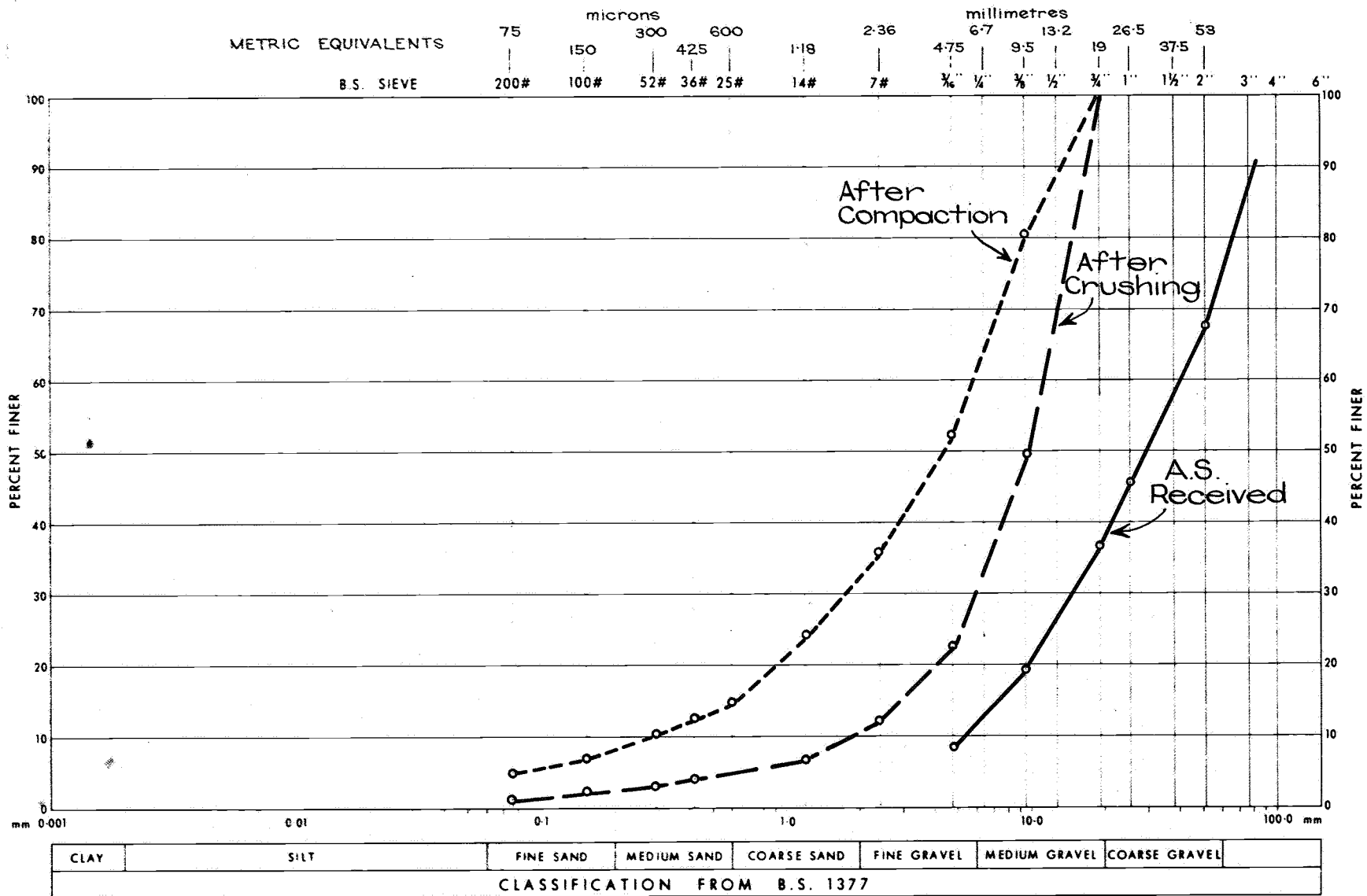
RESIDUAL SOIL (qpr) — Ch. B27.9 km

Scale: As shown

Date: 5-3-75

FIG: 19

Drg No. S11379



Note:- As received material crushed to pass  $\frac{3}{4}$ " (19mm) sieve.  
Results of one-point modified compaction test, M.D.D. 89 pc.t., O.M.C. 11%

ENGINEERING GEOLOGY  
SECTION

Compiled: J. Selby

Drm: J. Williams

Ckd:

