

# DEPARTMENT OF MINES SOUTH AUSTRALIA



GEOLOGICAL SURVEY  
ENGINEERING DIVISION

PROPOSED BUILDING - CHEST CLINIC, NORTH <sup>TCE</sup> ~~ADLAIDE~~  
TOWN ACRE 28, HD. ADELAIDE

FOUNDATION INVESTIGATIONS

DESIGN STAGE

Client: Public Buildings Department

by

R.F. JEUNE  
GEOLOGIST  
ENGINEERING GEOLOGY SECTION

Rept. Bk. No. 71/182

10th November, 1971.

70-1371/182

DEPARTMENT OF MINES  
SOUTH AUSTRALIA

PROPOSED BUILDING - CHEST CLINIC, NORTH <sup>TCE</sup>~~ADELAIDE~~  
TOWN ACRE 23, HD. ADELAIDE

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CH 1 and CH 2 and explanatory  
notes.

FIGURES

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1	Chest Clinic - North Terrace Site Plan	S9543

Rept. Bk. No. 71/182  
G.S. No. 4751  
D.M. No. 961/70

10th November, 1971.

DEPARTMENT OF MINES  
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TOWN ACRE 28, HD. ADELAIDE

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DESIGN STAGE

Client: Public Buildings Department

SUMMARY AND CONCLUSIONS

Two cable tool drill holes, 60.5 and 66 ft. deep have been put down at the site of a proposed 2 storey building adjacent to North Terrace, Adelaide. These indicated a succession up to 55 ft. thick of clays, silts, sands and gravel (possible alluvial deposits of the old River Torrens) overlying at least 8 ft. of sand (Carisbrook Sand). A zone of lime accumulation of variable strength, and about 10 ft. thick, occurs immediately beneath the topsoil. The clays beneath this zone are of moderate plasticity at a moisture content approximately equal to plastic limit and are stiff. They appear to be subject to some shrinkage and swelling movements with changes in moisture content.

Groundwater was cut at several depths in each hole. The water cut at a depth of 8 to 9.5 ft. appears to be perched water, connected to the ground surface.

The clays beneath the zone of lime accumulation appear suitable for foundations, either for piers, or driven or cast-in-situ piles. No significant weaker soil horizons are likely to occur beneath this clay.

However, care should be taken during construction to prevent access of water into these clays as this could cause decrease in strength and also instability of any unsupported excavation walls.

## INTRODUCTION

Two cable-tool foundation investigation holes were drilled by Mines Department personnel to Public Buildings Department purchase order C145295. Locations are shown in Figure 1. These holes were requested in order to determine foundation conditions beneath a proposed 2 storey building to be constructed at the site as a Chest Clinic.

Hole CH1 drilled between 8th and 13th October, 1971 reached a depth of 66 ft. Hole CH 2 drilled between 13th and 18th October, 1971 reached 60.5 ft. Sealed tube samples were recovered and standard penetration tests performed in both holes at nominal 5 ft. intervals. By arrangement with the client, the Geomechanics Division of the C.S.I.R.O. retained selected samples for investigation into soil suction phenomena. Geological logs of each hole are included in the Appendix.

## REGIONAL GEOLOGY

The Adelaide geological sheet (1:63,360 series) shows the site to be located on the Para Fault Block, with 10 to 20 ft. of recent alluvial clays and sands overlying approximately 400 ft. of Pleistocene mottled clays, sands and gravels. A small area of Tertiary sandy limestone is mapped immediately north of the site. Recent flood plain deposits are shown within the valley of the Torrens River.

## SITE GEOLOGY

### Topography

The site is flat, and covered by buildings, gardens, lawn, and gravel paths. Several large trees are growing near the rear boundary fence.

### Soil and Rock Types

All geological observations were limited to examination of cores from the 2 bores.

The geological succession as indicated by the drill cores is summarized in Table 1.

TABLE 1  
Geological Succession

Depth (ft.) from to		Thickness (ft.)	Age	Description
0	4 to 5	4 to 5	Recent	Fill and topsoil
4 to 5	Approx.15	Approx.10	Pleistocene to Recent	Zone of lime accumulation
Approx.15	37.5 to 42	24 to 27	Pleistocene to Recent	Silty clay, mottled
37.5 to 42	57 to 58.5	15 to 20	Pleistocene to Recent	Interbedded clays, silt and sands
57 to 58.5	Greater than 60 to 67	Greater than 8	?Plio-Plioste- cene	Sand (?Carisbrook Sand)

The soils between depths of 5 and 58.5 ft. are thought to be alluvial deposits related to the old Torrens River, but may be slightly atypical soils of the Hindmarsh Clay Formation.

### GROUNDWATER

Details of ground water observations made during drilling are shown in Table 2.

TABLE 2

Details of Ground Water Observations

Hole	Water Cut ft.	Static Level ft.	Estimated Flow galls/hr	pH	Salinity p.p.m.
CH 1	8	7.5	50	8.0	1385
"	39	28	100	8.0	1900
"	50	28	100	7.5	1900
"	58	28	100	8.0	2155
CH 2	9.5	9.5	50	8.0	1900
"	43	29	100	8.0	2755
"	55	29	100	8.0	3100

The perched water table between 8 ft. and 9.5 ft. appears to be in directed communication with the surface and could be expected to show a seasonal rise and fall. Suitable allowances should be made in designs incorporating basements, and in the planning of excavations. All deeper waters encountered rose considerably when tapped, and all stabilized at substantially the same level. However, salinities and pH values suggest that they may not all be directly inter-connected.

# DISCUSSION

## Recent fill and topsoil

These are rather variable gravelly and sandy silts and clays, in part organic. This unit has probably been substantially modified by man, and its disturbed and variable nature renders it unsuitable for foundations.

## Zone of lime accumulation

This is a zone of calcareous silt, silty sand, and gravel. The presence in hole CH 2 of large (7cm) angular, tabular fragments of strong calcrete,

coupled with relatively high resistance to penetration, suggests that lenses of strong (Table 3) calcrete may be present, but are likely to be irregular in distribution. The lower layers are saturated, but the texture of the whole unit is such as to suggest that some collapse may occur under other than very slight loads.

The irregular distribution of lime and of calcrete and the possibility of collapse under load suggests this unit is unsuitable foundations.

#### Mottled silty clay

This is a stiff grey and orange brown mottled silty clay with occasional thin lime accumulations along joint planes, and associated with bedding planes. The upper 1 to 2ft. is slightly calcareous. Occasional slickensided shear planes, and a weakly developed fine nutty structure suggest that some expansion and shrinkage may be taking place with changes in moisture content.

Since correlation with the Hindmarsh Clay Formation, which occurs beneath much of the Adelaide City Area, and the engineering properties of which are well-known, is uncertain, it is suggested that the relevant physical properties of this material should be checked by laboratory tests on the sealed tube samples.

#### Interbedded clays, silts, and sands

These are dominantly grey clays, silts, and sands, in part micaceous. Comparisons between holes CH1 and CH2 show only a general similarity in sequences, suggesting that individual beds may be rather variable and discontinuous.

