

DEPARTMENT OF MINES
SOUTH AUSTRALIA

$$\begin{array}{r} 13.5 \\ 3 \\ \hline 40.5m \end{array} \quad \text{GWL} = \frac{44}{8} = 40$$



GEOLOGICAL SURVEY
ENGINEERING DIVISION

GOVERNMENT OFFICE BUILDING - VICTORIA SQUARE WEST

TA.335 Hd. Adelaide

GEOLOGICAL INVESTIGATIONS - REPORT NO.1

DESIGN STAGE

- Public Buildings Department -

by

W.R.P. BOUCAUT
SUPERVISING GEOLOGIST
ENGINEERING DIVISION

11th July, 1970

Rept.Bk.No.70/105

69-20

70/105

DEPARTMENT OF MINES
SOUTH AUSTRALIA

Rept. Bk. No. 70/105
G.S. No. 4493
D.M. No. 968/69

GOVERNMENT OFFICE BUILDING -
VICTORIA SQUARE WEST

TA. 335 Hd. Adelaide

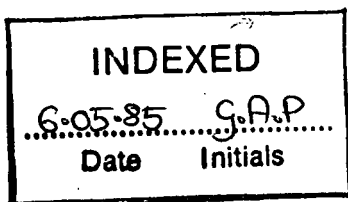
GEOLOGICAL INVESTIGATIONS - REPORT NO. 1

DESIGN STAGE

Client: Public Buildings Dept.

SUMMARY AND CONCLUSIONS

Drilling has indicated a relatively uniform near horizontal succession consisting of 40 to 50 ft. of stiff to very stiff* clay (Hindmarsh Clay formation) overlying 27 to 32 ft. of medium strong calcareous sandstone (Hallett Cove Sandstone formation), which in turn overlies at least 43 ft. of moderately compact silts and medium dense sands (Blanche Point Marls). The clay is limy and weaker in its upper 5 to 10ft. The sandstone contains numerous solution holes from a few mms. to 3ft. in size.



* Terms Underlined are defined in Appendix A.

DEPARTMENT OF MINES
SOUTH AUSTRALIA

GOVERNMENT OFFICE BUILDING -
VICTORIA SQUARE WEST

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<u>Fig. No.</u>	<u>FIGURES</u> <u>Title</u>	<u>Plan No.</u>
1	Government Office Building, TA.335, Hd. Adelaide, Locality Plan and Geological Section	70-379
2	Government Office Building, Victoria Square West, Observation Holes, Water Level Measurements	S.7850

11th July, 1970

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The main groundwater table occurs in the Hallet Cove Sandstone at about 53 ft. below ground surface. Perched groundwater occurs in limy horizon of the Hindmarsh Clay formation, probably associated with gilgai structures.

The clays of the Hindmarsh formation, below the weaker near surface materials (limy clay), could be utilized for raft-type foundation.

The Hallett Cove Sandstone would provide a suitable base for end-bearing piles. However its nature and thickness should be proved at each pile site, either before or during construction, as it is known that solution holes up to 20 ft. diameter can occur.

INTRODUCTION

Drilling to determine foundation condition beneath the proposed State Government Office Building, Victoria Square West was requested in a letter from the Director, Public Buildings Department, dated 20th October. The holes were to be put down by a combination of cable tool and diamond drilling, and were to be preserved for use as drainage bores beneath the building, as finally constructed.

The building will occupy an area of about 180 ft. by 100 ft. and will be multi-storey with a basement.

Four holes (CH 1 to 4) were put down at the corners of the proposed building. The holes were put down with a cable-tool rig to the limit of open tube sampling, usually about 50 ft. below ground surface, and then extended through rock for a further 24 to 33 ft.

This section in rock was then reamed out by a cable tool rig and the hole completed by open tube sampling. Sealed tube samples were taken continuously for the first 15 ft. of each hole (that is over the extent of the proposed basement and then at specified intervals for the remainder of the hole. Each hole has been preserved with plastic piping, perforated and screened in the bottom 20 ft, so that the hole can be used as a drainage bore beneath the basement of the building. Steel casing installed during drilling has also been left in each hole to assist in preservation during construction. This will be recovered during basement excavation.

Within about 3 ft. of each bore a further drill hole was put down to a depth of 15 ft. and cased with plastic pipe. These holes will be used to monitor "perched" ground water levels close to the ground surface.

Geological logs of the holes are included in Appendix A and their results are summarized in section on Figure 1.

