

# DEPARTMENT OF MINES SOUTH AUSTRALIA



GEOLOGICAL SURVEY ENGINEERING DIVISION

PROPOSED BUILDING - CHEST CLINIC, NORTH ADDED TOWN ACRE 28, HD. ADELAIDE

FOUNDATION INVESTIGATIONS

**DESIGN STAGE** 

Client: Public Buildings Department

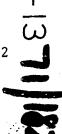
by

R.F. JEUNE

GEOLOGIST

ENGINEERING GEOLOGY SECTION

Rept.Bk.No. 71/182



### DEPARTMENT OF MINES SOUTH AUSTRALIA

# PROPOSED BUILDING - CHEST CLINIC, NORTH ABELATOE TOWN ACRE 28, HD. ADELAIDE

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APPENDIX - Logs of cable - tool holes CH 1 and CH 2 and explanatory notes.

#### FIGURES

Fig. No.	<u>Title</u>	Plan No.
1	Chest Clinic - North Terrace Site Plan	S954 <b>3</b>

Rept.Bk.No. 71/182 G.S. No. 4751 D.M. No. 961/70

### DEPARTMENT OF MINES SOUTH AUSTRALIA

Rept.Bk.No. 71/182 G.S. No. 4751 D.H. No. 961/70

PROPOSED BUILDING - CHEST CLINIC, NORTH ADELATE
TOWN ACRE 28, HD. ADELAIDE

#### FOUNDATION INVESTIGATIONS

#### DESIGN STAGE

Client: Public Buildings Department

#### SUMMARY AND CONCLUSIONS

Two cable tool drill holes, 60.5 and 66 ft. deep have been put down at the site of a proposed 2 storey building adjacent to North Terrace, Adelaide. These indicated a succession up to 55 ft. thick of clays, silts, sands and gravel (possible alluvial deposits of the old River Torrens) overlying at least 8 ft. of sand (Carisbrook Sand). A zone of lime accumulation of variable strength, and about 10 ft. thick, occurs immediately beneath the topsoil. The clays beneath this zone are of moderate plasticity at a moisture content approximately equal to plastic limit and are stiff. They appear to be subject to some shrinkage and swelling movements with changes in moisture content.

Groundwater was cut at several depths in each hole. The water cut at a depth of 8 to 9.5 ft. appears to be perched water, connected to the ground surface.

The clays beneath the zone of lime accumulation appear suitable for foundations, either for piers, or driven or cast-in-situ piles. No significant weaker soil horizons are likely to occur beneath this clay.

However, care should be taken during construction to prevent access of water into these clays as this could cause decrease in strength and also instability of any unsupported excavation walls.

#### INTRODUCTION

Two cable-tool foundation investigation holes were drilled by Mines Department personnel to Public Buildings Department purchase order C145295.

Locations are shown in Figure 1. These holes were requested in order to determine foundation conditions beneath a proposed 2 storey building to be constructed at the site as a Chest Clinic.

Hole CH1 drilled between 8th and 15th October, 1971 reached a depth of 66 ft. Hole CH 2 drilled between 15th and 18th October, 1971 reached 60.5 ft. Scaled tube samples were recovered and standard penetration tests performed in both holes at nominal 5 ft. intervals. By arrangement with the client, the Geomechanics Division of the C.S.I.R.O. retained selected samples for investigation into soil suction phenomena. Geological logs of each hole are included in the Appendix.

#### REGIONAL GEOLOGY

The Adelaide geological sheet (1:63,360 series) shows the site to be located on the Para Fault Block, with 10 to 20 ft. of recent alluvial clays and sands overlying approximately 400 ft. of Pleistocone mottled clays, sands and gravels. A small area of Tortiary sandy linestone is mapped immediately north of the site. Recent flood plain deposits are shown within the valley of the Torrens River.

#### SITE GEOLOGY

#### Topography

The site is flot, and covered by buildings, gardens, lawn, and gravel paths. Several large trees are growing near the rear boundary fence.

#### Soil and Rock Types

All geological observations were limited to examination of cores from the 2 boros.

The geological succession as indicated by the drill cores is summarized in Table 1.

TABLE 1
Geological Succession

Dep <b>fro</b> n	th (ft.)	Thickness Age (ft.)		Doscription	
4.5 044				Dribbin (Friedrick of Marie Commission of Marie Fredrick)	
0	4 to 5	4 to 5	Recent	Fill and topsoil	
4 to 5	Approx.15	Approx.10	Pleistocene to Recent	Zone of limo accumulation	
Approx.15	37.5 to 42	24 to 27	Pleistocene to Recent	Silty clay, mottled	
37.5 to 42	57 to 58.5	15 to 20	Pleistocene to Recent	Interbedded clays, silts and sands	
57 to 58.5.	Groater than 60 to 67	Greater then 8	PPlio-Plioste- cone	Sand (?Carisbrook Sand)	

The soils between depths of 5 and 58.5 ft. are thought to be alluvial deposits related to the old Torrens River, but may be slightly atypical soils of the Hindmarsh Clay Formation.

#### GROUNDWATER

Details of ground water observations made during drilling are shown in Table 2.

TABLE 2
Details of Ground Water Observations

Hole	Water Cut ft.	Static Level ft.	Estimated Flow galls/hr	рН	Salinity p.p.m.
CH 1	8	7.5	50	8.0	1385
44	39	28	100	8.0	1900
	50	28	100	7.5	1900
11	58	28	100	8.0	2155
CH 2	9.5	9.5	50	8.0	1900
11	43	29	100	8.0	2755
£1	55	29	100	8.0	3100

The perched water table between 8 ft. and 9.5 ft. appears to be in directed communication with the surface and could be expected to show a seasonal rise and fall. Suitable allowances should be made in designs incorporating basements, and in the planning of excavations. All deeper waters encountered rose considerably when tapped, and all stabilized at substantially the same level. However, salinities and pH values suggest that they may not all be directly interconnected.

#### DISCUSSION

#### Recent fill and topsoil

These are rather variable gravelly and sandy silts and clays, in part organic. This unit has probably been substantially modified by man, and its disturbed and variable nature renders it unsuitable for foundations.

#### Zone of lime accumulation

This is a zone of calcarcous silt, silty sand, and gravel. The presence in hole CH 2 of large (7cm) angular, tabular fragments of strong calcrote,

coupled with relatively high resistance to penetration, suggests that lenses of strong (Table 3) calcrete may be present, but are likely to be irregular in distribution. The lower layers are saturated, but the texture of the whole unit is such as to suggest that some collapse may occur under other than very slight loads.