**TABLE 3**  
**CLASSIFICATION OF ROCK CONDITIONS AND STRENGTH OF ROCK SUBSTANCE**

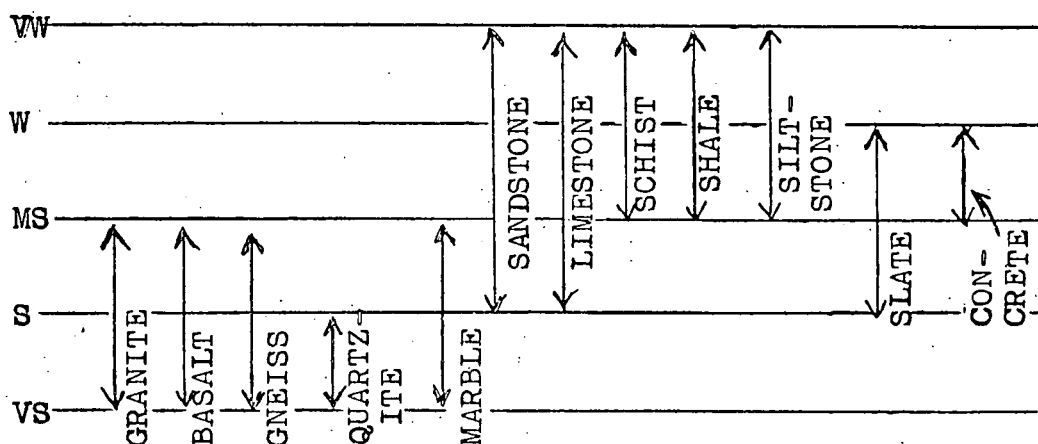
**1. ROCK CONDITION TERMS**

TERM	ABBRN	DEFINITION
Fresh	(F)	Substance shows no effects of chemical decomposition.
Chemically Decomposed	(D)	Substance is affected by chemical decomposition, but the exact process is not obvious.
Chemically Weathered	(W)	Substance shows effects of chemical decomposition processes which have occurred due to surface and near-surface agencies such as air and groundwater.
Chemically Altered	(A)	Substance shows effects of chemical decomposition processes which have occurred due to plutonic or volcanic fluids.
Extremely { Decomposed Weathered Altered	{ (XD) (XW) (XA)	Substance has been reduced to material which shows fabric of original rock, but which can be remoulded, i.e. soil substance. (Classified by Unified System).

**2. CLASSIFICATION OF ROCK SUBSTANCES BY UNCONFINED COMPRESSIVE STRENGTH**

TERM	ABBRN	UNCONFINED COMPRESSIVE STRENGTH	
		(Kg/sq.cm)	(lb/sq.in)
Very weak	VW	< 70	≤ 1000
Weak	W	70 - 200	1000 - 3000
Medium strong	MS	200 - 700	3000 - 10,000
Strong	S	700 - 1800	10,000 - 25,000
Very strong	VS	> 1800	> 25,000

**RANGE OF STRENGTHS OF SOME COMMON  
ROCK SUBSTANCES IN THE FRESH STATE\***



\*Samples of fresh rock tested to Australian Standard. For rocks showing planar anisotropy the long axis of the sample is normal to fabric planes.

**3. EXAMPLES OF USE OF CLASSIFICATION**

<u>Geological Name</u>	<u>Rock Condition Term</u>	<u>Strength Term</u>
Granite	Fresh	Strong
Granite	Weathered	Medium Strong
Schist	Fresh	Weak
Schist	Altered	Very Weak

Sands - (?Carisbrook Sand)

This is a dirty sand with rare thin gravel bands, intercepted at the bottom of both holes. This unit is likely to extend for several more feet and no weak horizons are likely to occur.

RFJ:CMH  
10.11.1971

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**APPENDIX**

**Logs of Cable Tool Drill Holes CH 1 & CH 2,  
and explanatory notes.**

## APPENDIX

### LOGS OF CABLE TOOL HOLES AND EXPLANATORY NOTES

#### NOTES ON DRILLING PROCEDURES

##### Equipment

The drilling is carried out with a cable tool drilling plant using sampling tubes attached, through a vacuum head, to the sampling tools (Figs. 1 and 2).

##### Sampling Procedures

##### S.A. Samples

To obtain, for logging purposes, an almost continuous series of samples, with a relatively small amount of sample disturbance, SA type samples are taken. These are obtained by driving an "S" tube, fitted with a Mark A shoe (Fig. 2), into the material to be sampled.

The assembly is lowered carefully to the bottom of the hole; and the tube driven exactly 1 foot, and the number of blows required for the 1 foot of penetration recorded.

The sample, or core, is extruded from the sampling tube using an hydraulic ram. The extruded core is sealed in a labelled plastic bag and stored in a core box (Fig. 3).

The hole is reamed with a "D" or "E" shoe (Fig. 2) and then the next sample is taken, using the same procedure as above. Thus the hole proceeds by alternate sampling, reaming (and where required, casing) operations, and the samples form a continuous record of the materials penetrated except for a few inches which may be lost between samples during reaming operations.

SA sampling equipment is a composite sampler for simple class sampling. Details are as follows:-

##### "S" SERIES CUTTING SHOES

MARK	FEATURES	USES
A	Inside clearance 3%. Area Ratio 33%.	Continuous open-tube sampling in strong soils, in which it causes little deformation. Samples extruded and used for logging purposes. Hole is reamed after each sample.
D	Shoe belled out to 4.29/32 in. (just greater than outside dia. of vacuum head)	Continuous open tube sampling where considerable deformation of sample is permissible. Essentially self-reaming.

MARK	FEATURES	USES
E	Shoe belled out to 5 7/16 in. (just less than internal dia. of 6 in casing)	Cleaning hole and reaming out hole.

### Sealed Tube (LB) Samples

Sealed tube samples, for laboratory testing, are taken at various intervals during drilling. These are obtained by driving an "L" type sampling tube with a Mark B cutting shoe (Fig. 4) a distance of 1 foot into the material to be sampled.

Before the sample is taken the hole is cleaned out to the depth specified. The hole is not reamed or cased for at least 1 foot from the bottom, however, because these operations can cause considerable disturbance in the soil below. The sampling assembly is lowered carefully to the bottom of the hole, the sampling tube driven exactly 1 foot, and the number of blows recorded.

The sample is sealed in the tube by inserting in each end, plastic seals with rubber sealing rings, and the tube is then labelled and stored in a Laboratory Sample Box.

LB sampling equipment is a composite sampler for obtaining samples with the least possible disturbance. Details are as follows:

Sampler tube - ASSAB tube cadmium plated  
 "L" type                      4.016 in I.D.  
                                     4.282 in O.D.

Mark B shoe - ASSAB tube, heat treated, cadmium plated  
                                     Area ratio 15%  
                                     Inside clearance - ~~nil~~ 0.4%  
                                     Outside clearance - nil  
                                     Cutting edge angle - 7°

### Standard Penetration Test

The Standard Penetration Test (Terzaghi et al 1948) is used to test the in-situ density of sands and to give an indication of the consistency of clays, and compactness of silts. However the test results can be affected by several geological factors such as degree of cementation, and size and shape of grains. These factors should be taken into account in interpretation of results.

The equipment is illustrated in Fig. 5 and consists of a 2in. diameter, sampling spoon (tube) and a hammer of standard weight (140 lbs).

With the equipment assembled as in Fig. 5 the hammer is allowed to fall on to the drill rods until the sampling shoe has penetrated 6 in. into the soil. The Standard Penetration Test is the number of blows (N) required to produce the next foot of penetration.

#### NOTES ON DRILL LOG SHEETS

The logs are plotted on a standard cable-tool log form.

Near the centre of the form a graphic log of the materials encountered is shown.

In the column to the right of the graphic log, the soils are classified and described according to the Unified Soil Classification (U.S. Dept. of Interior, Bureau of Reclamation 1966) as shown on Figure 6.

To the left of the graphic log is a geological description of the materials sampled. This includes:-

.....Geological age	}	Printed vertically
.....Soil unit name		
.....Type of material		
.....Mineral composition		
.....Grain shape		
.....Cementation		
.....Organic materials		

Water levels are indicated by a small arrow with the date at which the observation was made.

In the blows per foot column, a continuous histogram is made of the number of blows required to drive the sampling tube through each foot of material. A hatching code is used to distinguish various types of sample. This code is reproduced at the bottom of each log sheet.

In the column on the far right of the log sheet, readings of unconfined compressive strength ( $q_u$ ) made with a Soiltest Penetrometer, are recorded. The readings are plotted as a histogram. The Soiltest Penetrometer only gives true values of  $q_u$  when used in clays in which  $\phi = 0$ .