This report discusses the drilling results, and past experience in drilling and excavations elsewhere in the Adelaide city area.

SITE GEOLOGY

Soil and Rock Types

The drilling indicates a rather uniform near horizontal succession.

Results are summarized in Table 1. The Hallet Cove Sandstone is divided into two layers on the basis of strength of material. Layer A, the weaker zone, probably represents a weathered zone at the top of the sandstone, and appears to be variable in thickness across the site.

TABLE I

GEOLOGY OF SITE AREA AS INDICATED BY DRILLING, AND DESCRIPTION OF MATERIALS

DEPTH (FEET)		THICKNESS (FEET)		G E O L O G Y			E N G I N E E R I N G P R O P E R T I E S	
FROM	TO	MAX.	MIN.	AGE	UNIT & ENVIRON- MENT OF DEPOSITION	NOTES ON LITHOLOGY	DESCRIPTION	CONSISTENCY ETC., MOISTURE CONTENT
0	45-50	50	40	Pleistocene	Hindmarsh Clay Fluvio- Lacustrine	Mainly clay. Grey green to red-brown mottled, limy in upper 5-10 ft. A bed of clayey sand 4-7 ft. thick occurs about 28 ft. below the ground surface.	Clay SOIL, high plasticity (CH) Lower plasticity (CL) & limy (ML) in upper 5-10ft. Bed of SAND excess clayey fines (SC) to SAND poorly graded (SP) at depth of 28ft.	Clay is <u>stiff</u> to very stiff. Moisture content at about plastic limit. Sand is <u>dense</u> and humid to moist.
45- 45-50	47-55	9 (Hole CH3)	1 (Hole CH2)	Pliocene	Hallett Cove Sandstone, Shallow Marine.	Layer A Lime gravel with a white silt- clay matrix. Gravel is sandstone frag- ments.	SILT SOIL, low plas- ticity, (ML), white with up to 50% GRAVEL fragments to 30mm size.	Silt is <u>moderately</u> <u>compact</u> at moisture content greater than plastic limit.
47- 47-55	76.3- 78	31	22.5			Layer B Sandstone, fine grained, foss- iliferous, white to buff coloured, with calcareous cement. Contains solution holes from a few mms to 3 ft. in size - usually infilled.	Sandstone has rock properties. Infill in CLAY SOIL low plasticity, sandy with lime in silt sizes.	Rock is <u>medium</u> <u>strong</u> to <u>very</u> <u>strong</u> . Clay is <u>stiff</u> . Materials are saturated with moisture content of clay greater than plastic limit.
76.5- 76.5-81- 78	>81- >120 (End of holes)	>43	>5	Eocene	Blanche Point Marls. Marginal marine.	Mainly silt or sand beds, some micaceous. Fossiliferous (shelly) in part. Light grey (glaucon- itic) to dark brown (micaceous). Sand is mainly quartz fragments.	SILT SOIL High plasticity, sandy (MH) or SAND poorly graded (SP), to with excess silt/clay fines (SM/SC). Sand is fine to medium grained.	Silt is <u>moderately</u> <u>compact</u> , with mois- ture content less than or equal to plastic limit. Sand is <u>medium</u> <u>dense</u> and saturated.

Core losses recorded while drilling in the sandstone probably represent cavities, or lenses of sandy material washed away during drilling.

Groundwater

The main groundwater table was cut within the Hallett Cove Sandstone at about 53 ft. below the ground surface. The sandy horizon within the Hindmarsh Clay was wet but no groundwater inflows were recorded.

During drilling of the shallow observation holes in the summer period of February-March, groundwater was only encountered in hole CH3 observation. In this hole a small seepage occurred from a depth of about 4 ft, but water level in the hole was static at about 13.5 ft. until early April. Since the onset of periods of rain at this time, water has been recorded in all observation holes as shown on Figure 2.

Tests on a sample of the water taken when first cut in hole CH3 Observation, showed that the water was naturally occurring and was not from leaking service lines.

DISCUSSION

Hindmarsh Clay

Drilling results and experience at other sites, suggest that these clays are fairly uniform in lateral extent (Fig.2). They are highly plastic (CH) soils, stiff or very stiff, and usually at moisture content close to their plastic limit.

The clay is intersected by two sets of near-vertical joints roughly at right angles to each other, spaced about 5 cm. apart, which tend to divide

the mass into a series of vertical columns. Other less regular low dipping (30° to 45°) joints occasionally occur. (Ref. 1 and 2).