The irregular distribution of lime and of calcrete and the possibility of collapse under load suggests this unit is unsuitable foundations.

#### Mottled silty clay

This is a stiff grey and orange brown mottled silty clay with occasional thin lime accumulations along joint planes, and associated with bedding planes. The upper 1 to 2ft. is slightly calcareous. Occasional slickensided shear planes, and a weakly developed fine nutty structure suggest that some expansion and shrinkage may be taking place with changes in noisture content.

Since correlation with the Hindmarsh Clay Formation, which occurs beneath much of the Adelaide City Area, and the engineering properties of which are well-known, is uncertain, it is suggested that the relevent physical properties of this material should be checked by laboratory tests on the scaled tube samples.

#### Interbedded clays, silts, and sands

These are dominantly grey clays, silts, and sands, in part micaceous. Comparisons between holes CH1 and CH2 show only a general similarity in sequences, suggesting that individual beds may be rather variable and discontinuous.

### TABLE 3 CLASSIFICATION OF ROCK CONDITIONS AND STRENGTH OF ROCK SUBSTANCE

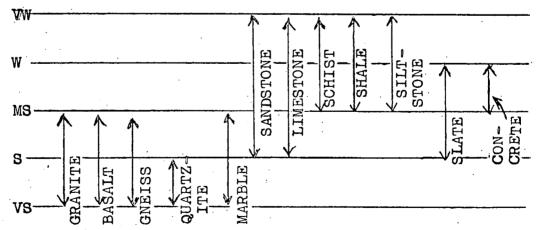
#### 1. ROCK CONDITION TERMS

TERM	ABBRN	73	DEFINITION
Fresh	<b>(</b> F)	• •	hows no effects of chemical
Chemically Decomposed	(D)		s affected by chemical decompos- the exact process is not obvious.
Chemically Weathered	(W)	Substance s	hows effects of chemical decom-
			ocesses which have occurred due and near-surface agencies such as
Chemically Altered	(A)	Substance so osition pro	hows effects of chemical decomp- cesses which have occurred due
Extremely (Decomposed (Weathered (Altered	(XX)	Substance h shows fabri be remoulde	or volcanic fluids. as been reduced to material which c of original rock, but which can d, i.e. soil substance. (Classi- fied System).

#### 2. CLASSIFICATION OF ROCK SUBSTANCES BY UNCONFINED COMPRESSIVE STRENGTH

TERM	ABBRN	UNCONFINED COMP	RESSIVE STRENGTH (1b/sq.in)	
Very weak	VW	<u> </u>	≤ 1000	
Weak Medium strong	W MS	70 <b>-</b> 200 200 - 700	1000 <b>- 3000</b> 3000 <b>- 10,</b> 000	
Strong Very strong	S Vs	700 <b>-</b> 1800 >1800	10,000 - 25,000	

### RANGE OF STRENGTHS OF SOME COMMON ROCK SUBSTANCES IN THE FRESH STATE\*



\*Samples of fresh rock tested to Australian Standard, For rocks showing planar anisotropy the long axis of the sample is normal to fabric planes.

#### 3. EXAMPLES OF USE OF CLASSIFICATION

Geological Name	Rock Condition Term	Strength Term
Granite	Fresh	Strong
Granite	Weathered	Medium Strong
Schist	Fresh	Weak
Schist	Altered	Very Weak

#### Sands - (?Carisbrook Sand)

This is a dirty sand with rare thin gravel bands, intercepted at the bottom of both holes. This unit is likely to extend for several more feet and no weak horizons are likely to occur.

RFJ: CMH 10.11.1971

#### **APPENDIX**

Logs of Cable Tool Drill Holes CH 1 & CH 2, and explanatory notes.

#### APPENDIX

#### LOGS OF CABLE TOOL HOLES AND EXPLANATORY NOTES

#### NOTES ON DRILLING PROCEDURES

#### Equipment

The drilling is carried out with a cable tool drilling plant using sampling tubes attached, through a vacuum head, to the sampling tools (Figs. 1 and 2).

## Sampling Procedures S.A. Samples

To obtain, for logging purposes, an almost continuous series of samples, with a relatively small amount of sample disturbance, SA type samples are taken. These are obtained by driving an "S" tube, fitted with a Mark A shoe (Fig. 2), into the material to be sampled.

The assembly is lowered carefully to the bottom of the hole, and the tube driven exactly 1 foot, and the number of blows required for the 1 foot of penetration recorded.

The sample, or core, is extruded from the sampling tube using an hydraulic ram. The extruded core is sealed in a labelled plastic bag and stored in a core box (Fig. 3).

The hole is reamed with a "D" or "E" shoe (Fig. 2) and then the next sample is taken, using the same procedure as above. Thus the hole proceeds by alternate sampling, reaming (and where required, casing) operations, and the samples form a continuous record of the materials penetrated except for a few inches which may be lost between samples during reaming operations.

SA sampling equipment is a composite sampler for simple class sampling. Details are as follows:-

#### "S" SERIES CUTTING SHOES

MARK	FEATURES	USES
A	Inside clearance 3%. Area Ratio 33%.	Continuous open-tube sampling in strong soils, in which it causes little deformation. Samples extruded and used for logging purposes. Hole is reamed after each sample.
D .	Shoe belled out to 4,29/32 in. (just greater than outside dia. of vacuum head)	Continuous open tube sampling where considerable deformation of sample is permissible. Essentially self-reaming.

MARK

FEATURES

USES

E Shoe belled out to 5 7/16 in. (just less than internal dia. of 6 in casing)

Cleaning hole and reaming out hole.

#### Sealed Tube (LB) Samples

Sealed tube samples, for laboratory testing, are taken at various intervals during drilling. These are obtained by driving an "L" type sampling tube with a Mark B cutting shoe (Fig. 4)adistance of 1 foot into the material to be sampled.

Before the sample is taken the hole is cleaned out to the depth specified. The hole is not reamed or cased for at least 1 foot from the bottom, however, because these operations can cause considerable disturbance in the soil below. The sampling assembly is lowered carefully to the bottom of the hole, the sampling tube driven exactly 1 foot, and the number of blows recorded.