REFERENCES

TERZAGHI, K. and PECK, R.B., 1948. Soil Mechanics in Engineering Practice. John Wiley and Sons.

UNITED STATES DEPARTMENT OF THE INTERIOR, BUREAU OF RECLAMATION,  
1966. Earth Manual, 2nd Edition.

## DESCRIPTIVE TERMS

### 1. CLAY SOILS

#### CONSISTENCY

CONSISTENCY	SYMBOL	UNCONFINED COMPRESSIVE STRENGTH (kg/sq. cm)	FIELD TEST	N
Very Soft	V.S.	less than 0.25	Easily penetrated several inches by fist.	2
Soft	S	0.25 to 0.5	Easily penetrated several inches by thumb.	2 to 4
Firm	F	0.5 to 1.0	Can be penetrated several inches by thumb with moderate effort.	4 to 8
Stiff	St	1.0 to 2.0	Readily indented by the thumb but penetrated only with great effort	8 to 15
Very Stiff	V.St.	2.0 to 4.0	Readily indented by thumb nail.	15 to 30
Hard (Extremely stiff)	H	over 4.0	Indented with difficulty by thumb nail.	30 and over

Based partly on Terzaghi, K. and Peck, R.B. 1966. Soil Mechanics in Engineering Practice, Wiley - New York.

#### MOISTURE CONTENT

Abbreviation	Meaning
MC ≈ LL	Moisture Content near liquid limit.
MC < LL	less than liquid limit.
MC > PL	greater than plastic limit.
MC ≈ PL	near
MC ≤ PL	less or equal to plastic limit.
MC < PL	less than
MC << PL	much less than

## 2. SILT SOILS

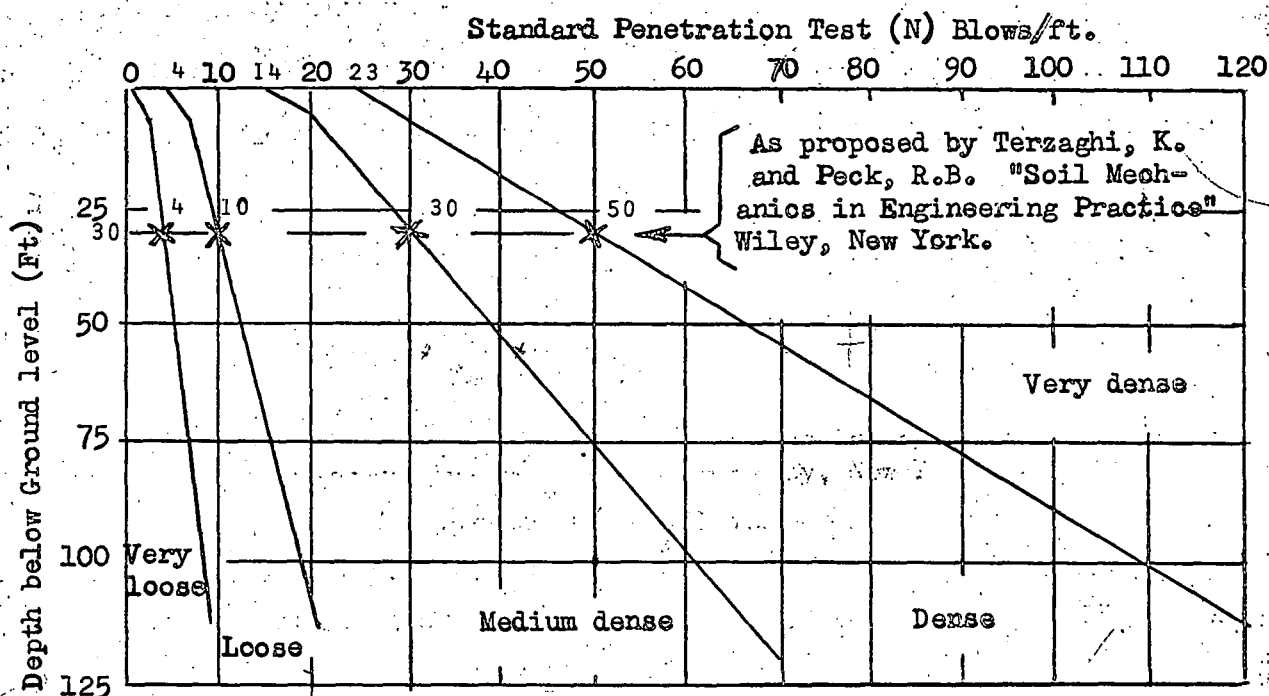
COMPACTNESS	SYMBOL	N
Loose	LS	0 to 8
Moderately compact	MC	8 to 15
Compact	C	15 to 30
Very Compact	VC	greater than 30

## 3. SANDS

### CLASSIFICATION OF SANDS BY STANDARD PENETRATION TEST

The relative density of granular soils has been judged from the results of Standard Penetration Tests carried out by the procedure described by Terzaghi and Peck (1948) bearing in mind the limitations of the method as discussed by Gibbs and Holtz (1957). At all times the water in the drill hole was kept at the level of surrounding groundwater.

### EFFECT OF OVERBURDEN PRESSURE ON STANDARD PENETRATION TEST

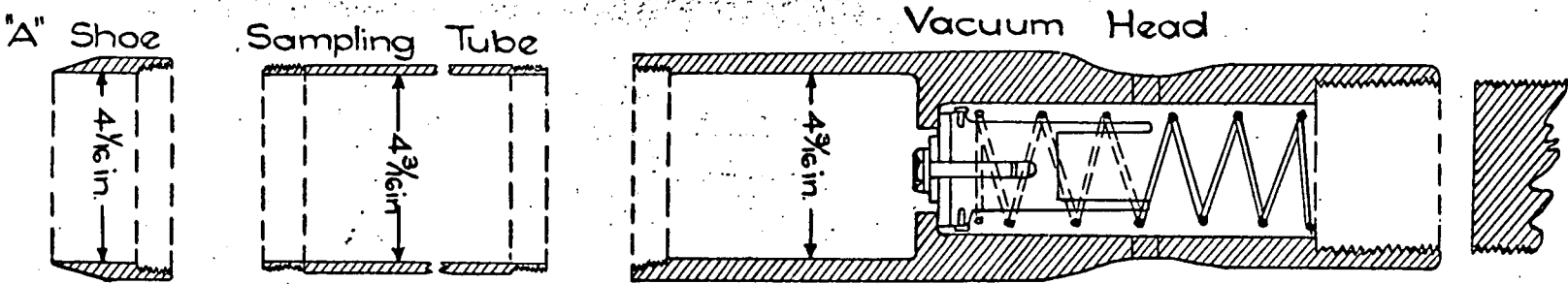
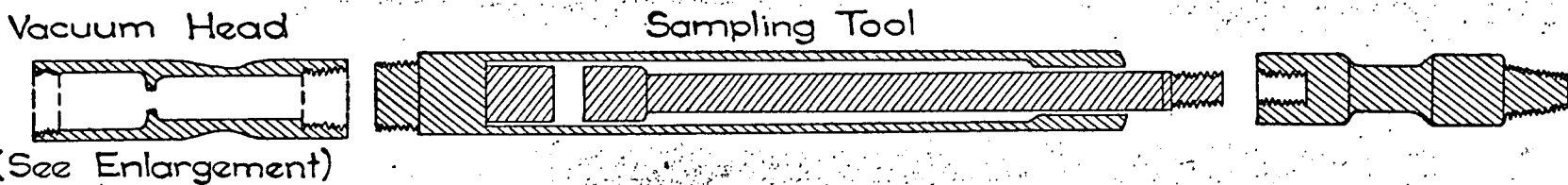
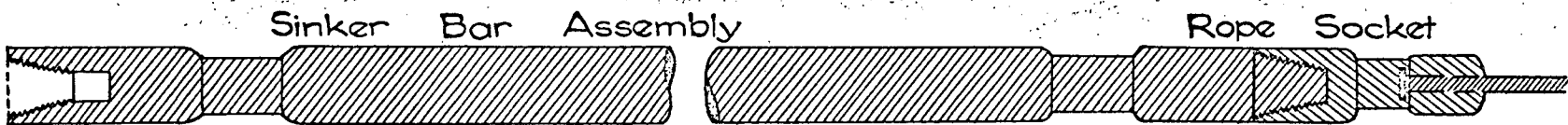


Based on Gibbs, H.J. & Holtz, W.G. (1957) "Research on Determining the Density of Sands by Spoon Penetration Testing" Vol. I Proc. 4th Int. Conf. SM & FE, London.