DEPARTMENT OF MINES — SOUTH AUSTRALIA

TARCOOLA — ALICE SPRINGS RAILWAY

TESTING OF MATERIALS

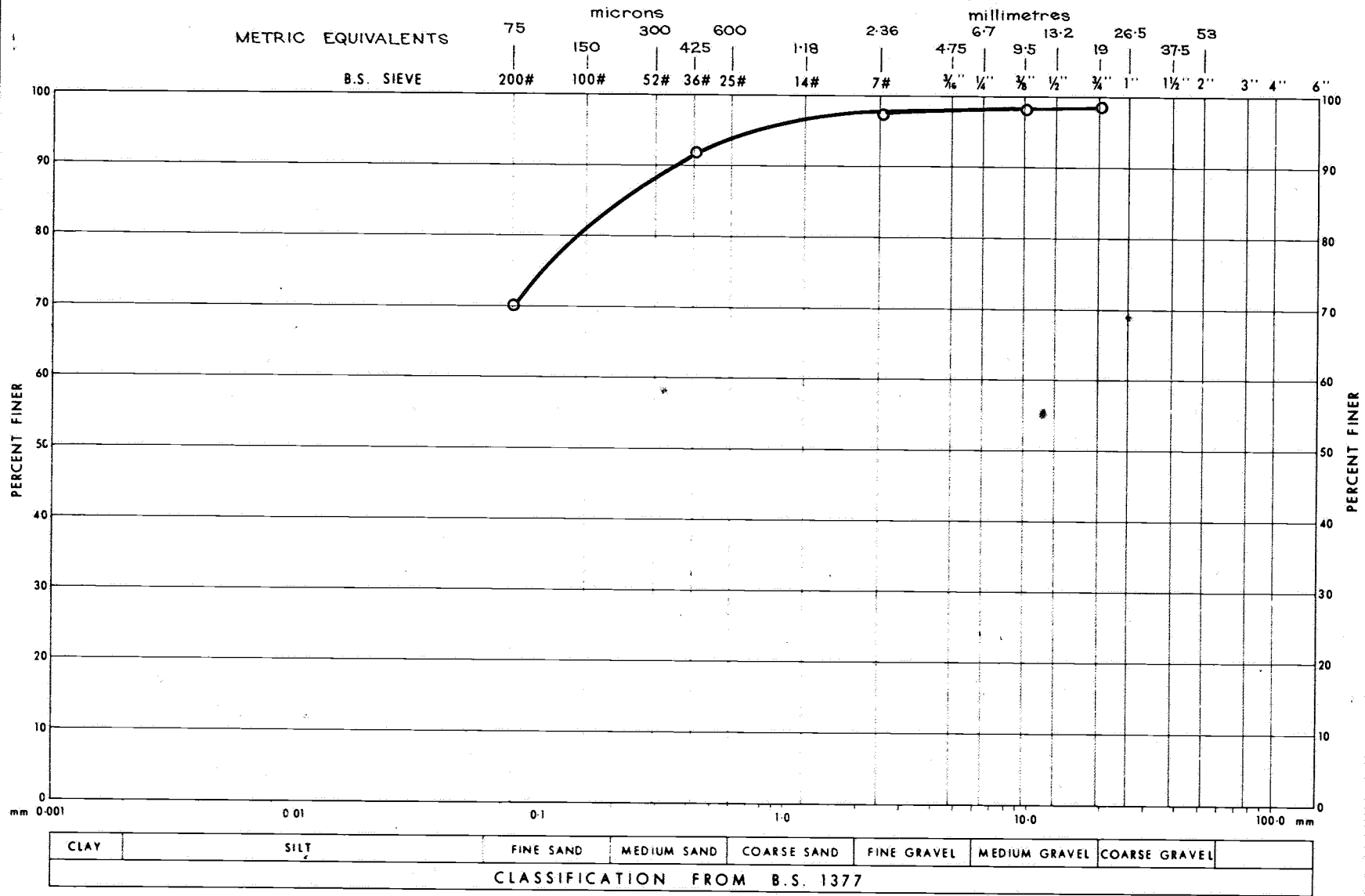
BULLDOG SHALE (Kib) - ch B492 km

Scale: As shown

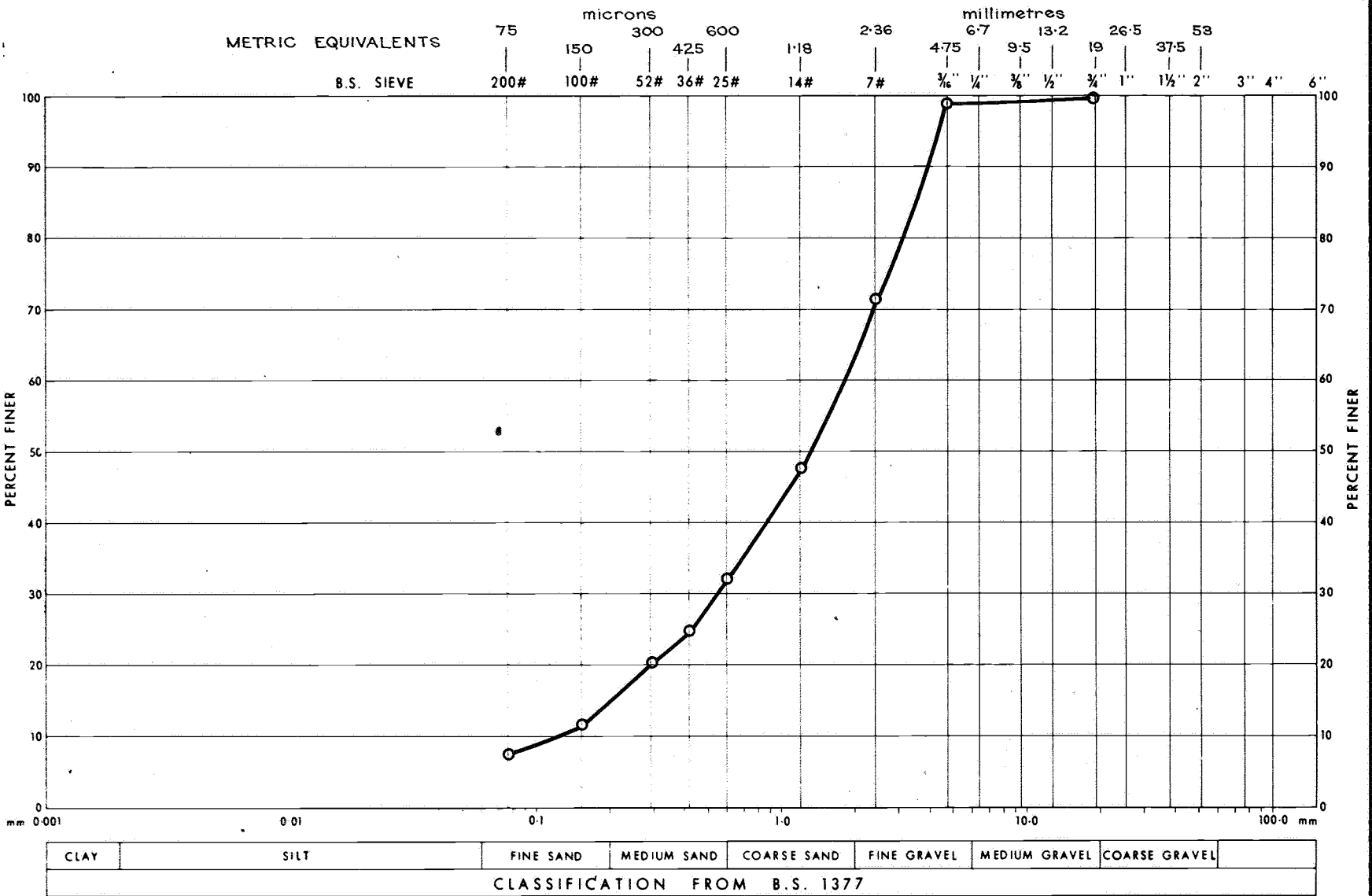
Date: 17th Sept '74

FIG: 20

Drg No. S11380







# MODIFIED A.A.S.H.O. COMPACTION TEST

## ATTERBERG LIMITS: Non-plastic

Liquid Limit % Plasticity Index  
Plastic Limit % Linear Shrinkage %

## COMPACTION:

Maximum Dry Density 95.0 lbs/cub.ft.  
Optimum Moisture Content 21.5 %  
Natural Moisture Content NA %  
Compacted Dry Density at Natural Moisture Content  
lbs/cub.ft.  
%AASHO

CALIFORNIAN BEARING RATIO: (estimated)

42 %

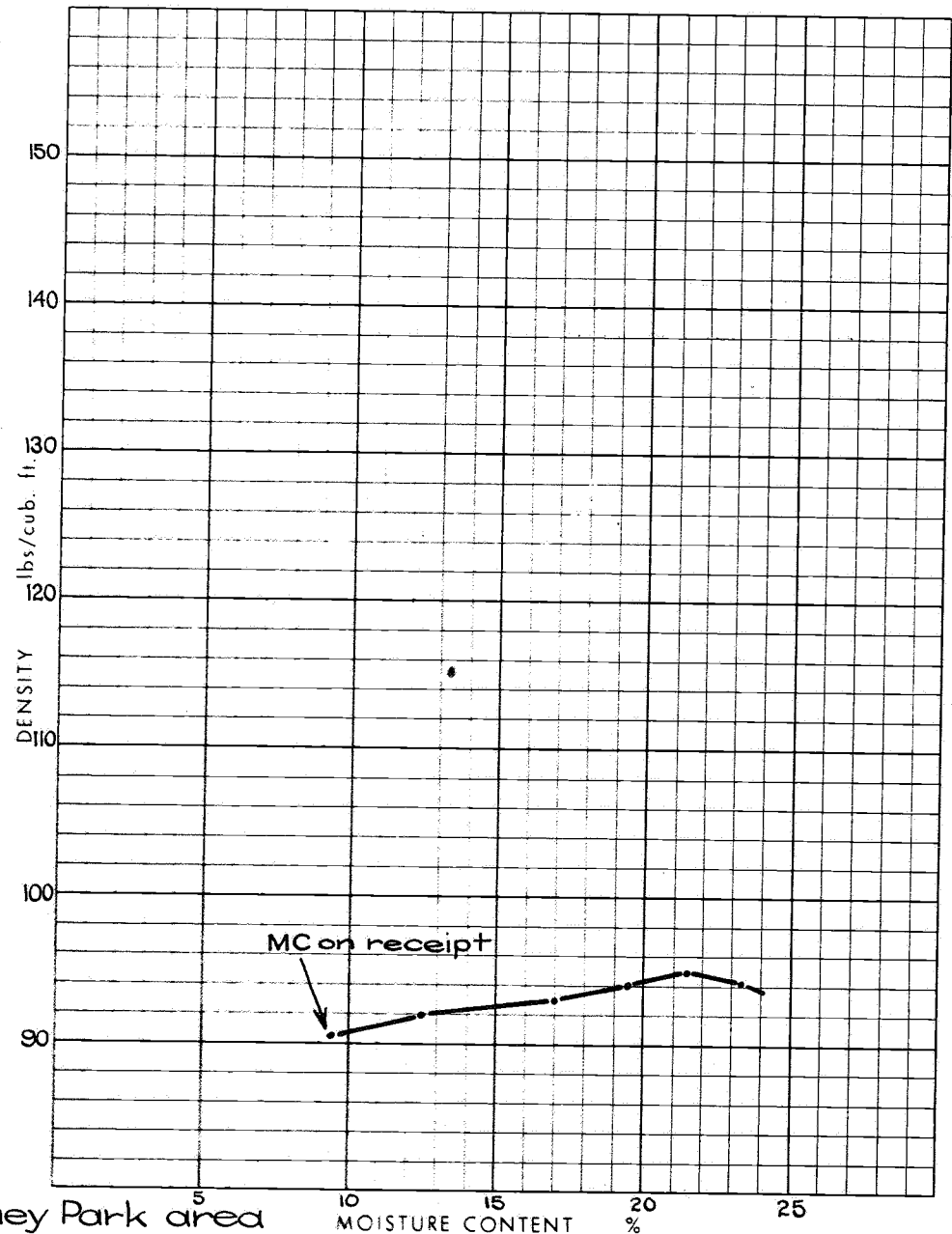
## COMMENTS:

Saline water used

## DESCRIPTION OF MATERIAL

Red-brown silty sand

SAMPLE No. B 100.0 Km LOCATION Cadney Park area



ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Drawn: J.W.