The sample is sealed in the tube by inserting in each end, plastic seals with rubber sealing rings, and the tube is then labelled and stored in a Laboratory Sample Box.

LB sampling equipment is a composite sampler for obtaining samples with the least possible disturbance. Details are as follows:

Sampler tube -ASSAB tube cadmium plated "L" type 4.016 in I.D. 4.282 in O.D.

Mark B shoe - ASSAB tube, heat treated, cadmium plated Area ratio 15%

Inside clearance - mil O·4%

Outside clearance - nil Cutting edge angle - 7

#### Standard Penetration Test

The Standard Penetration Test (Terzaghi et al 1948) is used to test the in-situ density of sands and to give an indication of the consistency of clays, and compactness of silts. However the test results can be affected by several geological factors such as degree of cementation, and size and shape of grains. These factors should be taken into account in interpretation of results.

The equipment is illustrated in Fig. 5 and consists of a 2in. diameter, sampling spoon (tube) and a hammer of standard weight (140 lbs).

With the equipment assembled as in Fig. 5 the hammer is allowed to fall on to the drill rods until the sampling shoe has penetrated 6 in. into the soil. The Standard Penetration Test is the number of blows (N) required to produce the next foot of penetration.

#### NOTES ON DRILL LOG SHEETS

The logs are plotted on a standard cable-tool log form.

Near the centre of the form a graphic log of the materials encountered is shown.

In the column to the right of the graphic log, the soils are classified and described according to the Unified Soil Classification (U.S. Dept. of Interior, Bureau of Reclamation 1966) as shown on Figure 6.

To the left of the graphic log is a geological description of the materials sampled. This includes:-

......Geological age

.....Soil unit name

Printed vertically

.......Type of material

......Mineral composition

.......Grain shape

......Cementation

.....Organic materials

Water levels are indicated by a small arrow with the date at which the observation was made.

In the blows per foot column, a continuous histogram is made of the number of blows required to drive the sampling tube through each foot of material. A hatching code is used to distinguish various types of sample. This code is reproduced at the bottom of each log sheet.

In the column on the far right of the log sheet, readings of unconfined compressive strength (qu) made with a Soiltest Penetrometer, are recorded. The readings are plotted as a histogram. The Soiltest Penetrometer only gives true values of qu when used in clays in which  $\emptyset = 0$ .

#### REFERENCES

TERZAGHI, K. and PECK, R.B., 1948. Soil Mechanics in Engineering Practice. John Wiley and Sons.

UNITED STATES DEPARTMENT OF THE INTERIOR, BUREAU OF RECLAMATION, 1966. Earth Manual, 2nd Edition.

#### DESCRIPTIVE TERMS

#### 1. CLAY SOILS

#### CONSISTENCY

CONSISTENCY	SYMBOL	UNCONFINED COMPRESSIVE STRENGTH (kg/sq. om)	field test n
Very Soft	v.s.	less than 0.25	Easily penetrated several 2 inches by fist.
Soft	S	0.25 to 0.5	Easily penetrated several 2 to 4 inches by thumb.
Firm	F	0.5 to 1.0	Can be penetrated several 4 to 8 inches by thumb with moderate effort.
Stiff	St	1.0 to 2.0	Readily indented by the 8 to 15 thumb but penetrated only with great effort
Very Stiff	۷۰St.	2.0 to 4.0	Readily indented by thumb 15 to 30 nail.
Hard (Extremely stiff)	Н	over 4.0	Indented with difficulty 30 and by thumb nail. over

Based partly on Terzaghi, K. and Peck. R.B. 1966. Soil Mechanics in Engineering Practice, Wiley - New York.

#### MOISTURE CONTENT

	Abl	rev	iati	on	Meaning
]	MC	~	IT		Moisture Content near liquid limit.
]	MC	<	LL		" less than liquid limit.
	MC	>	PL .		greater than plastic limit.
. 1	MC	· <u>~</u> .	PL		near " the transfer of the tra
. !	MC	«	T'		" " less or equal to plastic limit.
. 1	MC	<	PL.	1	" less than " "
	MC	<b>.&lt;</b> .<	PL.		much less than " "

#### 2. SILT SOILS

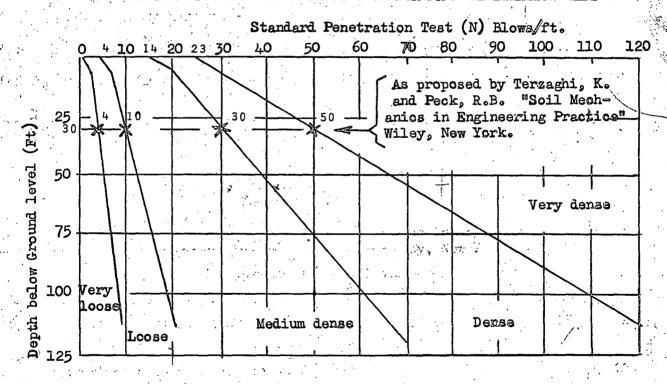
COMPACTNESS	SYMBOL	And Manager National Control
Loose	Ls	0 to 8
Moderately compact	MC	8 to 15
Compact	C	15 to 30
Very Compact	VC	greater than 30

#### 3. SANDS

#### CLASSIFICATION OF SANDS BY STANDARD PENETRATION TEST

The relative density of granular soils has been judged from the results of Standard Penetration Tests carried out by the procedure described by Terzaghi and Peck (1948) bearing in mind the limitations of the method as discussed by Gibbs and Holtz (1957). At all times the water in the drill hole was kept at the level of surrounding groundwater.