### REFERENCES

TERZAGHI, K., and PECK, 1948. "Soil Mechanics in Engineering Practice". Wiley. New York.

GIBBS, H.T. and HOLTZ, W.G., 1957. Research on Determining the Density of Sands by Spoon Penetration Testing. Proc. 4th Inter. Conf. SM & FE, London, Vol. 9.



APPENDIX FIG.1

DEPARTMENT OF MINES - SOUTH AUSTRALIA

ENGINEERING  
GEOLOGY  
SECTION

*W. Blayden*  
SENIOR GEOLOGIST

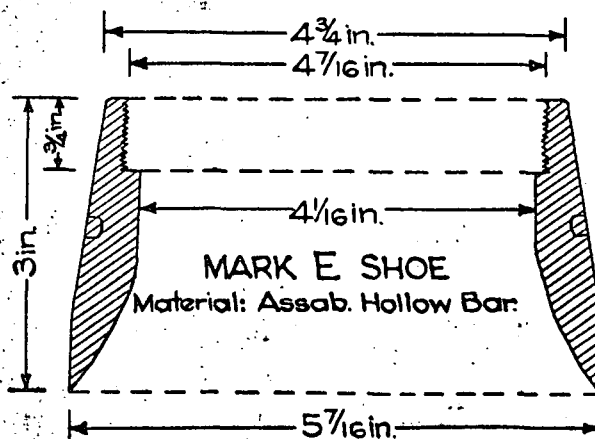
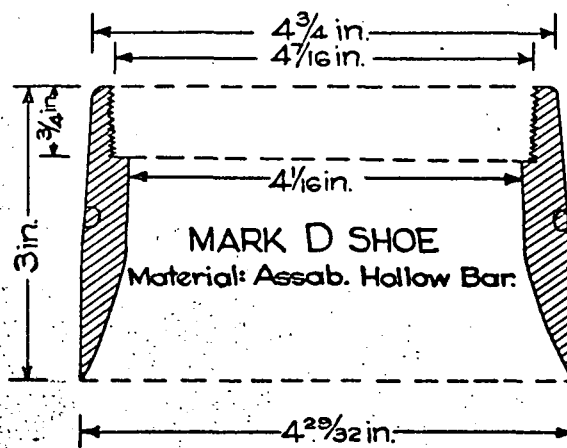
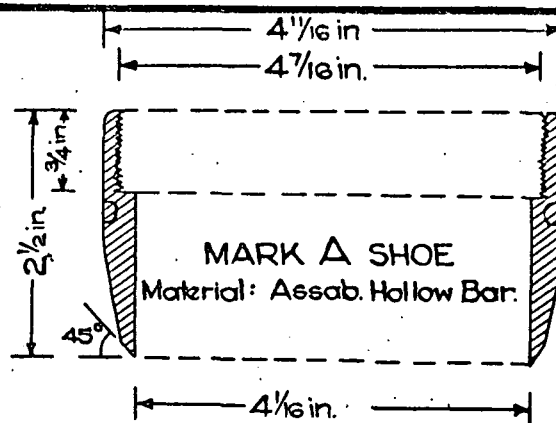
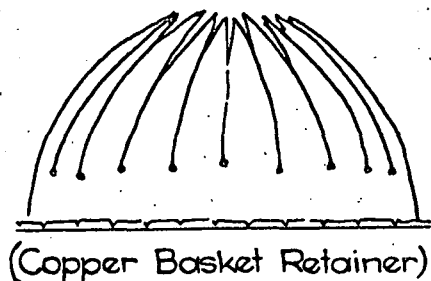
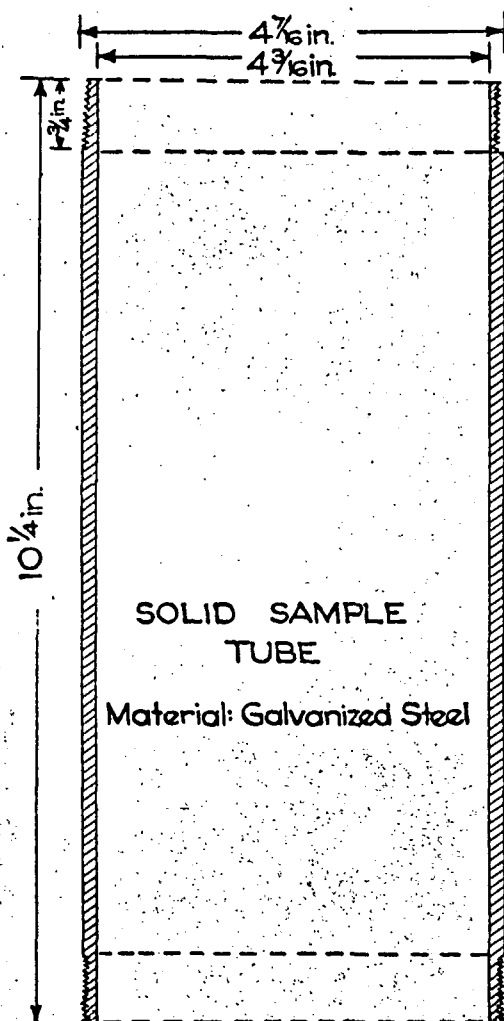
Drn. DHS  
Tcd. AMSD  
Ckd. L.V.M.  
Ext. DA

CABLE-TOOL DRILLING  
SAMPLING TOOL ASSEMBLY  
WITH "S" SERIES SHOES

SCALE: NOT TO SCALE

S 4416  
MB

DATE: 21 Oct '69



**APPENDIX FIG. 2**

**DEPARTMENT OF MINES — SOUTH AUSTRALIA**

ENGINEERING  
GEOLOGY  
SECTION

Drn D.H.G.

Tcd. A.M.S.D.

Ckd. L.V.W.

Exd. D.H.