ENGINEERING PROPERTIES OF MATERIALS

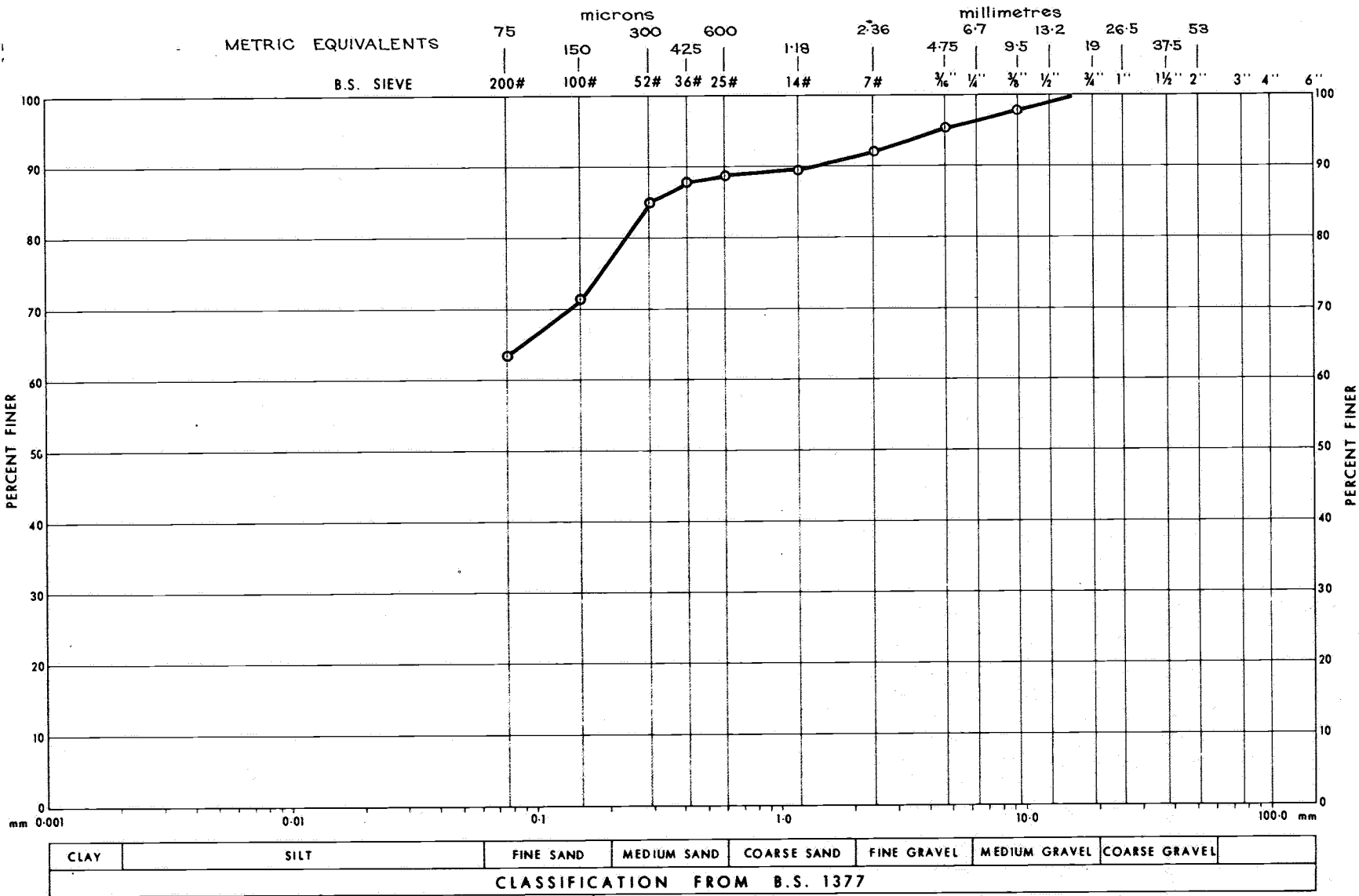
Checked:

RESIDUAL SOIL (Qpr)

Date: 17th Sept '74

FIG. 23

Draw No. 511383



ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 17th Sept '74

Dwn: J. Williams.

TESTING OF MATERIALS

FIG: 24

Ckd:

RESIDUAL SOIL (Qpr)-ch B.130.0K

Drq No. S11384

# MODIFIED A.A.S.H.O. COMPACTION TEST

## ATTERBERG LIMITS:

Liquid Limit 48 % Plasticity Index 26  
 Plastic Limit 22 % Linear Shrinkage NT %

## COMPACTION:

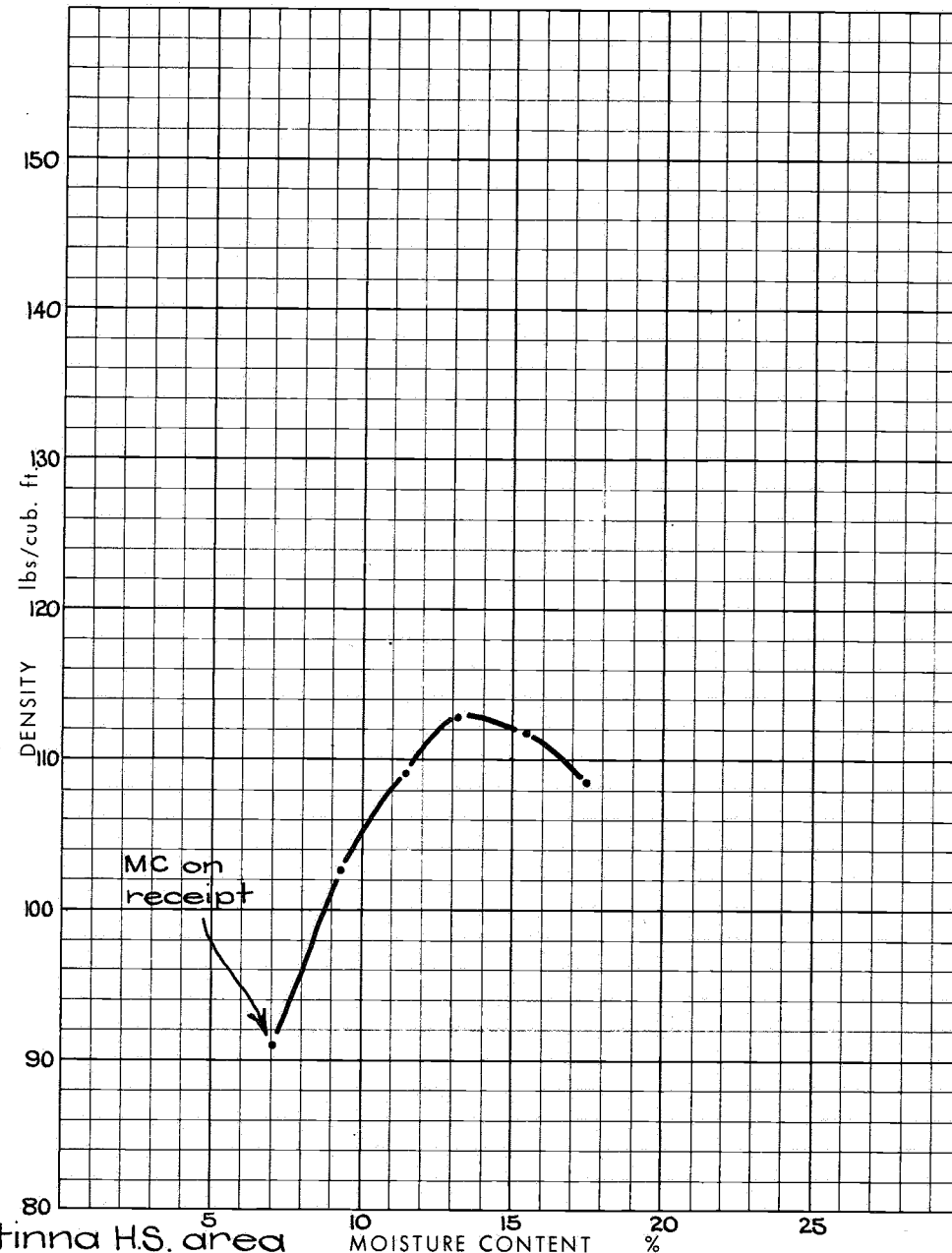
Maximum Dry Density 113 lbs/cub.ft.  
 Optimum Moisture Content 14 %  
 Natural Moisture Content NA %  
 Compacted Dry Density at Natural Moisture Content  
 lbs/cub.ft.  
 %AASHO

CALIFORNIAN BEARING RATIO: (estimated)  
 8.2 %

COMMENTS:  
 Saline water used.

DESCRIPTION OF MATERIAL  
 Red brown sandy, silty clay

SAMPLE No. B.130-0 km. LOCATION Wintinna H.S. area



ENGINEERING GEOLOGY  
 SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 17th. Sept. '74