#### EFFECT OF OVERBURDEN PRESSURE ON STANDARD PENETRATION TEST

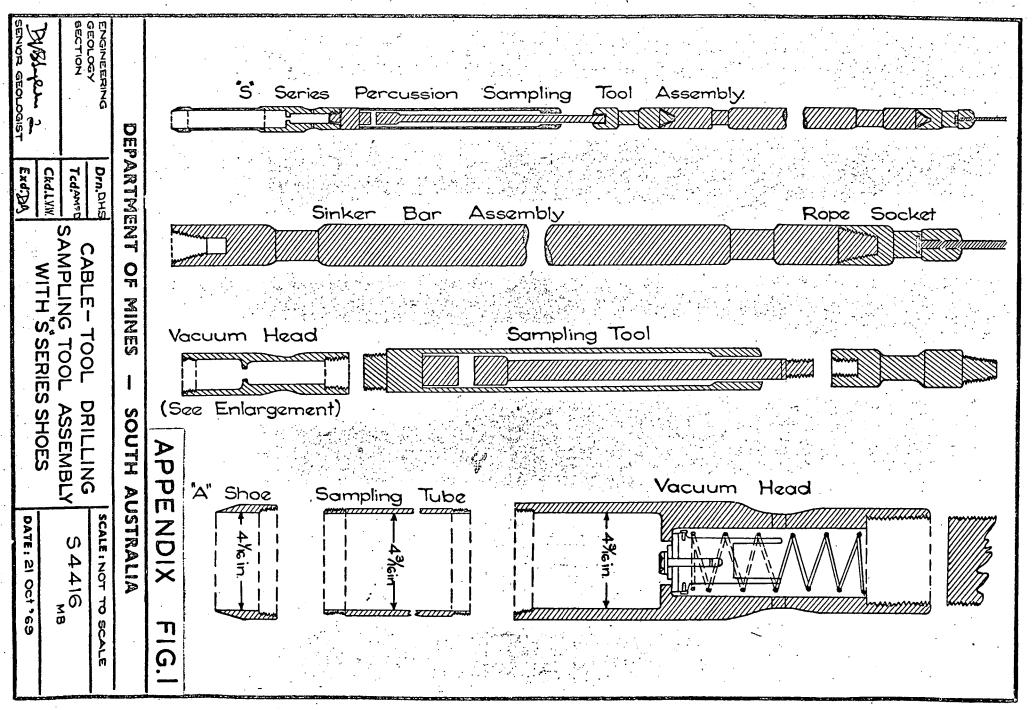


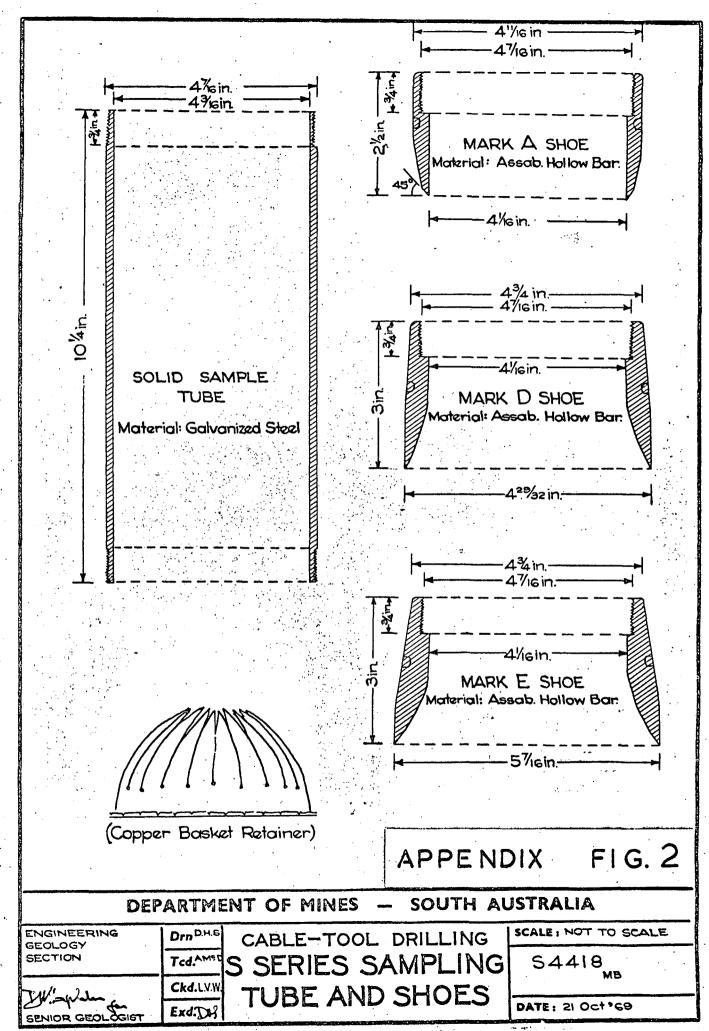
Based on Gibbs, H.J. & Holtz, W.G. (1957) "Research on Determining the Density of Sands by Spoon Penetration Testing" Vol. I Proc. 4th Int. Conf. SM & FE, London.

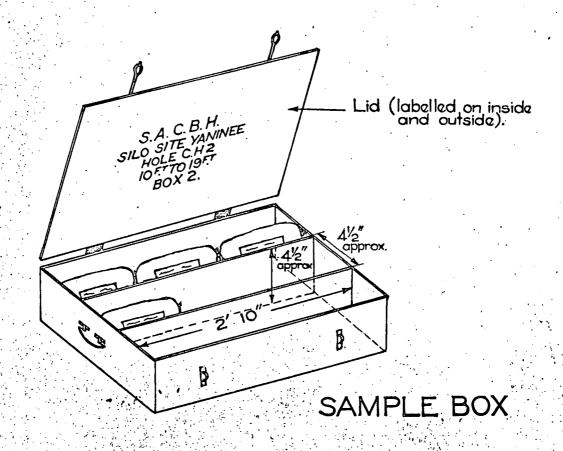
#### REFERENCES

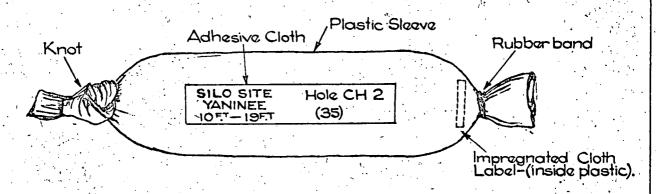
TERZAGHI, K., and PECK, 1948. "Soil Mechanics in Engineering Practice".
Wiley. New York.

GIBBS, H.T. and HOLTZ, W.G., 1957. Research on Determining the Density of Sands by Spoon Penetration Testing. Proc. 4th Inter. Conf. SM & FE, London, Vol. 9.