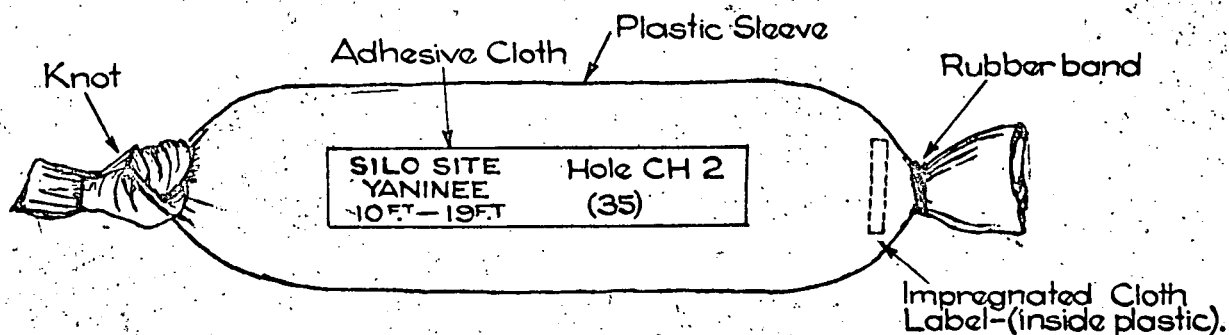
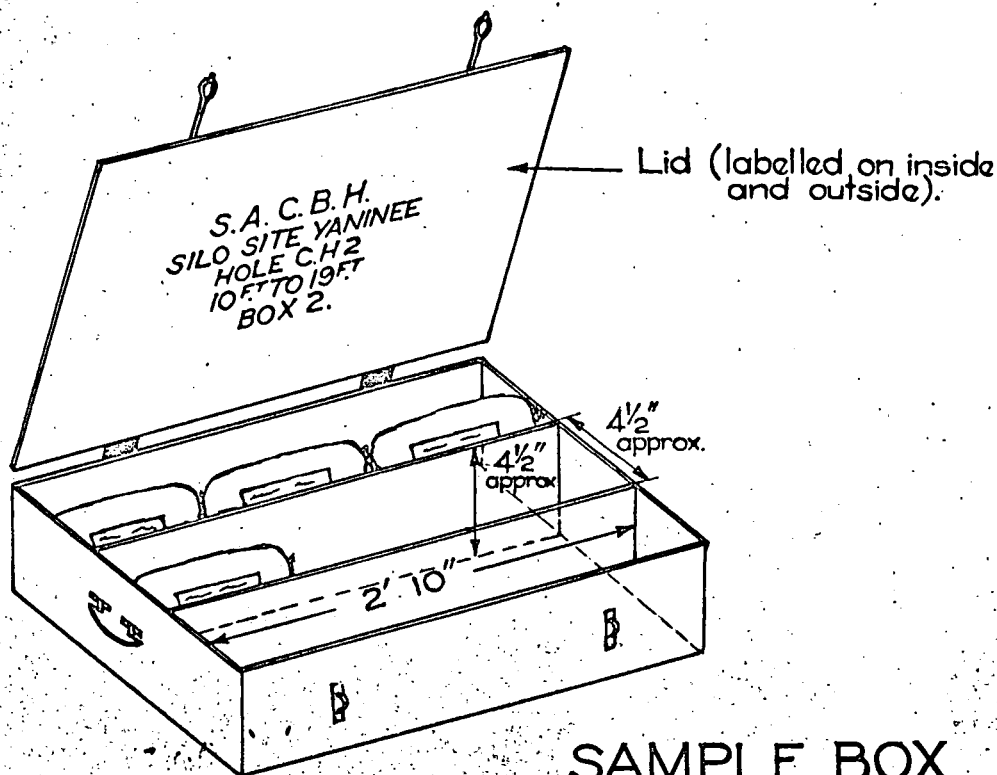
**CABLE-TOOL DRILLING  
S SERIES SAMPLING  
TUBE AND SHOES**

SCALE: NOT TO SCALE

S4418  
MB

DATE: 21 Oct '69

*J.H. Spiller*  
SENIOR GEOLOGIST



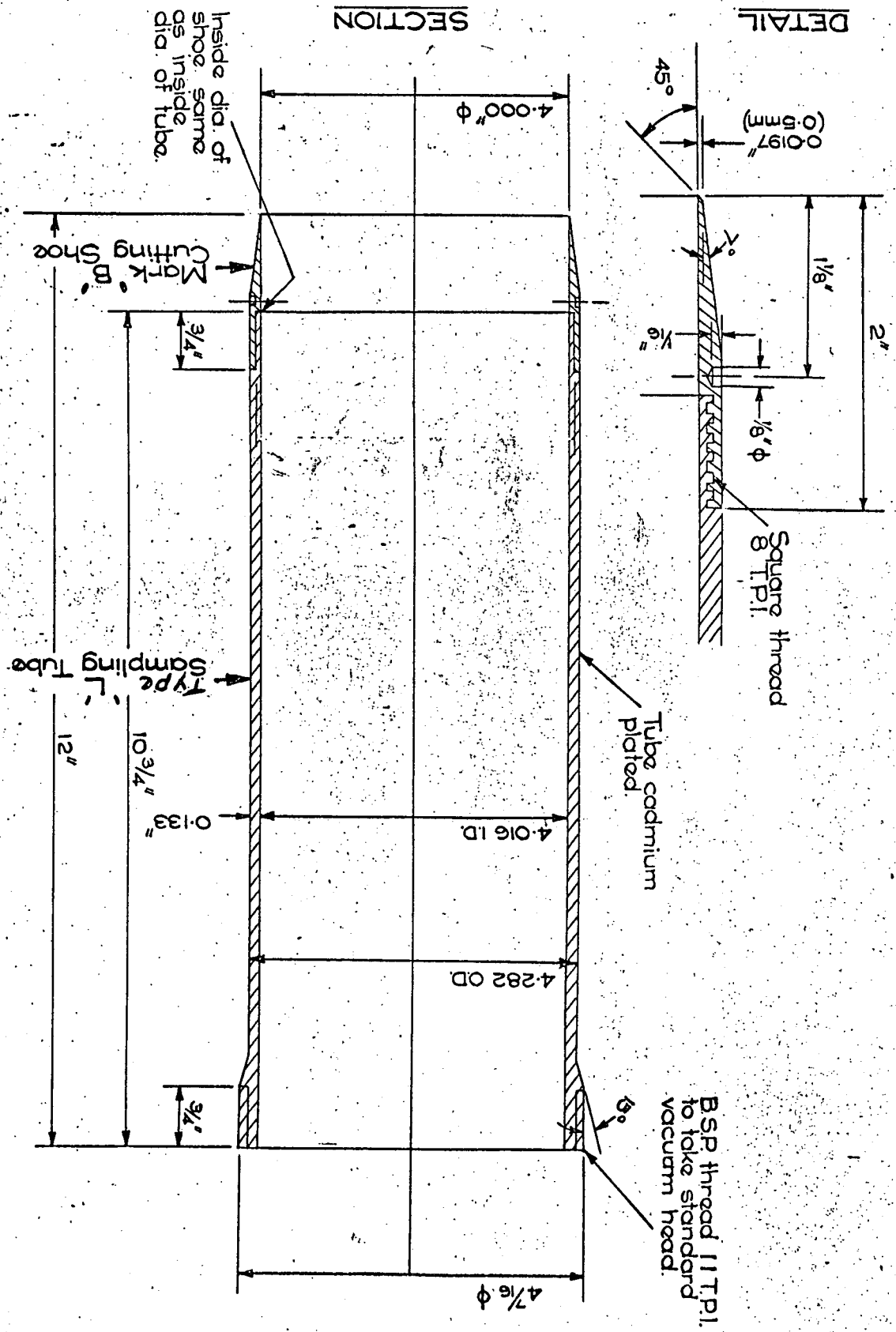
EXTRUDED SAMPLE  
SEALED IN PLASTIC SLEEVE.