The upper 10 ft. of clay is limy and is considerably weaker than the underlying clay. It is probable that the upper surface of the clay is irregular and deformed into a series of mounds and depressions (gilgai structures) (Ref. 3). These depressions often contain pools of perched groundwater.

The sand, clayey sand, or clayey silt horizon about 30 ft. below the ground surface is dense to medium dense.

The following notes are made based on experience in similar materials elsewhere in the Adelaide City area.

....Auger drilling is feasible through the Hindmarsh Clay formation.

Casing is generally required as any water cut can weaken the clays and cause collapse. (Ref. 4).

....Excavation can be readily carried out in the Hindmarsh Clay formation by mechanical equipment. The joints control the overall strength of the clay, which may be significantly lower than that obtained in the laboratory by testing of small clay samples (Ref. 5). In most cases of failure of excavation walls in the city, failure has occurred along pre-existing joint planes. However, in most of these failures, water has been allowed to seep into the joints, thus weakening the clay mass further (Ref. 3). Seepage of water from "gilgai" structures is often a major cause of these failures.

....Permanent drainage of water from leaking pipes and perched water tables is usually desirable in buildings with basements.

Hallett Cove Sandstone

All holes penetrated the base of this formation, and showed a total thickness of Hallett Cove Sandstone formation of from 27 to 32 ft.

Observations at other sites in the city, and variations between each drill hole at the site, indicate that the sandstone is extremely variable both laterally and vertically. The rock substance ranges from very weak to strong, and the rock mass is weakened by joints, solution cavities, clay seams or pockets, and lenticular beds of silt and sand.

The upper surface of the sandstone is also highly irregular in shape. Buried erosional pinnacles and sink holes are common. One large sinkhole approximately 20 ft. in diameter was discovered during pier boring at the Public Library Site, Kintore Avenue, (Ref. 4).

The following notes are made based on experience in similar materials elsewhere in the Adelaide city area:-

....Piers have founded in the Hallet Cove Sandstone, usually at the top of Layer B. As the material in this formation is usually very variable, it is advisable to prove the presence and thickness of sandstone at each pier site, usually by visual examination and drilling at each pier base during construction. (Ref. 6).

....Belling in Layer A by machine generally has not been possible and hand mining methods have been used.

....Cement grouting has been used in an attempt to strengthen the Hallett Cove Sandstone. Generally, it has been of doubtful value due to clay and fine sand in joints and cavities, preventing the penetration of grout. The grout takes can vary considerably

depending on the number of solution cavities and fissures intersected.

Blanche Point Marls

Hole CH1 penetrated some 43 ft. of this formation which consists of fairly strong soils, mainly compact silts and medium dense sands. At depth the soils become micaceous.

WRPB:PMM
11th July, 1970

W.R.P. BOUCAUT
SUPERVISING GEOLOGIST
ENGINEERING DIVISION

REFERENCES

- STAPLEDON, D.H. 1970. Changes and Structural Defects developed in some South Australian clays, and their engineering consequence. Symposium on Soils and Earth Structures in Arid Climates, Adelaide May 1970, The Institution of Engineers, Australia.
- COX, J.B. 1970. A review of the geotechnical characteristics of the soils in the Adelaide City area. Symposium on Soils and Earth Structures in Arid Climates, Adelaide May 1970, the Institution of Engineers, Australia.
- ALLCHURCH, P.D. Gilgai structures in "mottled clays" beneath the City of Adelaide. Geol. Survey, S. Aust. Quart. Geol. Notes No. 14.
- ALLCHURCH, P.D. Karst topography of the Hallett Cove Sandstone in the Adelaide City Area. Geol. Survey. S. Aust. Quart. Geol. Notes No. 24.
- PROF. SKEMPTON & LAROCHELLE 1965. The Bradwell slip: a short term failure in London Clay. Geotechnique Vol. XV No. 3, 1965.
- AMERICAN STANDARD BUILDING CODE. Requirement for Excavations and Foundations. Appendix p.18, clause 2.1 A.S.C.E. Manual of Engineering Practice No. 32 (A.S.A. Document A56.1-1952).

APPENDIX A

LOGS OF DRILLHOLES AND EXPLANATORY NOTES

APPENDIX A

LOGS OF CABLE TOOL HOLES AND EXPLANATORY NOTES

NOTES ON DRILLING PROCEDURES

Equipment

The drilling is carried out with a cable tool drilling plant using sampling tubes attached, through a vacuum head, to the sampling tools (Figs. 1 and 2).