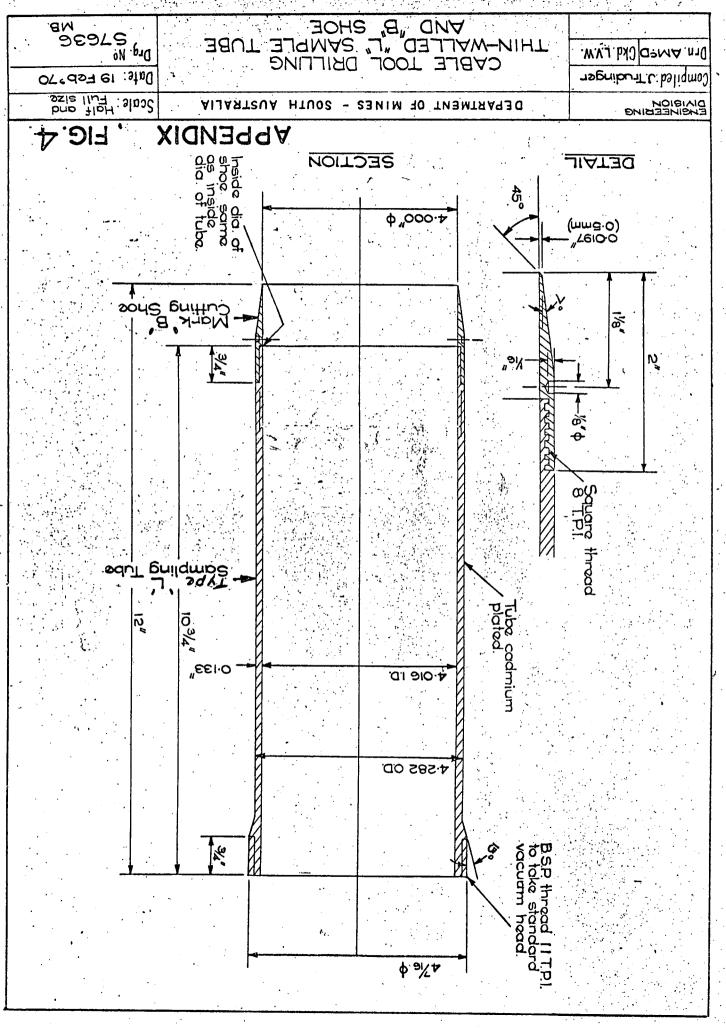


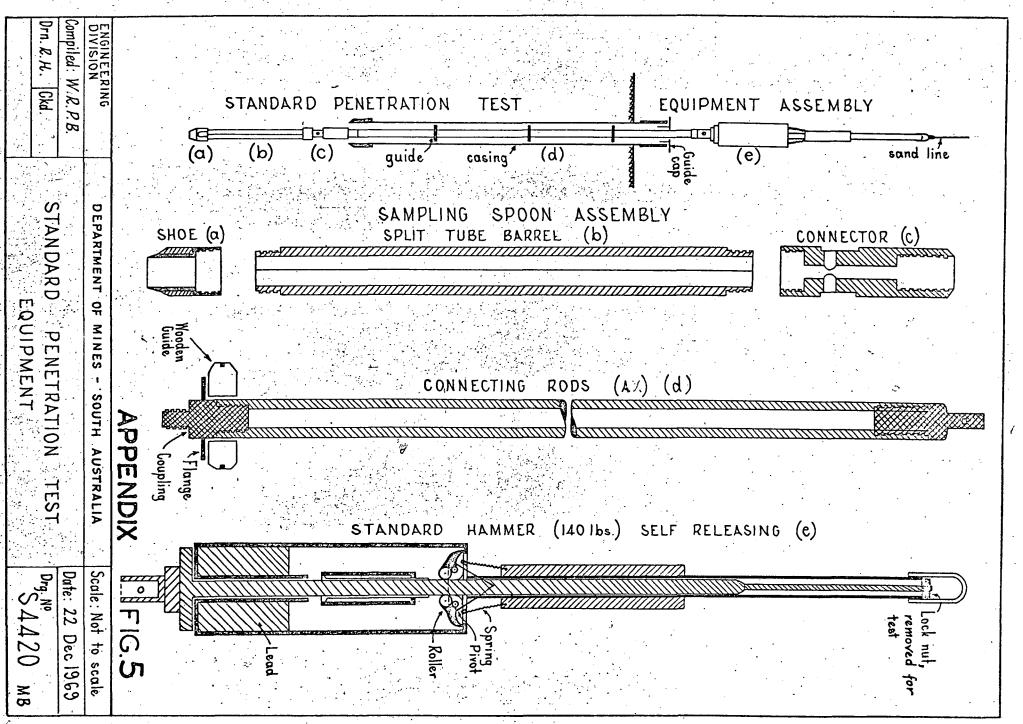




EXTRUDED SAMPLE SEALED IN PLASTIC SLEEVE.

	APPENDIX	FIG. 3
ENGINEERING DIVISION	DEPARTMENT OF MINES - SOUTH AUSTRALIA	Scale:
Compiled: WR.P.B		Date: 17 Dec 269
DrnAMSD. Ckd.	CABLE TOOL DRILLING LABELLING AND BOXING OF	Drg. Nº
	EXTRUDED SAMPLES	57580 MD