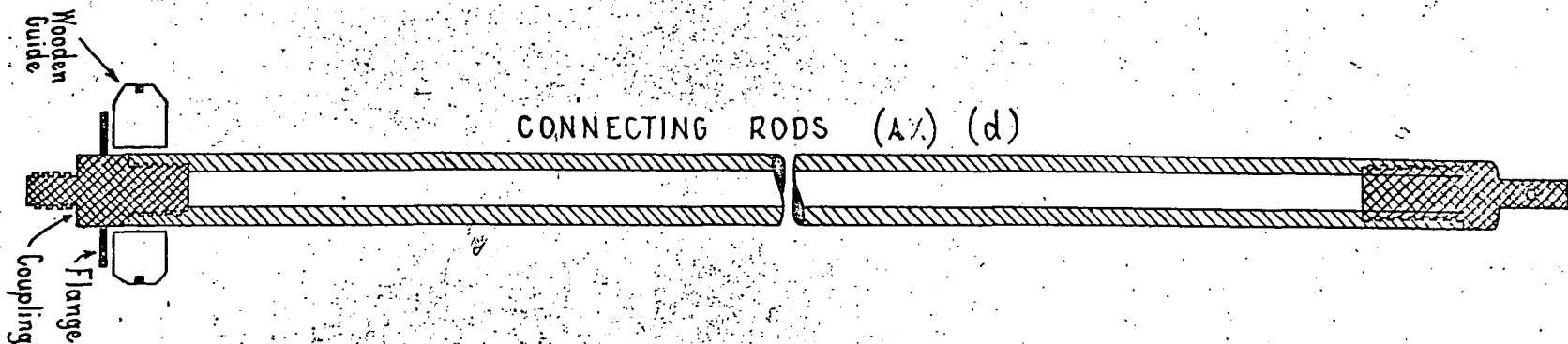
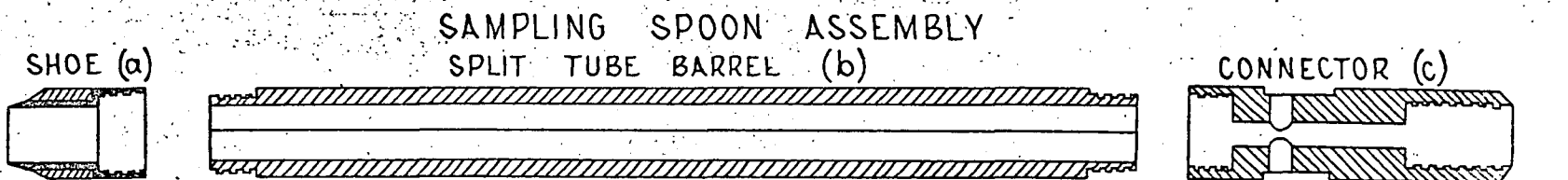
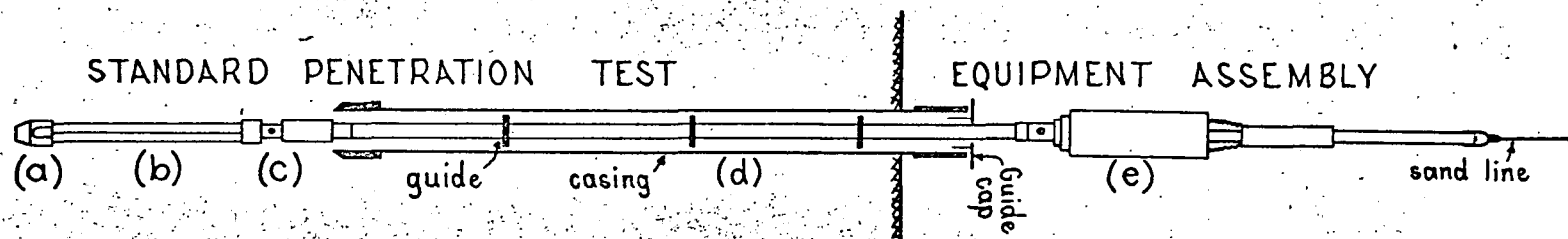
APPENDIX FIG. 3

ENGINEERING DIVISION	DEPARTMENT OF MINES - SOUTH AUSTRALIA	Scale:
Compiled: W.R.P.B		Date: 17 Dec '69
Drn AMSD. Ckd.	CABLE TOOL DRILLING LABELLING AND BOXING OF EXTRUDED SAMPLES	Drg. No S7580 MD

# CABLE TOOL DRILLING THIN-WALLED "L" SAMPLE TUBE AND "B" SHOE

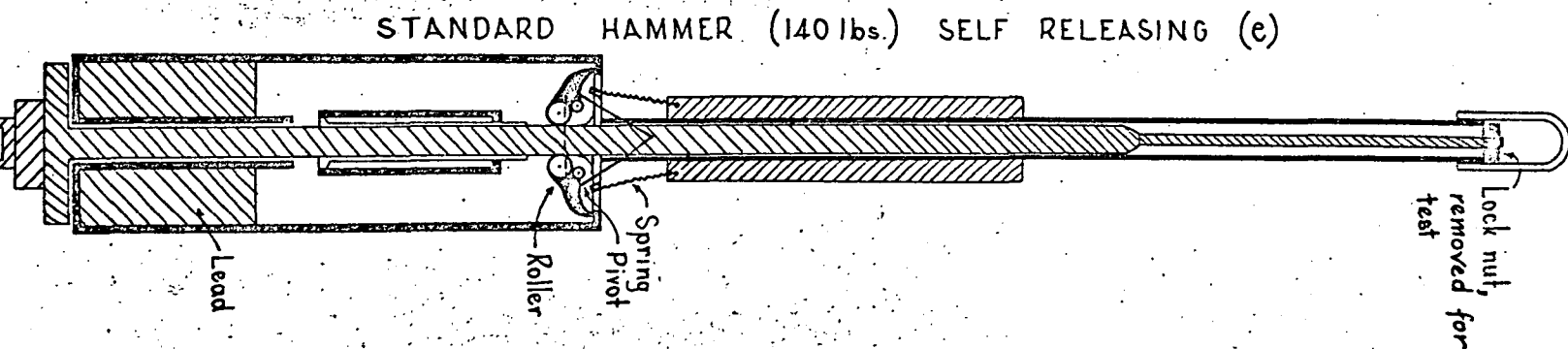
APPENDIX FIG. 4





# APPENDIX

FIG.5



ENGINEERING  
DIVISION

Compiled: W.R.P.B.

Drn. R.H. Old.