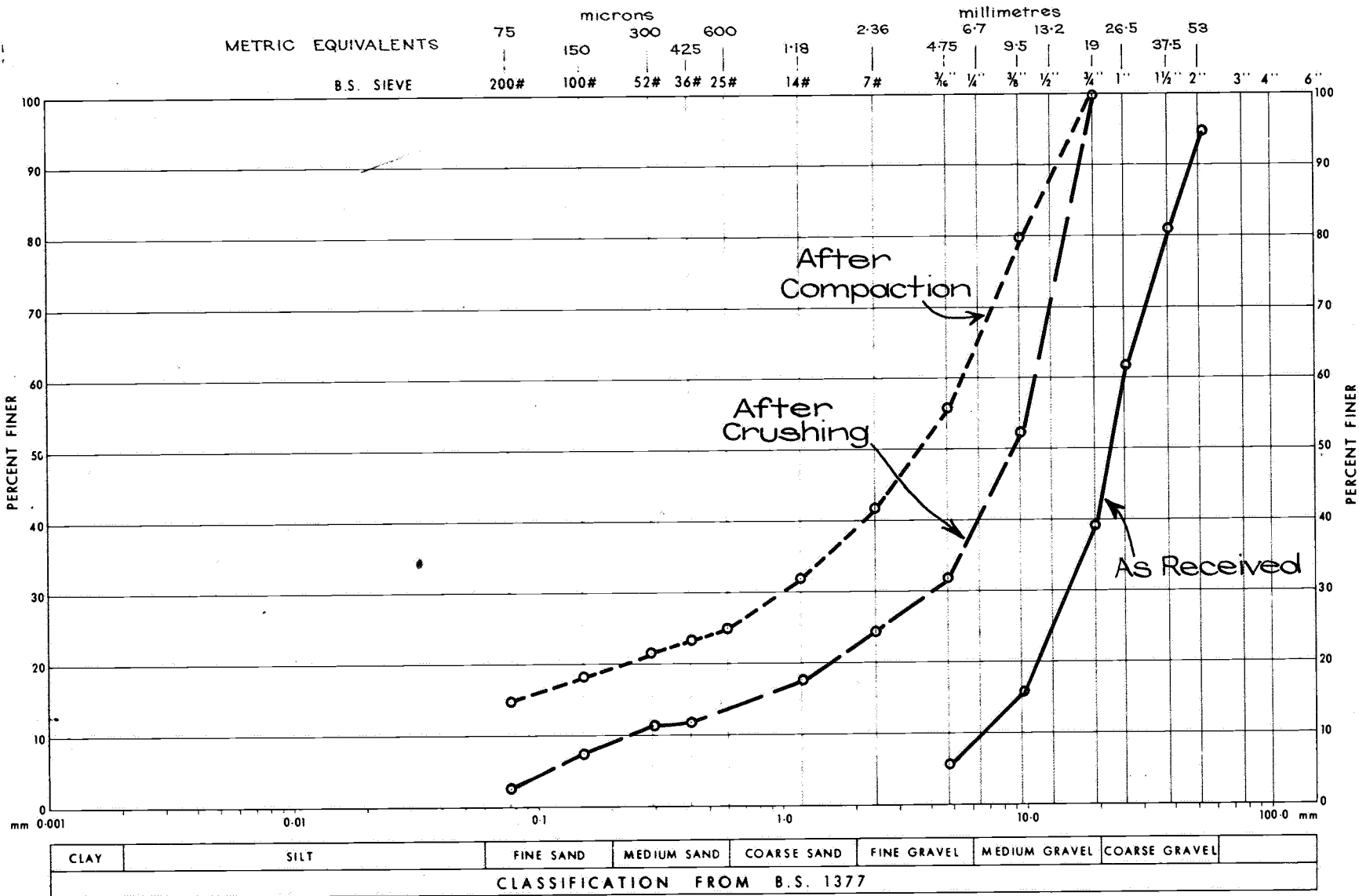
Dr: J. Williams

ENGINEERING PROPERTIES OF MATERIALS

FIG: 25

RESIDUAL SOIL (Qpr)

Dr: No. S.11385



Note :- As received material crushed to pass 3/4" (19 mm.) sieve.  
Results of one-point modified compaction test, M.D.D. 82 p.c.f., O.M.C. 29%

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 17th Sept '74

Drm: J. Williams

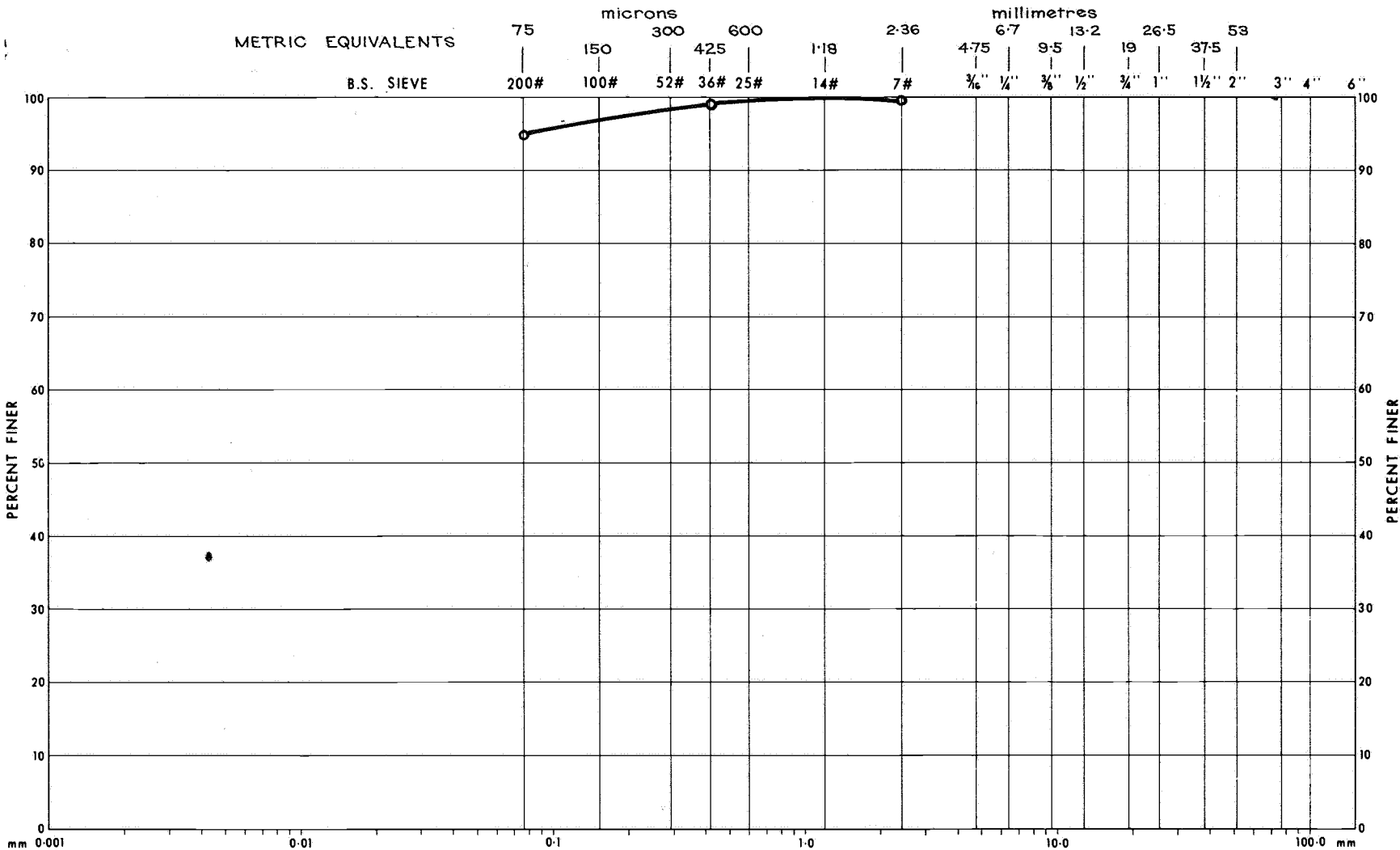
TESTING OF MATERIALS

FIG: 26

Ckd:

BULLDOG SHALE (Kib) — ch B139.1 km

Drg No. S11386



|                               |      |           |             |             |             |               |               |
|-------------------------------|------|-----------|-------------|-------------|-------------|---------------|---------------|
| CLAY                          | SILT | FINE SAND | MEDIUM SAND | COARSE SAND | FINE GRAVEL | MEDIUM GRAVEL | COARSE GRAVEL |
| CLASSIFICATION FROM B.S. 1377 |      |           |             |             |             |               |               |

Note :- Compaction curve not available. Results of standard AASHO test,  
M.D.D. 92 p.c.f., O.M.C. 26%.  
L.L. 62, P.L. 24, P.I. 38, L.S. 16, Est. C.B.R. 4.0%, field M.C. 16.3%.

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: CHMCN

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 5-3-75

Dra: J. Williams

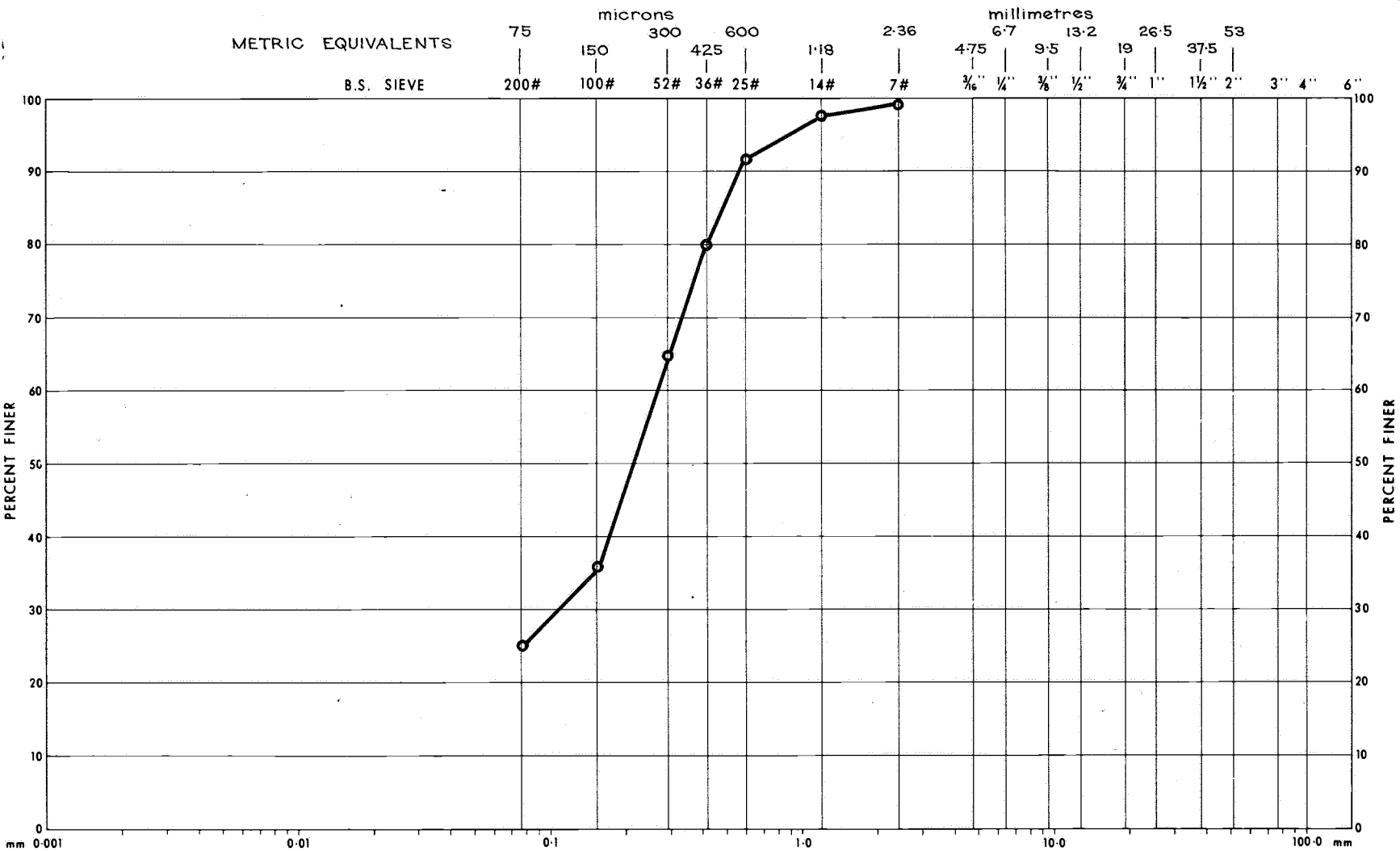
TESTING OF MATERIALS

FIG: 27

Ckd:

BULLDOG SHALE (Kib) - Ch. B144.0 km.