	, ENGINEERING GEOLOGY SECTION							SOIL	S CLASSIFICATION CHA	RT DEPARTMENT OF MINES SOUTH AUSTRALIA			
(E	xcluding parti		DENTIFICATION han 0.25 ft. and ba			estimate	d.weights)	GROUP SYMBOL	GROUP NAME and typical materials	LABOR	RATORY CLASSIFICATION CRIT	ERIA	
More than 50% of material is smaller than No 200 B.S. sieve size.	Markatara da a	CLEAN GRAVELS Little or no fines	Wide range in grain size and substantial amounts of all intermediate particle sizes				imounts	G W	GRAVEL, well graded; gravel sand mixtures, little or no fines	NDS SP	Cu = $\frac{D60}{D10}$ Greater than 4 Cc = $\frac{(D30)^2}{D100}$ Between one and 3		
			Predominantly one size, or a range of sizes, with some intermediate sizes missing.				es, with	G P	GRAVEL, poorly graded; gravel sand mixtures, little or no fines.		Not meeting all acadation requirements for		
		GRAVELS	Non-plastic fines - for identification see ML below.				1L below.	GM	GRAVEL, excess silty fines; poorly graded gravel - sand - sitt mixtures	ions. classified es, as follor vELS GP ine cases, u	Atterberg limits below "A" line or PI less than 4	Above "A" line wi between 4 and 7	
		Appreciable amount of fines	Plastic fines - for identification see C L below				w	GC	GRAVEL, excess clayey fines; poorly graded gravel-sand-clay mixtures		Atterberg limits above "A" line with PI greater than 7	borderline cases in ing use of dual sym	
	More than 50% of the coarse fraction is smaller than 2mm.	CLEAN SANDS Little or no fines	Wide range in gro all intermediate pa	_		tantial an	nounts of	sw	SAND, well graded; well graded sands, gravelly sands, little or no fines.	fract soils of fin GW GW GW GW	Cu = D60 Greater than 6  Cc = (D30) <sup>2</sup> Between one and 3  D10×D60	`	
			Predominantly one size or a range of sizes, with some intermediate sizes missing.				th some	SP	SAND, poorly graded; poorly graded sands gravelly sands, little or no fines	soil rained n tage F FINES n 12	Not meeting all gradation requireme	nts for SW	
		DIRTY SANDS Appreciable amount of fines	Non plastic fines - for identification see M L below				below	SM	SAND, excess silty fines; poorly graded sand - silt mixtures	Tringer per CEN	or PI less than 4	Above "A" line with between 4 and 7 conderline cases ing use of dual sy	
			Plastic fines - for identification see CL below					sc	SAND excess clayey fines; poorly graded sand-clay mixtures.	Coo ide Of Of Charles No Mo			
	FIELD INVESTIGATION PROCEDURES  on fraction smaller than 0.4 mm. (passing B.S. 36 sieve)  SOIL CAST SOILTHREAD SHINE DILATANCY ODOUR DRY STRENG							GROUP SYMBOL	GROUP NAME ( and typical moterials )	t pas n ac	··.		
	SILTS AND CLAYS Liquid limit less than 50	(wet soil)  Forms fragile Cracks form v kneaded while	cast Thick crumbly	None to very dull	Distinct	-	None to	ML	SILT SOIL low plasticity; inorganic silts and very fine silty or clayey sands, rock flour.	4		A.E.	
		ly without breaking	st maybe handled free vithout breaking Can be aded moist without pointed as fine as a lead pencil, but as to the hand.  St maybe handled free the hand to significant as fine as a lead pencil, but as fragile.			CL	CLAY SOIL, low plasticity; inorganic clays of low to medium plasticity, gravelly clays, sandy clays silty clays, lean clays	~ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Ен				
	l -		chesive Soft, weak here thread	None to very dull	Slight to distinct	Decayed organic matter	Low	OL	ORGANIC SOIL low plasticity; organic sitts and silt clays of low plasticity	SIZE	ОН		
	AND CLAYS Liquid limit more than	Moderately plastic cohesive. Materia adheres somewho hand	weak to medium thread. May be of to the crumbly.	Dull	None to slight	Not significan	Moderate Powdered soil feels floury	МН	SILT SOIL, high plasticity; inorganic silts, micaceous or diatomaceous fine sandy or silty soils elastic silts.	s a d	CL OL OF MH		
		Very plostic and tve. Material very to the hand. Gre to touch.	cohes- visticky can be rolled to asy to a pin point	Very glossy	None'	Strong earthy.	High to very high. Cannot be powdered by finger pressure	I .	CLAY SOIL, high plasticity; inorganic clays of high plasticity, for clays	\$   0   7	ML ML ML 10 20 30 40 50 60 70	80 90 100	
		Plastic and cohes Feels slightly sp Greasy to touch	weak to medium thread. Often soft and fibrous	+0 40 -4	None	Decayed organic matter	Moderate to high Powdered soil may be fibrous	ОН	ORGANIC SOIL, high plasticity; organic clays of medium to high plasticity.		10 20 30 40 50 60 70 LIQUID LIMIT  PLASTICITY CHART		
	ORGANIC SOILS Readily identified by colour, odour, spongy feel and frequently by fibrous texture.						uently	Pt ··	PEATY SOIL; Peat and other highly organic soils	FO	FOR LABORATORY CLASSIFICATION OF FINE GRAINED SOILS		

64-246 MG United States Department of the Interior,

Bureau of Reclamation "Earth Manual"

First Edition, Denver COLORADO 1960

FIG. 6

DEPARTMENT OF MINES - SOUTH AUSTRALIA CH HOLE PROJECT CHEST CLINIC LOG OF CABLE TOOL HOLE SERIAL No. NORTH TERRACE TOWN ACRE 28 R.L. Surface 40: 5 th FT FEATURE FOUNDATIONS HUNDRED. AQELAIDE R.L. Collar . LOCATION GROUNDS OF HOUSE CO-ORDS Datum FIELD TEST DATA SOIL DESCRIPTION GROUP DEPTH CONTENT CONTENT GRAPHIC GEOLOGICAL NOTES GROUP NAME BLOWS SOILTEST AND CLASSIFICATION TR'METER Units 米 1 Z 3 4 Unified Soil Classification, U.S.B.R. PER FOOT Earth Manual 2nd Edition 1966 20 40 60 80 III SILT SOIL, low plasticity, 20-30% Sono CLAY SOIL, high plasticity orange-brown, with scattered lime nodules up to Icm. SILT AND CLAY, ORGANIC SILT SOIL, low plasticity, Brown to red-brown, organic or rop, lime nodules 7714 トエエ -1 ML D 15 at base. SILT SOIL, low plasticity. Brown-grey calcareous silt with strong lime CALCAREOUS SILT I ML SPT 8 Blows nodules up to 2cm. at base. DLS AND SILTY SAND - ZONE of LIME ACCUMULATION SAND, excess silty fines. Grey calcareous silty sand with 10 I : , SM scattered strong lime accretions 51 o. I up to Icm. Slightly sticky when remoulded. \_ Gradational Change 2ft. Zone. Slightly calcareous. 15 Vertical joint with roots. CH CLAY SOIL, moderate plasticity. 20 Nutty, grey, silty clay with orange St 70 brown mottles. RECENT Icm. thick lime occumulation. MOTTLED GREY & 25 ORANGE - BROWN SILTY EISTOCENE OCCASIONAL LIME ACCUMULATIONS 32 12.10.71 ASSOCIATED WITH JOINTS, SHEAR 2 cm. thick lime accumulation. 30 PLANES, AND SPT.18 Blows BEDDING PLANES Shear plane, slickenslided. Dip 20 Shear plane, slickensided Dip30 Joint plane, lime filled. Dip45 35 MOISTURE CONSISTENCY COMPACTNESS TYPE OF SAMPLE ENGINEERING GEOLOGY SECTION DENSITY (Sands) CONTENT Dec VS. - Very Soft Ls- Loose VL - Very Loose H - Humid LOGGED BY A shoe (SA) DRILL No. S - Soft D - Damp Water -MC - Moderately L - Loose R.F. JEUNE TYPE RUSTON " (SD) level, F - Firm Compact MD-Medium M - Moist DATE 18 10 71 DRILLER STURAK (SE) E , " (date) C — Compact W -- Wet Dense St - Stiff START . 8:10.71 TRACED DWW (SG) Sealed Tube --SAL V. St. - Very Stiff D - Dense S - Saturated FINISH . 13:10 71 WC CHECKED WAPB Compact - Liquid Limit H -- Hard Woter cut SHEET . 1. OF 2 DRG values refer to clay soils only and PL-Plastic Limit Standard Peneprovide an indication of their consistency