Sampling Procedures

To obtain, for logging purposes, an almost continuous series of samples, with a relatively small amount of sample disturbance, SA type samples are taken. These are obtained by driving an "S" tube, fitted with a Mark A shoe (Fig. 2), into the material to be sampled.

The assembly is lowered carefully to the bottom of the hole, and the tube driven exactly 1 foot, and the number of blows required for the 1 foot of penetration recorded.

The samples, or core, is extruded from the sampling tube using an hydraulic ram. The extruded core is sealed in a labelled plastic bag and stored in a core box (Fig. 3).

The hole is reamed with a "D" or "E" shoe (Fig. 2) and then the next sample is taken, using the same procedure as above. Thus the hole proceeds by alternate sampling, reaming (and where required, casing) operations, and the samples form a continuous record of the materials penetrated except for a few inches which may be lost between samples during reaming operations.

SA sampling equipment is a composite sampler for simple class sampling. Details are as follows:

"S" SERIES CUTTING SHOES

MARK	FEATURES	USES
A	Inside clearance 3%. Area ratio 33%.	Continuous open-tube sampling in strong soils, in which it causes little deformation. Hole is reamed after each sample.
D	Shoe belled out to 4 29/32 in. (just greater than outside dia. of vacuum head)	Continuous open tube sampling where considerable deformation of sample is permissible. Essentially self-reaming.
E	Shoe belled out to 5 7/16 in. (just less than internal dia. of 6 in. casing)	Cleaning hole and reaming out hole.

SAL Samples

Sealed tube samples, for laboratory testing, are taken at various intervals during drilling.

The drilling procedure is similar to that for obtaining SA Samples, but in order to completely fill the sampling tube, it is driven 1.5 ft. into the material to be sampled. On removal from the drill hole, both ends of the sampling tube are sealed with paraffin wax and screwed caps are fitted to the tube. The sealed tubes are labelled as in Fig. 4.

Standard Penetration Test

The Standard Penetration Test (Ref. 1) is used to test the in-situ density of sands and to give an indication of the consistency of clays, and compactness of silts. However, the test results can be effected by several geological factors such as degree of cementation, and size and shape of grains. These factors should be taken into account in interpretation of results.

The equipment is illustrated in Fig. 5 and consists of a 2 in. diameter, sampling spoon (tube) and a hammer of standard weight (140 lb.).

With the equipment assembled as in Fig. 5 the hammer is allowed to fall on to the drill rods until the sampling shoe has penetrated 6 in. into the soil. The Standard Penetration is the number of blows (N) required to produce the next foot of penetration.

NOTES ON DRILL LOG SHEETS

The logs are plotted on a standard cable-tool log form.

Near the centre of the form a graphic log of the materials encountered is shown.

In the column to the right of the graphic log, the soils are classified and described according to the Unified Soil Classification (Ref. 2), (Fig. 6).

To the left of the graphic log is a geological description of the materials sampled. This includes:-

..... Geological age	}	printed vertically
..... Soil unit name		
..... Type of material		
..... Mineral composition		
..... Grain shape		
..... Cementation		
..... Organic materials		

Water levels are indicated by a small arrow with the date at which the observation was made.

In the blows per foot column, a continuous histogram is made of the number of blows required to drive the sampling tube through each foot of material. A hatching code is used to distinguish various types of sample. This code is reproduced at the bottom of each log sheet.

In the column on the far right of the log sheet, readings of unconfined compressive strength (q_u) made with a Soiltest Penetrometer, are recorded. The readings are plotted as a histogram. The Soiltest Penetrometer only gives true values of q_u when used in clays in which $\phi = 0$.

REFERENCES

1. TERZAGHI, K. and PECK, R.B. 1948. Soil Mechanics in Engineering Practice. John Wiley and Sons.
2. UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF RECLAMATION, 1966 Earth Manual, 2nd Edition.

APPENDIX

LOGS OF DIAMOND DRILL HOLES AND EXPLANATORY NOTES

NOTES ON DRILLING PROCEDURES

Equipment

The core sizes are as follows:-

<u>Symbol</u>	<u>Nominal Diameter of Cores (inches)</u>
NXC (NX casing)	2.8
NMLC	2.0
BMLC	1.4

The NMLC and BMLC cores were obtained with "M" type stationery inner tube core barrels fitted with bottom discharge bits. The inner tubes were of the split type, ensuring minimum disturbance of the core during removal from the barrel.