Drg No. S11387



|                               |      |           |             |             |             |               |               |
|-------------------------------|------|-----------|-------------|-------------|-------------|---------------|---------------|
| CLAY                          | SILT | FINE SAND | MEDIUM SAND | COARSE SAND | FINE GRAVEL | MEDIUM GRAVEL | COARSE GRAVEL |
| CLASSIFICATION FROM B.S. 1377 |      |           |             |             |             |               |               |

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 17th Sept. '74

Drm: J. Williams

TESTING OF MATERIALS

FIG: 28

Ckd: DRIFT SAND/RESIDUAL SOIL - ch. B.160-0km  
(qrs/qpr)

Drq No. S11388

# MODIFIED A.A.S.H.O. COMPACTION TEST

## ATTERBERG LIMITS:

Liquid Limit 20 % Plasticity Index 8  
Plastic Limit 12 % Linear Shrinkage %

## COMPACTION:

Maximum Dry Density 130 lbs/cub.ft.  
Optimum Moisture Content 8.0 %  
Natural Moisture Content NA %  
Compacted Dry Density at Natural Moisture Content  
lbs/cub.ft.  
%AASHO

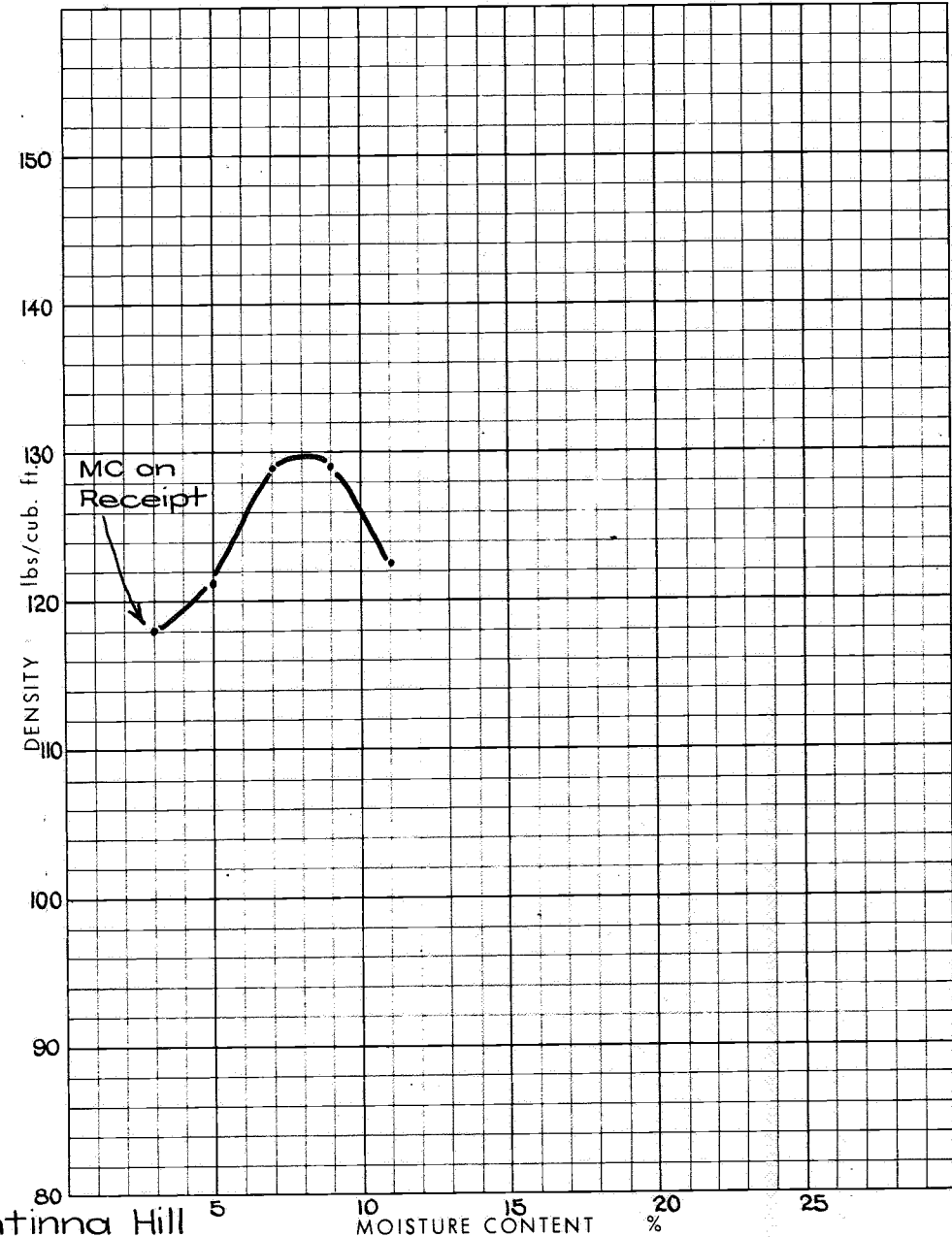
CALIFORNIAN BEARING RATIO: (estimated)  
14.5 %

## COMMENTS:

Saline water used

DESCRIPTION OF MATERIAL  
Red brown silty sand

SAMPLE No. B.160.0 km LOCATION Wintinna Hill



ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 17th Sept. 74

Drm: J. Williams

ENGINEERING PROPERTIES OF MATERIALS

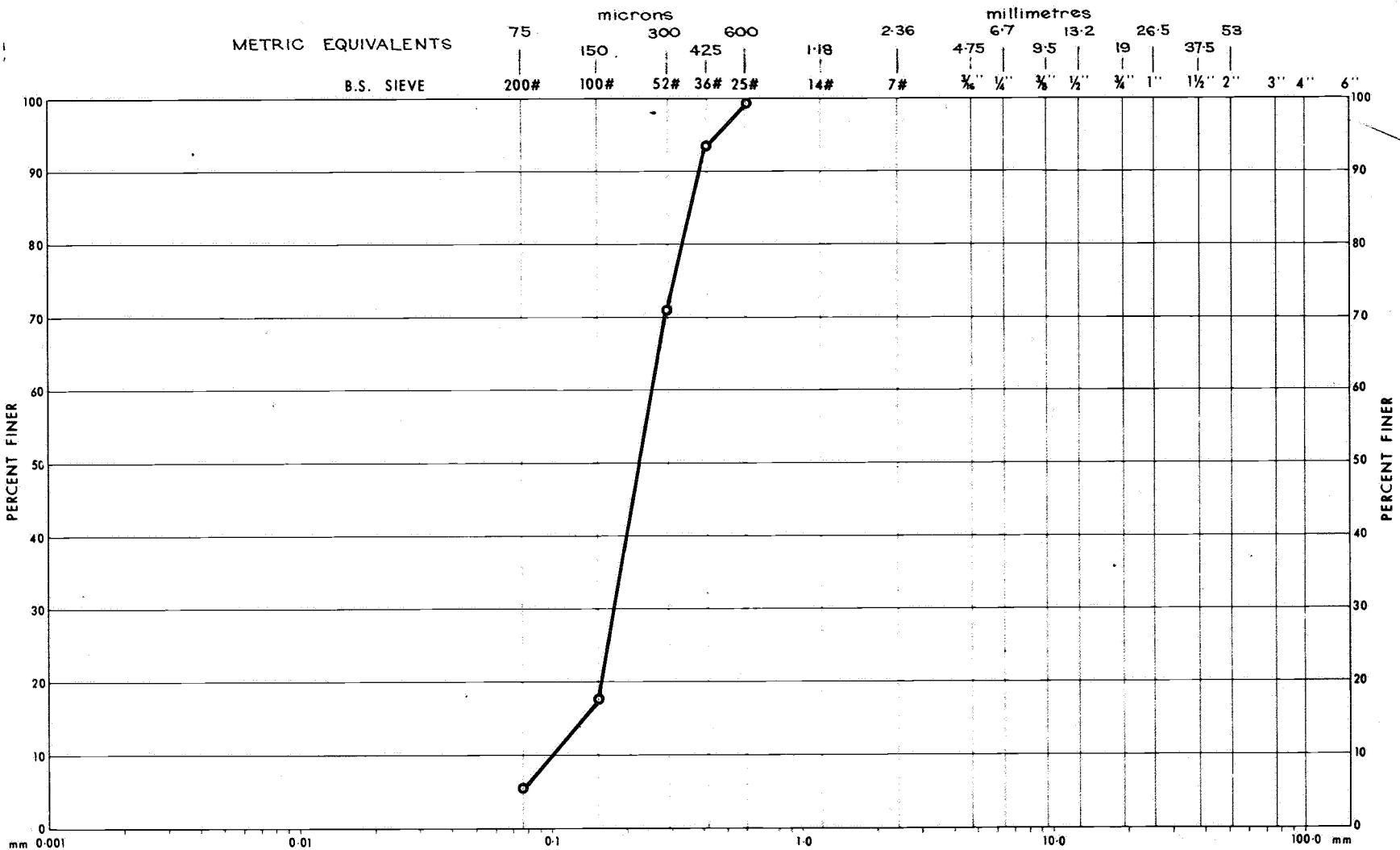
FIG: 29

Ckd:

DRIFT SAND/RESIDUAL SOIL (Qrs/Qpr)

Drg No. S11389





|                               |      |           |             |             |             |               |               |
|-------------------------------|------|-----------|-------------|-------------|-------------|---------------|---------------|
| CLAY                          | SILT | FINE SAND | MEDIUM SAND | COARSE SAND | FINE GRAVEL | MEDIUM GRAVEL | COARSE GRAVEL |
| CLASSIFICATION FROM B.S. 1377 |      |           |             |             |             |               |               |

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 17th. Sept. '74

Dra: J. Williams

TESTING OF MATERIALS

FIG: 30

Ckd:

DUNE SAND (Qrd)-ch B. 164.7 km

Drg No. S11390

# MODIFIED A.A.S.H.O. COMPACTION TEST

ATTERBERG LIMITS: Non-plastic

Liquid Limit % Plasticity Index

Plastic Limit % Linear Shrinkage %

## COMPACTION:

Maximum Dry Density 116 lbs/cub.ft.

Optimum Moisture Content 10 %

Natural Moisture Content NA %

Compacted Dry Density at Natural Moisture Content  
lbs/cub.ft.  
%AASHO

CALIFORNIAN BEARING RATIO: (estimated)

13.7%

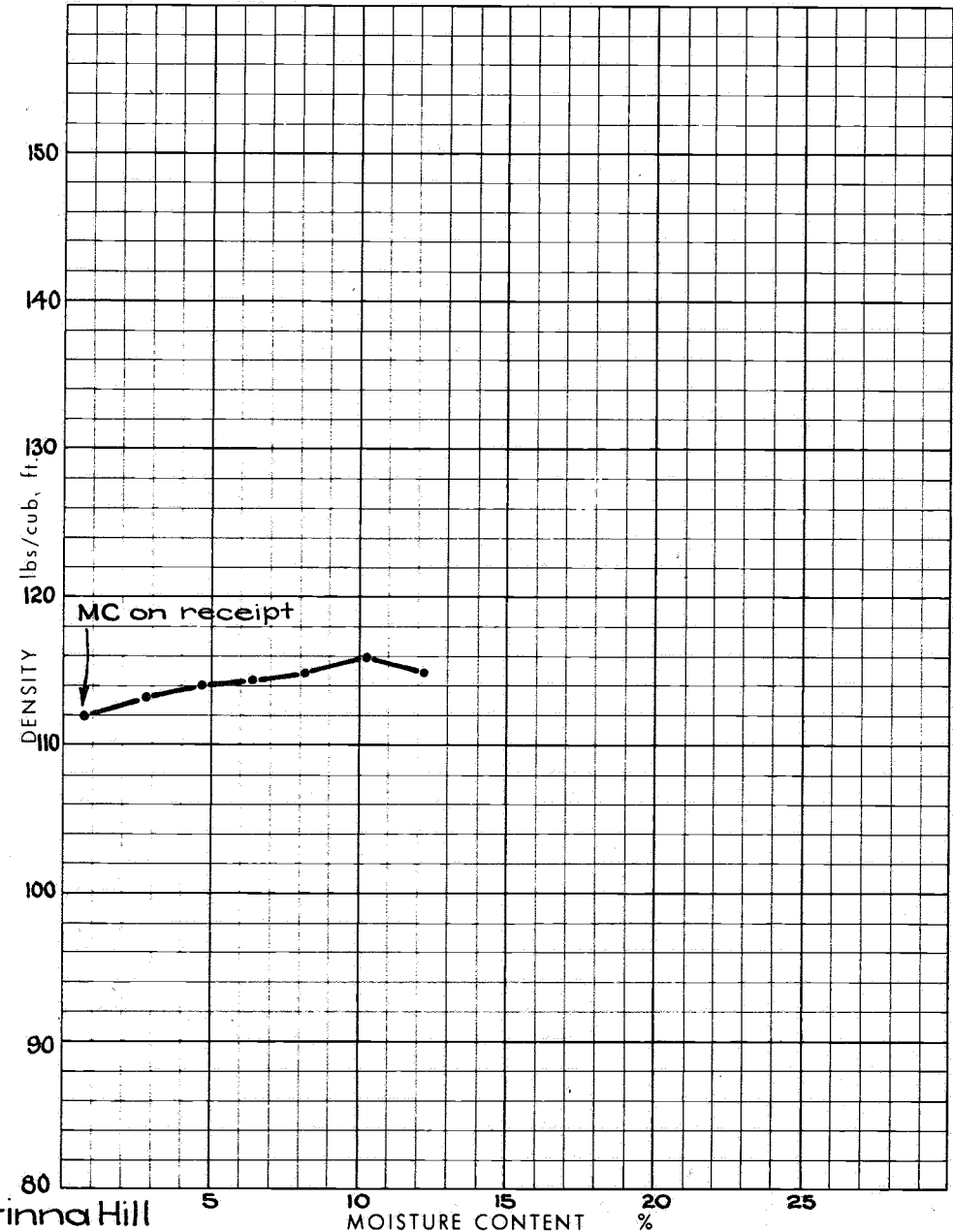
## COMMENTS:

Saline water used.

DESCRIPTION OF MATERIAL  
Fine to medium sand

SAMPLE No. B.164.7

LOCATION Wintinna Hill



ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J. Selby

Drm: J.W.

TARCOOLA — ALICE SPRINGS RAILWAY

ENGINEERING PROPERTIES OF MATERIALS

DUNE SAND (Qrd)