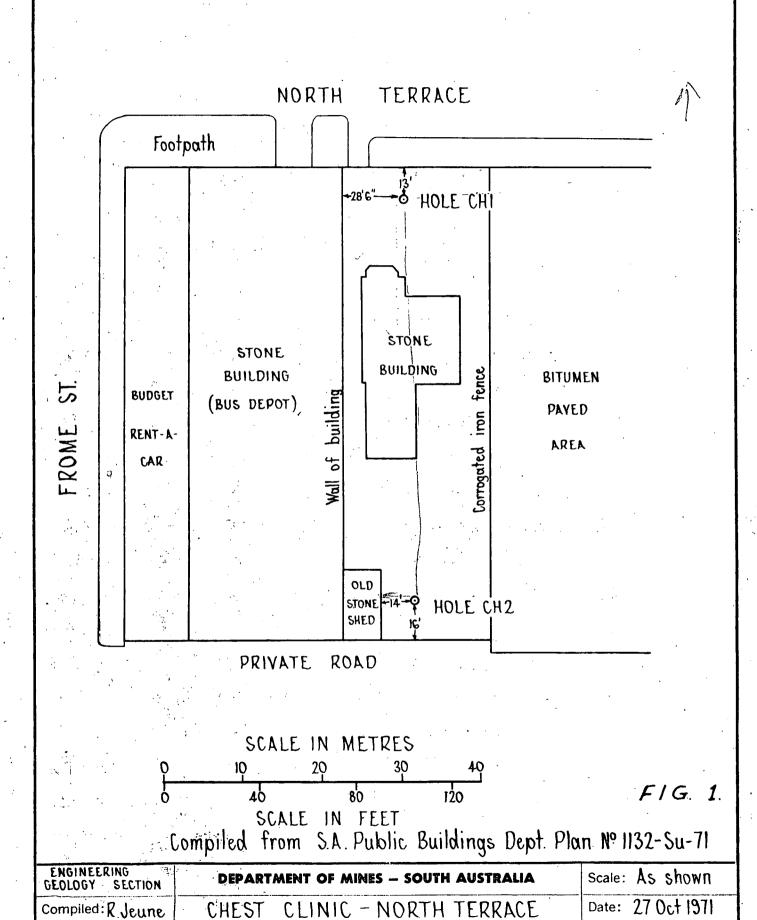
DEPARTMENT OF MINES - SOUTH AUSTRALIA CH BOLE LOG OF CABLE TOOL HOLE PROJECT CHEST CLINIC SERIAL No. NORTH TERRACE TOWN ACRE 28 R.L. Surface FEATURE FOUNDATIONS HUNDRED ADELAIDE R L Collar LOCATION GROUNDS OF HOUSE CO-ORDS Datum FIELD TEST DATA SOIL DESCRIPTION CONTENT GROUP GEOLOGICAL NOTES GRAPHI GROUP NAME DEPTH SOILTEST BLOWS AND CLASSIFICATION TR'METER Units # Unified Soil Classification, U.S.E.P. Earth Manual 2nd Edmon 1966 PER FOOT 20 40 60 80 35 U As for Sheet 1. SPT 18 Blows 70 SILT SOIL moderate plasticity.
Grey micaceous clayey silt with
orange brown mottles and rare was
strong from accretions up to 6mm W MC 40 CLAY SOIL, moderate plasticity. Grey silty clay with orange-brown mottles. Thin (Icm) basal Irangan 3 SPT 12 Blows SILT SOIL, moderate plasticity Micae-ous Thin (2-3mm) fine sandeclay brainations. WIMC INTERBEDDED CLAYS, 45 CH SILTS, AND DIRTY CLAY SOIL, moderate plasticity. 4 SPT 16 Blows SANDS, IN PART Nutty, grey, silty clay with orange-MICACEOUS. brown mottles. OCCASIONAL WEAK EISTOCENE IRONPANS 50-WCD SAND, excess silty fines. SM MD Bedded orange sgrey silty sands. SPT 19 Blaws SILT SOIL, low plasticity Laminated grey and orange-brown sits. ML D MC ZZ PL CLAY SOIL, moderate plasticity. Grey. Thin silt laminations. ST CH SILT SOIL, moderate plasticity. Grey 55 D MC clayey silt. TW SPT 9 Blows SAND, excess silty fines, Light greysilty fine sand, 4mm laminations SM W MD ML -CL SILT SOIL &CLAY SOIL, low plasticity. Le W MC PLIO-PLEISTOCENE Thin (Icm) gravel band. 60 CARISBROOKE DIRTY SAND MD SPT 10 Blows SM SANO excess silty fines RARE THIN GRAVEL Orange-brown silty fine sand. BANDS. 65 OF HOLE 67 ft. RELATIVE CONSISTENCY (Clays) MOISTURE COMPACTNESS TYPE OF SAMPLE ENGINEERING GEOLOGY DENSITY (Sands CONTENT Dec. VS. - Nery Soft VL - Very Loose Ls-Loose LOGGED BY A shoe (SA) DRILL No. S -- Soft Water -MC - Moderately L -- Loose D - Domo (SD) TYPE RUSTON R.F. JEUNE level, -F - Firm Compact MD-Medium M - Moist n (SE) DATE 18.10.71 DRILLER STURAK (date) C — Compact W - Wet - Stiff Dense START 8.10.71 TRACED D.W.W. G \*\*
Sealed Tube -SAL V. St. - Very Stiff VC - Very D - Dense 5 - Saturated FINISH . 13-10-71 CHECKED WAPB WCI VD - Very Dense LL - Liquid Limit Compact Water cut H. - Hard SHEET 2. OF 2 S9546a/1 \* These values refer to clay soils only and Standard Pene-tration Test-SPT. PL-Plastic Limit provide an indication of their consistency