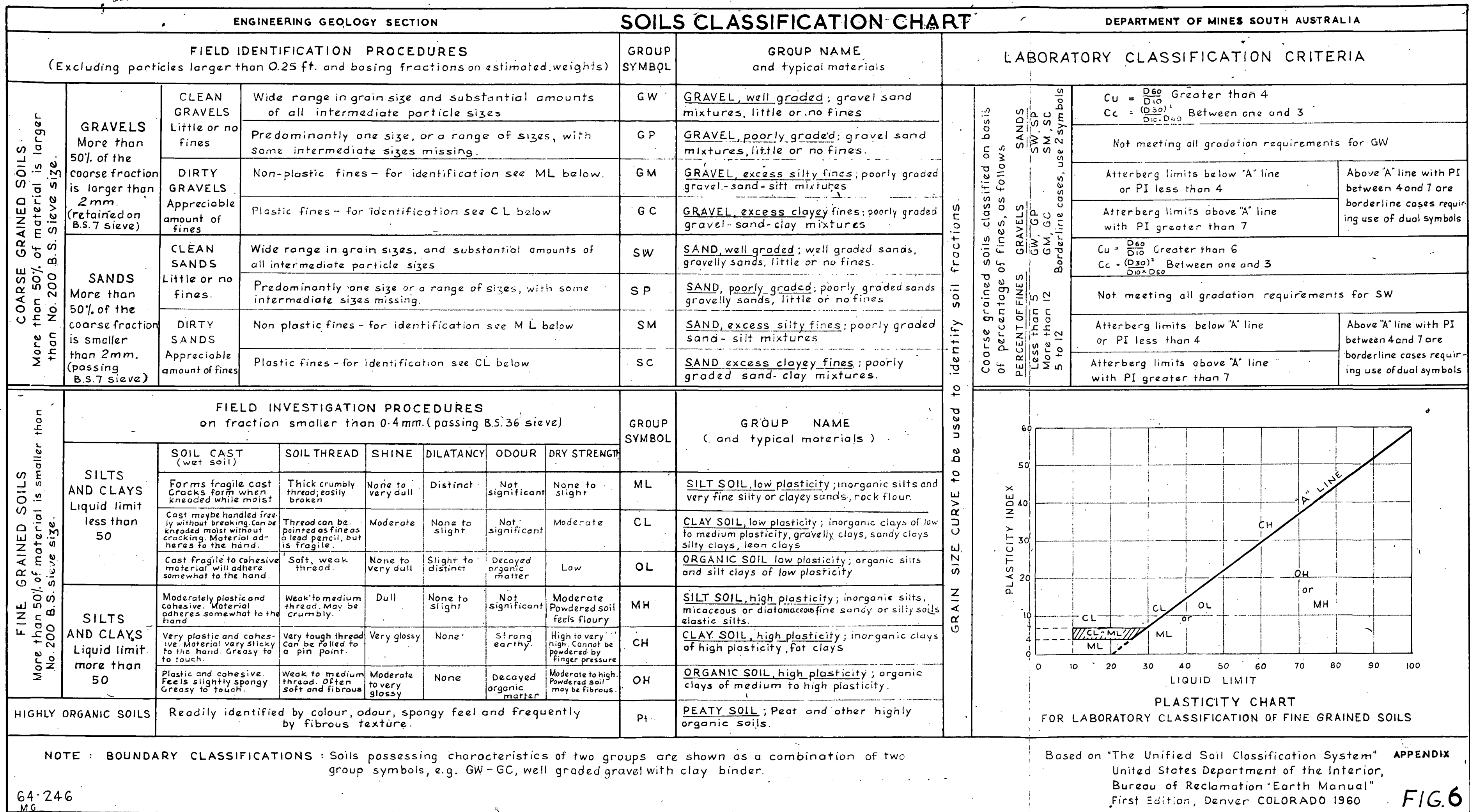
DEPARTMENT OF MINES - SOUTH AUSTRALIA

STANDARD PENETRATION TEST  
EQUIPMENT

Scale: Not to scale

Date: 22 Dec 1969

Dwg. No  
S4420 MB



PROJECT **CHEST CLINIC**  
**NORTH TERRACE**

FEATURE **FOUNDATIONS**  
LOCATION **GROUNDS OF HOUSE**

DEPARTMENT OF MINES · SOUTH AUSTRALIA

# LOG OF CABLE TOOL HOLE

TOWN **ACRE 28**  
HUNDRED **ADELAIDE**  
CO-ORDS

HOLE **CH /**

SERIAL No.

R.L. Surface **40.5m** FT.

R.L. Collar FT.

Datum

GEOLOGICAL NOTES AND CLASSIFICATION			R.L. (FEET)	DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	CEMENTATION	MOISTURE CONTENT	Consistency	Compaction	FIELD TEST DATA	
													BLOWS PER FOOT 20 40 60 80	SOILTEST P.T.R. METER Units * 1 2 3 4
RECENT	FILL AND TOPSOIL	SILT AND CLAY, ORGANIC				ML	SILT SOIL, low plasticity. 20-30% Sand. Friable, black, moderate organic content.							
					CH	CLAY SOIL, high plasticity. Orange-brown, with scattered lime nodules up to 1cm.								
PLEISTOCENE - RECENT		CALCAREOUS SILT AND SILTY SAND - ZONE of LIME ACCUMULATION				ML	SILT SOIL, low plasticity. Brown to red-brown, organic at top, lime nodules at base.							
			5			ML	SILT SOIL, low plasticity. Brown-grey calcareous silt with strong lime nodules up to 2cm. at base.							
						SM	SAND, excess silty fines. Grey calcareous silty sand with scattered strong lime accretions up to 1cm. slightly sticky when remoulded.							
			10				Gradational Change							
		15				2ft. Zone. Slightly calcareous.								
						Vertical joint with roots.								
		20				CLAY SOIL, moderate plasticity. Nutty, grey, silty clay with orange-brown mottles.								
		25				1cm. thick lime accumulation.								
						27 1/2 EL 40.5 8.1m 32 12-10-71								
		30				2cm. thick lime accumulation.								
		MOTTLED GREY & ORANGE-BROWN SILTY CLAY. OCCASIONAL LIME ACCUMULATIONS ASSOCIATED WITH JOINTS, SHEAR PLANES, AND BEDDING PLANES					Shear plane, slickensided. Dip 20°							
							Shear plane, slickensided. Dip 30°							
							Joint plane, lime filled. Dip 45°							
			35											

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS. — Very Soft	LS — Loose	VL — Very Loose	H — Humid	DRILL No. <b>2</b>	LOGGED BY <b>R.F. JEUNE</b>
D " (SD)	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	TYPE <b>RUSTON</b>	DATE <b>18-10-71</b>
E " (SE)	F — Firm	C — Compact	MD — Medium Dense	M — Moist	DRILLER <b>STURAK</b>	TRACED <b>R.W.W.</b>
G " (SG)	St. — Stiff	VC — Very Compact	D — Dense	W — Wet	START <b>8-10-71</b>	CHECKED <b>W.R.P.B.</b>
Sealed Tube - SAL	V. St. — Very Stiff	VD — Very Dense	PL — Plastic Limit	S — Saturated	FINISH <b>13-10-71</b>	
Standard Penetration Test - SPT	H. — Hard			LL — Liquid Limit	SHEET <b>1 OF 2</b>	DRG No. <b>S9546/1</b>
	* These values refer to clay soils only and provide an indication of their consistency.					<b>Ha6</b>

## LOG OF CABLE TOOL HOLE

PROJECT **CHEST CLINIC  
NORTH TERRACE**FEATURE **FOUNDATIONS**LOCATION **GROUPS OF HOUSE**TOWN ACRES **28**HUNDRED **ADSLAIDE**

CO-ORDS

HOLE CH /

SERIAL No

R.L. Surface. FT.

R.L. Collar. FT.

Datum.

GEOLOGICAL NOTES AND CLASSIFICATION		R.L. (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME United Soil Classification, U.S.S.R. Earth Manual 2nd Edition 1966	WATER LEVEL CC CL LEVEL	MOISTURE CONTENT MOD. LINE CONSISTENCY Sample 1 (10cm)	FIELD TEST DATA		
								BLOWS PER FOOT 20 40 60 80	SOILTEST P.T.R. METER Units * 1 2 3 4	
PLEISTOCENE - RECENT	ALLUVIUM	35		CL-CH	As for sheet 1.					
		37.5		ML-MH	SILT SOIL, moderate plasticity. Grey micaceous clayey silt with orange-brown mottles and rare weak strong iron accretions up to 6mm.	W	MC		SPT 18 Blows	
		40		CL-CH	CLAY SOIL, moderate plasticity. Grey silty clay with orange-brown mottles. Thin (1cm) basal ironpan.	PL	St		SPT 12 Blows	
				ML-MH	SILT SOIL, moderate plasticity. Micaceous. Thin (2-3mm) fine sand clay laminations.	W	MC			
		45		CL-CH	CLAY SOIL, moderate plasticity. Nutty, grey, silty clay with orange-brown mottles.	PL	St		SPT 16 Blows	
		50								
				SM	SAND, excess silty fines. Bedded orange & grey silty sands.	S	MD		SPT 19 Blows	
				ML	SILT SOIL, low plasticity. Laminated grey and orange-brown silts.	D	MC			
				CL-CH	CLAY SOIL, moderate plasticity. Grey. Thin silt laminations.	PL	St			
				ML-MH	SILT SOIL, moderate plasticity. Grey clayey silt.	D	MC		SPT 9 Blows	
? PLIO-PLEISTOCENE	? CARISBROOKE SAND	58.5		SM	SAND, excess silty fines. Light grey silty fine sand, 4mm laminations.	W	MD			
		60		ML-CL	SILT SOIL & CLAY SOIL, low plasticity. Grey. 1cm laminations. Weak basal ironpan.	W	MC			
				SM	Thin (1cm) gravel band.				SPT 10 Blows	
		65			SAND excess silty fines Orange-brown silty fine sand.					
					— END OF HOLE 67 ft.					
					3 20 2					

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS. — Very Soft	LS — Loose	VL — Very Loose	H — Humid	DRILL No. 2	LOGGED BY
D " (SD)	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	TYPE RUSTON	R.F. JEUNE
E " (SE)	F — Firm	C — Compact	MD — Medium Dense	M — Moist	DRILLER STURAK	DATE 18.10.71
G " (SG)	St — Stiff	VC — Very Compact	D — Dense	W — Wet	START 8.10.71	TRACED D.W.W.
Sealed Tube — SAL	V St. — Very Stiff	VD — Very Dense	LL — Liquid Limit	S — Saturated	FINISH 13.10.71	CHECKED WRPB
Standard Penetration Test-SPT	H. — Hard		PL — Plastic Limit	LL — Liquid Limit	SHEET 2 OF 2	DRG No. S9546a/1
	* These values refer to clay soils only and provide an indication of their consistency.					Ha6



## LOG OF CABLE TOOL HOLE

PROJECT **CHEST CLINIC****NORTH TERRACE**FEATURE **FOUNDATIONS**LOCATION **GROUPS OF HOUSE****Town Acre 28****HUNDRED. ADELAIDE**

CO-ORDS

HOLE **CH 2**

SERIAL No.