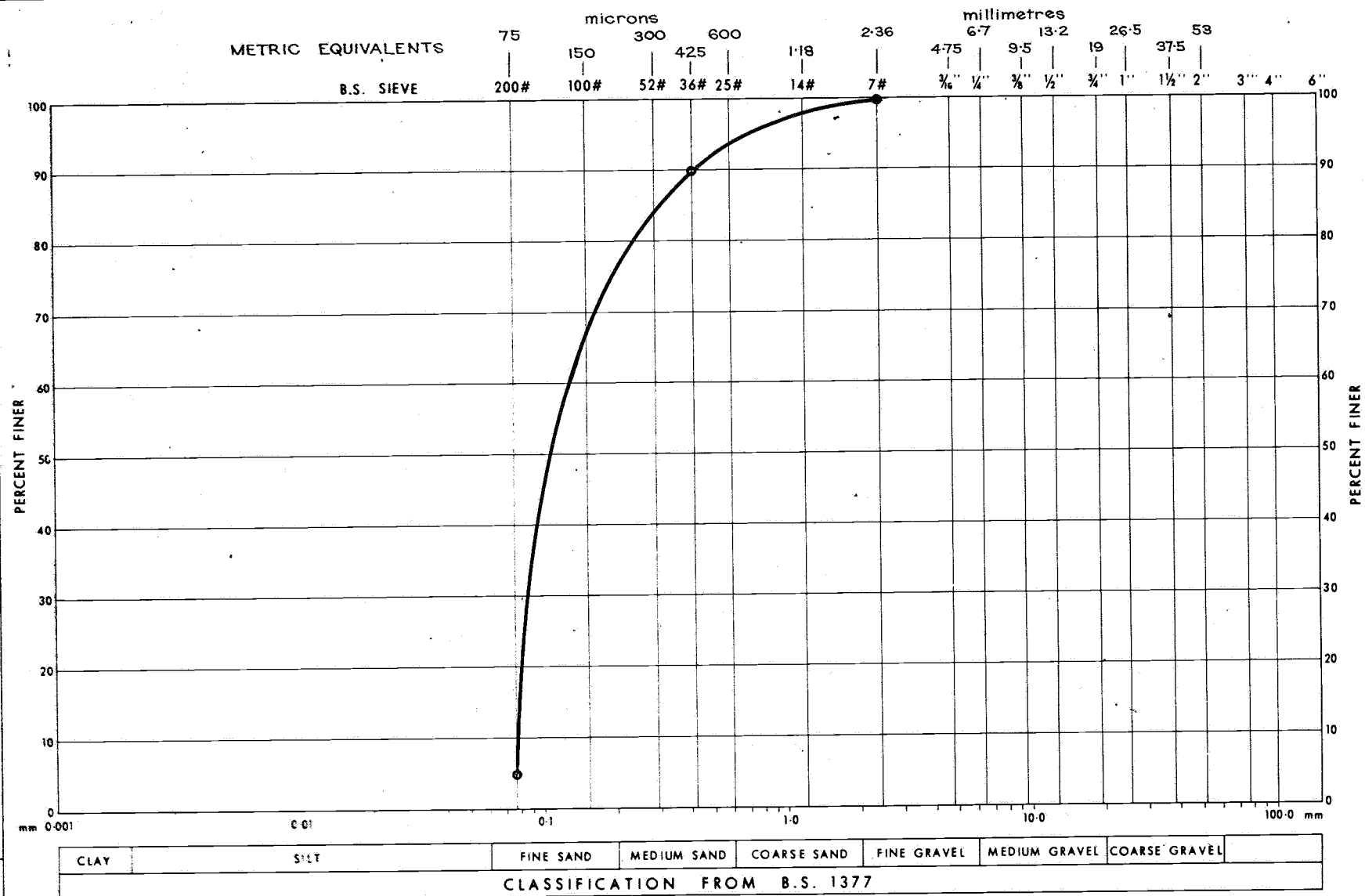
Date: 17th Sept '74

FIG: 31

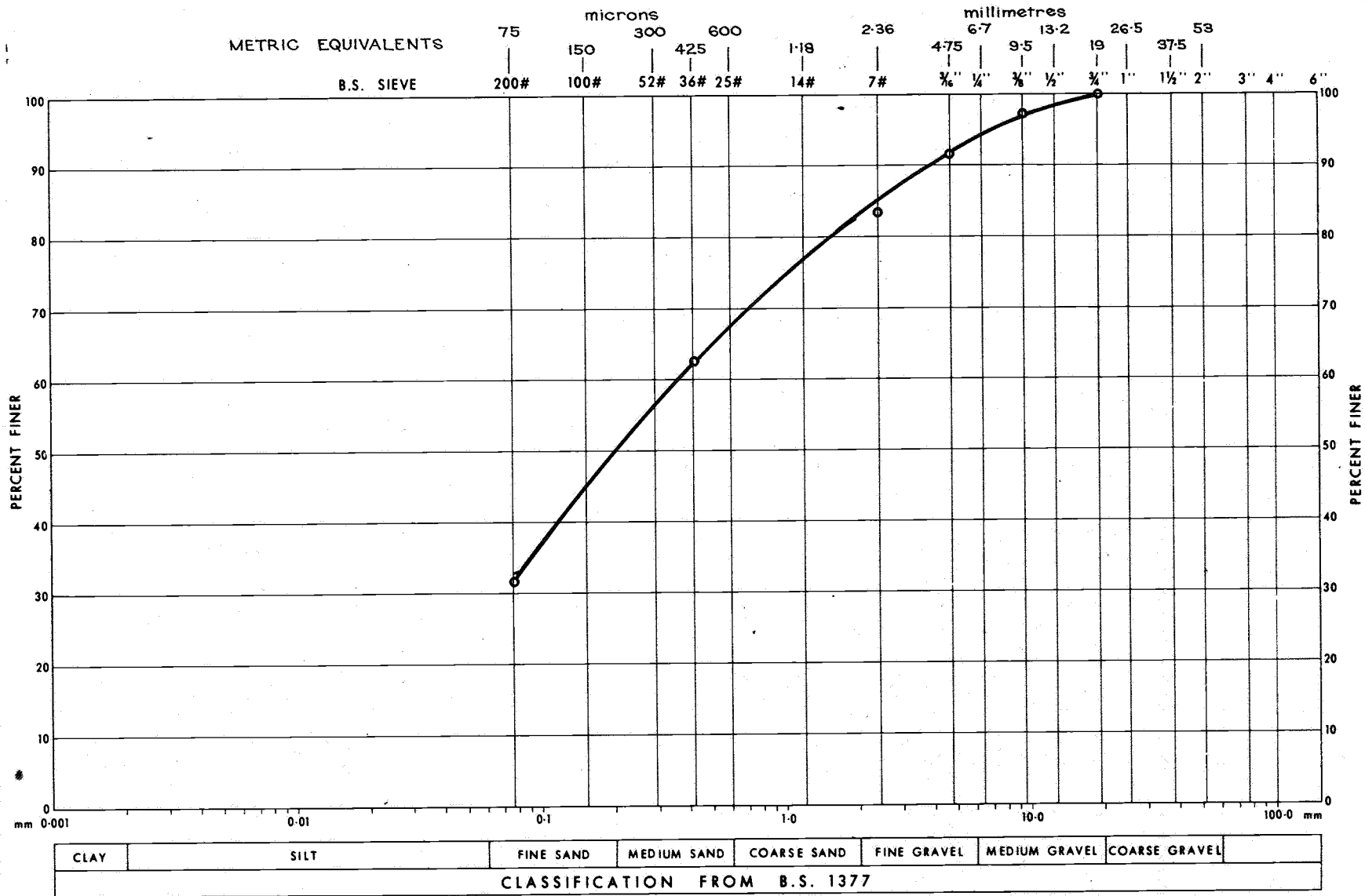
Ckd:

Drg No. S11391

|                                |                                |                                       |  |
|--------------------------------|--------------------------------|---------------------------------------|--|
| ENGINEERING GEOLOGY<br>SECTION |                                | DEPARTMENT OF MINES — SOUTH AUSTRALIA |  |
| Compiled: GHMCN                | TARCOOLA — ALICE SPRINGS       | RAILWAY                               |  |
| Drn: J. Williams               | TESTING OF MATERIALS           |                                       |  |
| Ckd:                           | DUNE SAND(qrd) - Ch. B179.0 km |                                       |  |
| Date: 5.3.75                   |                                | Scale: As shown                       |  |
| FIG: 32                        |                                | Drg No. S 11392                       |  |



NOTE - Compaction curve not available. Results of Standard A.A.S.H.O. test, M.D.D. 114 p.c.f., O.M.C. 11%  
Non-plastic, L.S.O, est. C.B.R. 14%, field M.C. 0.5%



NOTE : Compaction curve not available. Results of Standard A.A.S.H.O. test, M.D.D. 137 p.c.f., O.M.C. 9.0%  
 L.L. 20, P.L. 9, P.I. 11, L.S. 5, est. C.B.R. 19%, field M.C. 3.5%.

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: CHMCN

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 5-3-75

Drawn: J. Williams

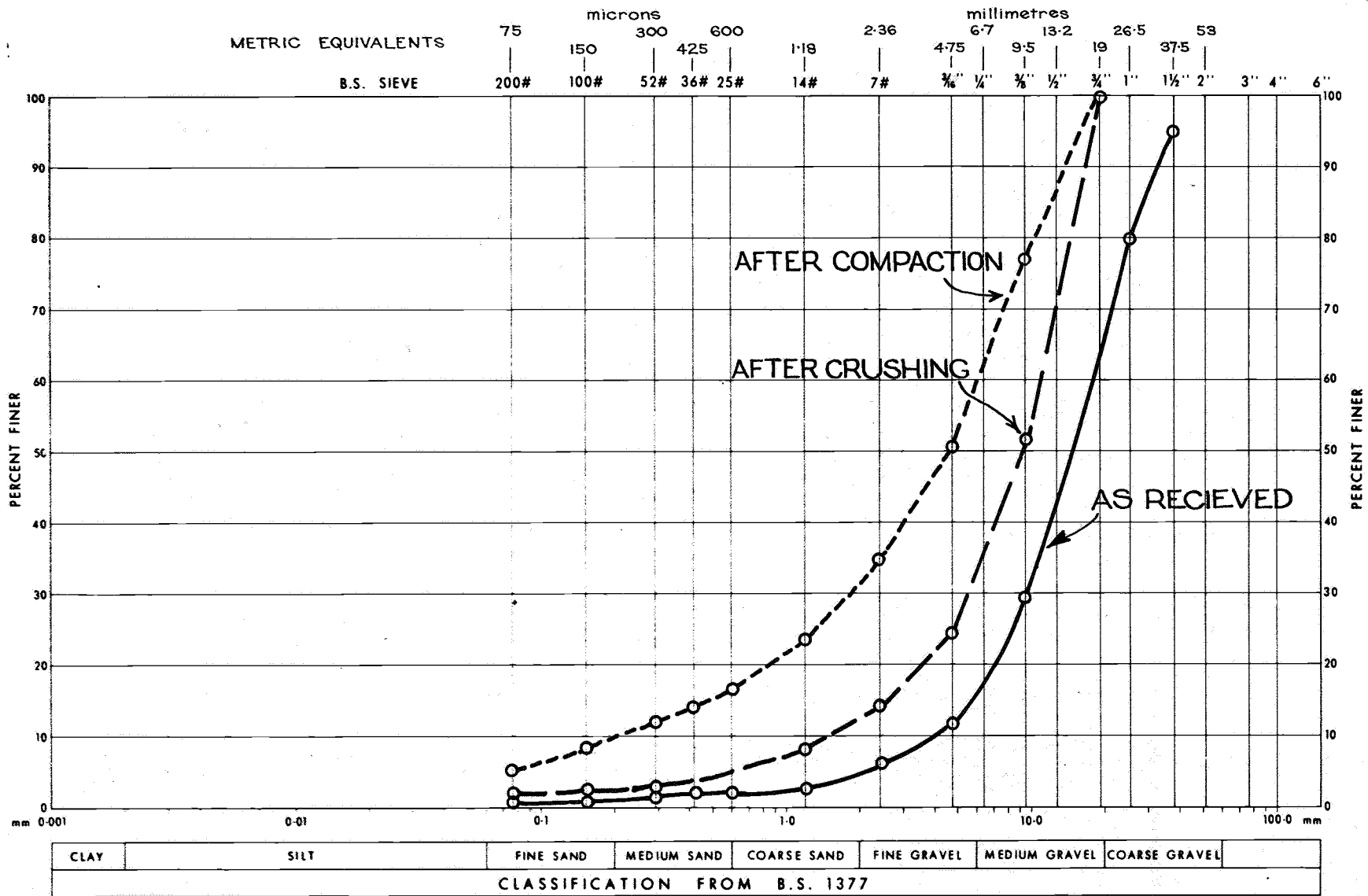
TESTING OF MATERIALS

FIG: 33

CKD:

RESIDUAL SOIL (qpr) - Ch. B190.0 km

Drp No. S11393



NOTE : As-received material crushed to pass 3/4" (19mm.) sieve.  
Results of one-point modified compaction test, M.D.D.103 p.c.f. 0.M.C.12%

ENGINEERING GEOLOGY  
SECTION

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Scale: As shown

Compiled: J.Selby

TARCOOLA — ALICE SPRINGS RAILWAY

Date: 17th Sept. '74

Dra: J. Williams

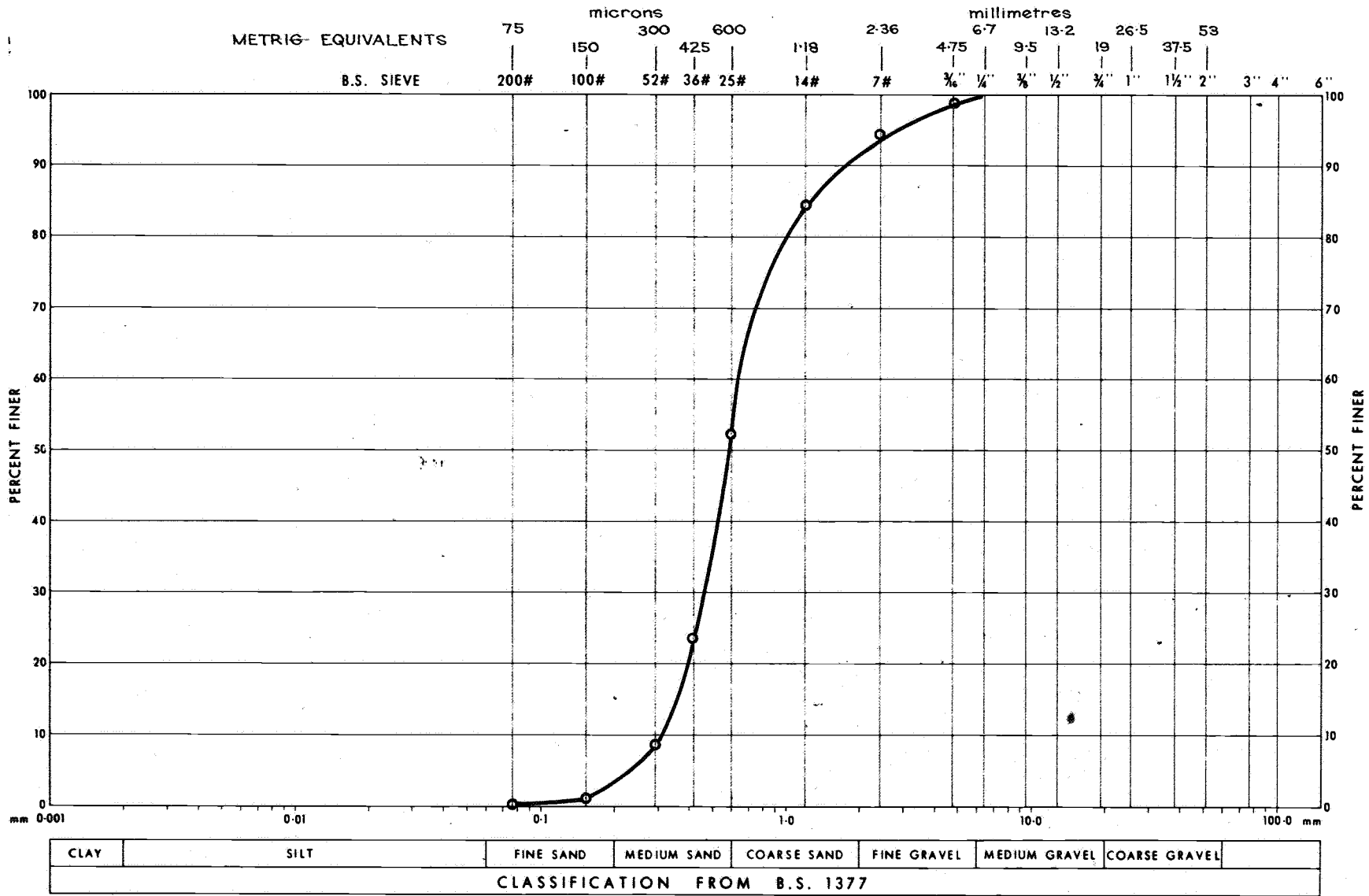
TESTING OF MATERIALS

FIG: 34

Ckd:

BULLDOG SHALE(klb) Mt Willoughby  
Borrow Pit

Drg No. S11394



NOTE :- Proposed as a source of fine concrete aggregate.

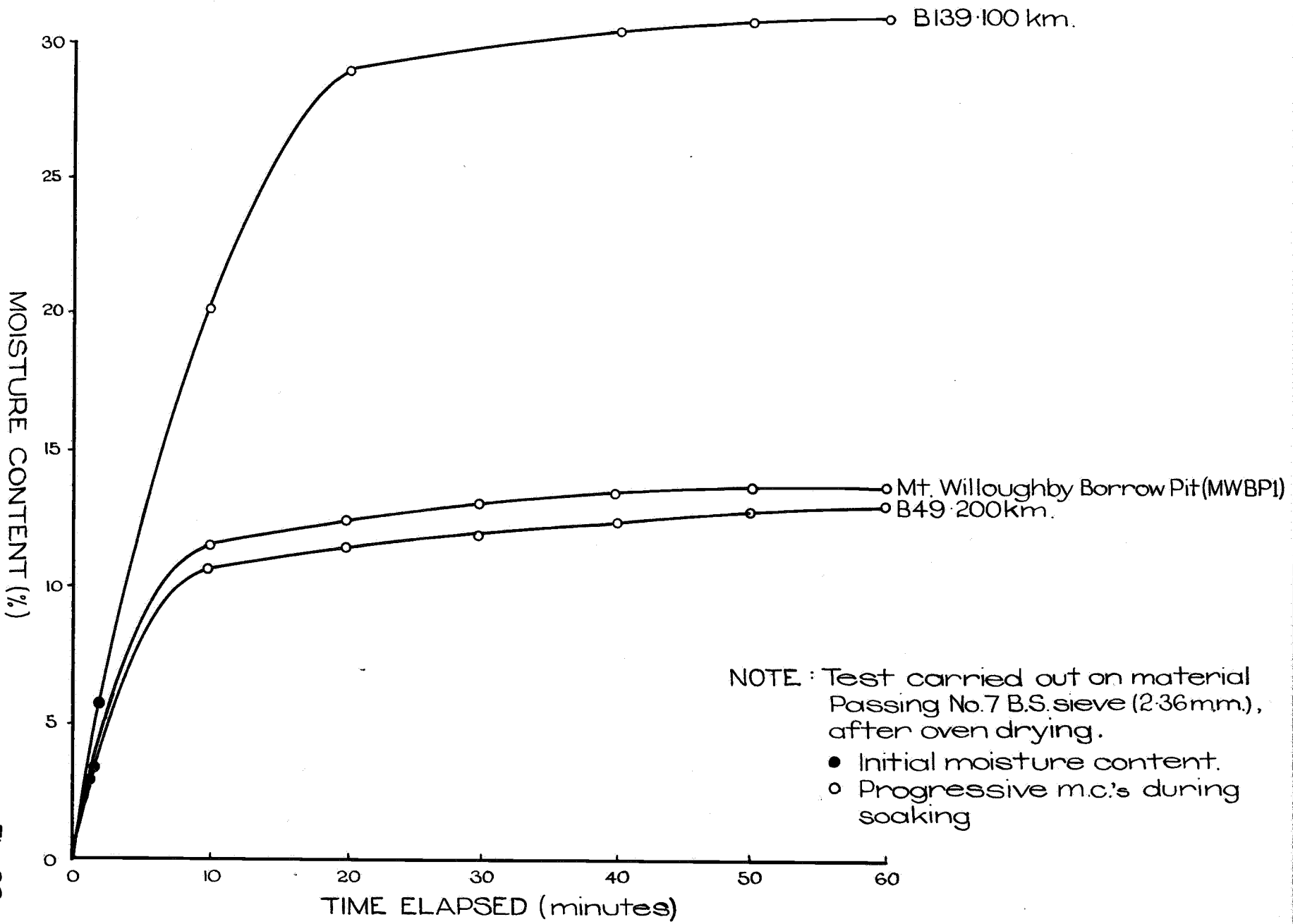


Fig. 36

DEPARTMENT OF MINES — SOUTH AUSTRALIA

|             |                                |              |
|-------------|--------------------------------|--------------|
| Drn. G.M.M. | TARCOOLA-ALICE SPRINGS RAILWAY | SCALE:       |
| Tcd. J.W.   | WATER ABSORPTION TESTS ON      | S11396       |
| Ckd.        | BULLDOG SHALE (K1b)            | DATE: 5-3-75 |
| Exd.        |                                |              |