DEPARTMENT OF MINES SOUTH AUSTRALIA HOLE CH 2 LOG OF CABLE TOOL HOLE PROJECT CHEST CLINIC SERIAL No. NORTH TERRACE TOWN ACRE 28 R.L. Surface, 41:5 m FT FEATURE FOUNDATIONS HUNDRED ADELAIDE R.L. Coller . GROUNDS OF HOUSE LOCATION. CO-ORDS Datum FIELD TEST DATA SOIL DESCRIPTION GEOLOGICAL NOTES GRAPHI LOG GROUP NAME SOIL TEST AND CLASSIFICATION **BLOWS** Unified Soil Classification, U.S.B.R PER FOOT TR'METE Earth Manual 2nd Edition 1966 20 40 60 80 ML SILT SOIL, low plasticity. Friable brown silt 30% sand and gravel to 1 cm. SILT AND SANDY \_,= DAA SILT SOIL, low plasticity. Friable SILT, ORGANIC ML red-brown to orange-brown clayey silt. Roots and burrows common in upper 12" Lime accretions, strong, up to 2 cms, abundant in lower 24" Ø\_**6**\_ S.P.T. 31 Blows GRAVEL, excess silty fines. calcareous gravel, strong, up to 7cms, in calcareous, silty sand matrix. Ls GM I L CALCAREOUS SILTY SAND AND GRAVEL Gradational change—1 SAND, excess silty fines. \*= -ZONE OF LIME 20% calcareous gravel up to 2 cms. Grey calcareous gravelly sand in silt matrix Gradational change ACCUMULATION. SM POSSIBLY WEAK CALCRETE z SC Ls ŢĪ, 15 Ift. Upper Zone slightly calcareous S.P.T. 6 Blows Shear plane, slickenslided, DIP 20° 20 SPT 10 Blos CLAY, SOIL, moderate plasticity. 3 Nutty gray silty clay with orange-brown and red MOTTLED GREY AND なり ORANGE -BROWN 4 brown mottles SILTY CLAY 25 OCCASIONAL LIME SPT.98lova ACCUMULATIONS ASSOCIATED WITH Shear plane, slickenslided, JOINTS, SHEAR PLANES, Dip 45 AND BEDDING PLANES 30 18-10-71 SPT 15 Blo - Ifi. Zone dark brown clay 35-RELATIVE MOISTURE CONSISTENCY (Clays) TYPE OF SAMPLE COMPACTNESS ENGINEERING GEOLOGY SECTION 1 DENSITY (Sands) CONTENT (Silts) VS. - Very Soft VL --- Very Loose Ls-- Loose A shoe (SA) H - Humid LOGGED BY DRILL No. 2 S --- Soft MC — Moderately L -- Loose - Damp Water , \*\* (SD) R.F.JEUNE DATE 20-10-71 TYPE RUSTON ود ,level F --- Firm Compact MD-Medium M - Moist # (SE) DRILLER STURAK (date) St. - Stiff C - Compact W --- Wet \* TRACED DJM START 13-10-71 G Seoled Tube --SAL V. St. -- Very Stiff V C - Verv D -- Dense S -- Saturated WC M 18-10-71 CHECKED WAPS FINISH . — Hard Compact VD -- Very Dense LL - Liquid Limit Water cut PL-Plastic Limit SHEET / OF 2 DRG Standard Pene-These values refer to clay soils only and S 9551 Ha6 provide an indication of their consistence

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DEPARTMENT OF MINES SOUTH HOLE CH 2 LOG OF CABLE TOOL HOLE PROJECT CHEST CLINIC SERIAL No. Town Acre 28 NORTH TERRACE R.L. Suiface. FEATURE : **FOUNDATIONS** HUNDRED. ADELAIDE R.L. Collar . GROUNDS OF HOUSE LOCATION. CO-ORDS Datum SOIL DESCRIPTION FIELD TEST DATA GEOLOGICAL NOTES GRAPHI LOG GROUP NAME SOILTEST **BLOWS** AND CLASSIFICATION Unified Soil Classification, U.S.B.R. PER FOOT Earth Manual 2nd Edition 1966 20 40 60 80 35 -24. Zone of lime accumulation ú for Sheet 1. 40 SPT 26 Blows Gradational Change SILT SOIL, moderate plasticity Grey micaceous clayey silt, MC orange-brown mottled CLAY SOIL, moderate plasticity INTERBEDDED CLAYS. Nutty, grey, silty clay SILTS, AND DIRTY SAT. 12 BIONS SAND, excess silty fines SANDS, IN PART MICACEOUS 2 Grey micaceous silty fine sand with thin (Icm) bands of grey, MD moderately plastic clay W 50 SM O 5 ft. Basal zone of grey micaceous clayey silt, moderately 17 Blows compact CLAY SOIL moderate plasticity H2-73 Nutty, grey, silty clay. 5 55 SILT SOIL, moderate plasticity MC 4₹ Grey micaceous clayey silt: II Blok SAND, excess silty fines. Grey DIRTY SAND, SM W MD and orange brown, micaceous RARE PEBBLES silty fine sand SAND, poorly graded. Micaceous quartz sand with rare pebbles S MD 60 115 END OF HOLE 60 ft MOISTURE TYPE OF SAMPLE CONSISTENCY COMPACTNESS ENGINEERING GEOLOGY SECTION DENSITY (Sands) CONTENT (Clays A shoe (SA) -Very VL - Very Loose Ls-Loose H --- Humid LOGGED BY DRILL No. S --- Soft MC — Moderately L --- Loose D - Damp (5D) Water TYPE RUSTON R. F. JEUNE F - Firm leveL Compact MD-Medium M --- Moist + (SE) DRILLER STURAK St. - Stiff – Compact W --- Wet ' Densa TRACED DJM (SG) START . 413-10-71. V. St. -- Very Stiff V C - Very D -- Dense 5 — Saturated FINISH . 18-10-71 CHECKED WRPS VD - Very Dense Hord LL - Liquid Limit values refer to clay soils only and Standard Pene-These SHEET 2 OF 2 DRG \$9551-a Hab PL-Plastic Limit



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Drn. R. H. Ckd. R. F. J.