R.L. Surface

FT.

R.L. Collar

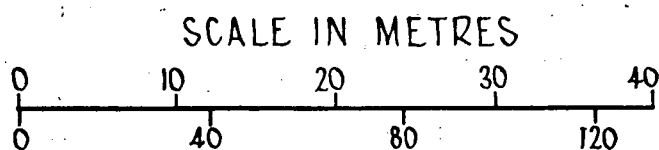
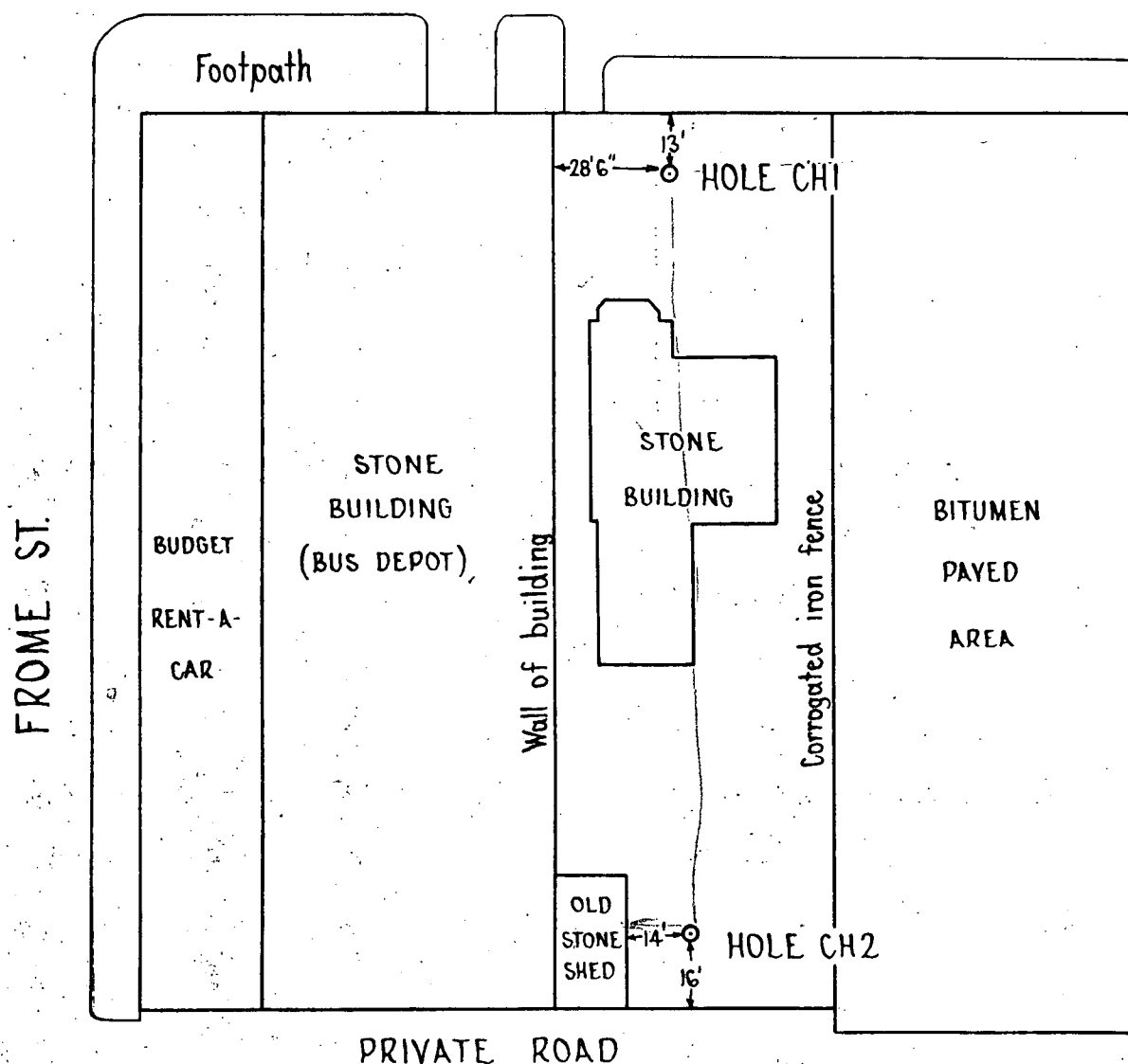
FT.

Datum

GEOLOGICAL NOTES AND CLASSIFICATION		RL (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	MOISTURE CONTENT	Consistency	Compact Density	FIELD TEST DATA			
										BLOWS PER FOOT 20 40 60 80	SOIL TEST P.T.R. METER Units * 1 2 3 4		
PLEISTOCENE - RECENT	ALLUVIUM	35		CL-CH	* 24 ft. Zone of lime accumulation  As for Sheet 1.					SPT 31 Blows			
		40		CL-CH						SPT 26 Blows			
		45		ML-MH	Gradational Change SILT SOIL, moderate plasticity Grey micaceous clayey silt, orange-brown mottled	W MC							
		50		CL-CH	CLAY SOIL, moderate plasticity Nutty, grey, silty clay	≈ PL St				SPT 12 Blows			
		55		SM	SAND, excess silty fines Grey micaceous silty fine sand with thin (1cm) bands of grey, moderately plastic clay	W MD				SPT 17 Blows			
PLEISTOCENE	CARBONACEOUS SAND	60		CL-CH	CLAY SOIL, moderate plasticity Nutty, grey, silty clay	≈ PL St				SPT 11 Blows			
				ML-MH	SILT SOIL, moderate plasticity Grey micaceous clayey silt	W MC							
				SM	SAND, excess silty fines. Grey and orange brown micaceous silty fine sand	W MD							
				SP	SAND, poorly graded. Micaceous quartz sand with rare pebbles END OF HOLE 60 ft	S MD							

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS. — Very Soft	Ls — Loose	VL — Very Loose	H — Humid	DRILL No. <b>2</b>	LOGGED BY <b>R.F. JEUNE</b>
D " (SD)	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	TYPE <b>RUSTON</b>	DATE <b>20-10-71</b>
E " (SE)	F — Firm	C — Compact	MD — Medium Dense	M — Moist	DRILLER <b>STURAK</b>	TRACED <b>DJM</b>
G " (SG)	St. — Stiff	VC — Very Compact	D — Dense	W — Wet	START <b>13-10-71</b>	CHECKED <b>WJ8</b>
Sealed Tube — SAL	V. St. — Very Stiff		VD — Very Dense	S — Saturated	FINISH <b>18-10-71</b>	
Standard Penetration Test-SPT	H. — Hard			LL — Liquid Limit	SHEET <b>2 OF 2</b>	DRG No. <b>S9551-a Ha6</b>
	* These values refer to clay soils only and provide an indication of their consistency.					
				PL — Plastic Limit		

# NORTH TERRACE



SCALE IN FEET

Compiled from S.A. Public Buildings Dept. Plan No 1132-Su-71

FIG. 1.

ENGINEERING GEOLOGY SECTION	DEPARTMENT OF MINES - SOUTH AUSTRALIA	Scale: As shown
Compiled: R. Jeune	CHEST CLINIC - NORTH TERRACE	Date: 27 Oct 1971
Drn. R.H. Ckd. R.F.J.	SITE PLAN	Drg. No. S9543 Ha5