Storing and marking of core

Cores are stored in wooden boxes, each compartment of which is designed to contain five feet of core. The internal length for each compartment is actually five feet one inch, to allow for 100 per cent core recovery. Roughness of the ends of the core, and small inaccuracies in measurement when breaking it to fit the box, make it difficult to fit five feet of core in a compartment of exactly that length. The boxes are marked with consecutive compartment numbers at one end, and the drilled depths from the surface in feet at the other.

The core was boxed in this manner at the drill site, the core being placed in its appropriate place in the box as soon as it was extracted from the core barrel. The bottom of each lift was marked with paint or indelible ink immediately it was placed in the box, and a corresponding mark made on the side of the core box. The measured depth of the hole in feet from the surface was painted on the side of the core box and on the core. Timber blocks cut to the correct length indicate core not recovered (red blocks), and core removed for testing (white blocks).

The core has been stored at the Department of Mines, Drilling and Mechanical Branch, Dalglish Street, Thebarton, South Australia.

NOTES ON DIAMOND DRILL LOG SHEETS

The logs are plotted on a vertical scale of one inch = 10 feet (1:120) or one inch = five feet (1:60). In the column headed "Log", places where core was obtained are shown by stippling. Places where core was lost are shown by blank spaces.

The descriptions given on the log sheet refer only to materials recovered as core. Core is lost by the material being

ground or washed away during the drilling process; it may usually be inferred that such material is relatively weak. The weakness may arise from weathering or else from sheared, crushed, or closely jointed rock. It cannot always be assumed that the material not recovered is weak, since even solid rock core may be ground away and lost during drilling operations.

To the left of the graphic log is a geological description of the materials sampled. This includes:-

... Geological age)	
... Rock unit name)	Printed vertically
... Type of material)	
... Mineral composition		
... Cementation		
... Physical description of core		

Classification of the rock substance in terms of its strength and its condition (eg. weathering, alteration) is shown graphically in the column "Strength Term". The terms used in the classification are defined in Table 1. Where the substance has soil properties this is shown graphically in the column, and immediately to the left of the column under "Group Symbol", the symbol representing the remoulded sample as classified under the Unified Soils Classification (USBR 1966) is given.

The "Fracture Log" to the right of the graphic log column shows the degree of fracturing of the core by means of a histogram-type plot. Degree of fracturing means the degree to which the rock has mechanically broken up along geological defects such as joints, cleavage planes, foliation planes, bedding planes or seams. Fresh fractures across the fabric of the rock, not along the existing planar geological defects, are not included. In sections in which no core was recovered, the fracture log column is left blank.

In the column marked "Structures" the angles shown on joints, bedding, or other geological structures are the angles which they make with the plane at 90° to the axis of the core, unless otherwise stated.

Percentage loss of drilling water as recorded by the driller is shown graphically in the column "Drill Water Loss %".

REFERENCE

1. UNITED STATES BUREAU OF RECLAMATION 1966, Earth Manual 2nd Edition.

DESCRIPTIVE TERMS

1. CLAY SOILS

CONSISTENCY

CONSISTENCY	SYMBOL	UNCONFINED COMPRESSIVE STRENGTH (kg/sq. cm)	FIELD TEST	N
Very Soft	V.S.	less than 0.25	Easily penetrated several inches by fist.	2
Soft	S	0.25 to 0.5	Easily penetrated several inches by thumb.	2 to 4
Firm	F	0.5 to 1.0	Can be penetrated several inches by thumb with moderate effort.	4 to 8
Stiff	St	1.0 to 2.0	Readily indented by the thumb but penetrated only with great effort	8 to 15
Very Stiff	V.St.	2.0 to 4.0	Readily indented by thumb nail.	15 to 30
Hard (Extremely stiff)	H	over 4.0	Indented with difficulty by thumb nail.	30 and over

Based partly on Terzaghi, K. and Peck, R.B. 1966. Soil Mechanics in Engineering Practice, Wiley - New York.

MOISTURE CONTENT

Abbreviation	Meaning
MC = LL	Moisture Content near liquid limit.
MC < LL	" " less than liquid limit.
MC > PL	" " greater than plastic limit.
MC ≈ PL	" " near " "
MC ≤ PL	" " less or equal to plastic limit.
MC < PL	" " less than " "
MC << PL	" " much less than " "

2. SILT SOILS

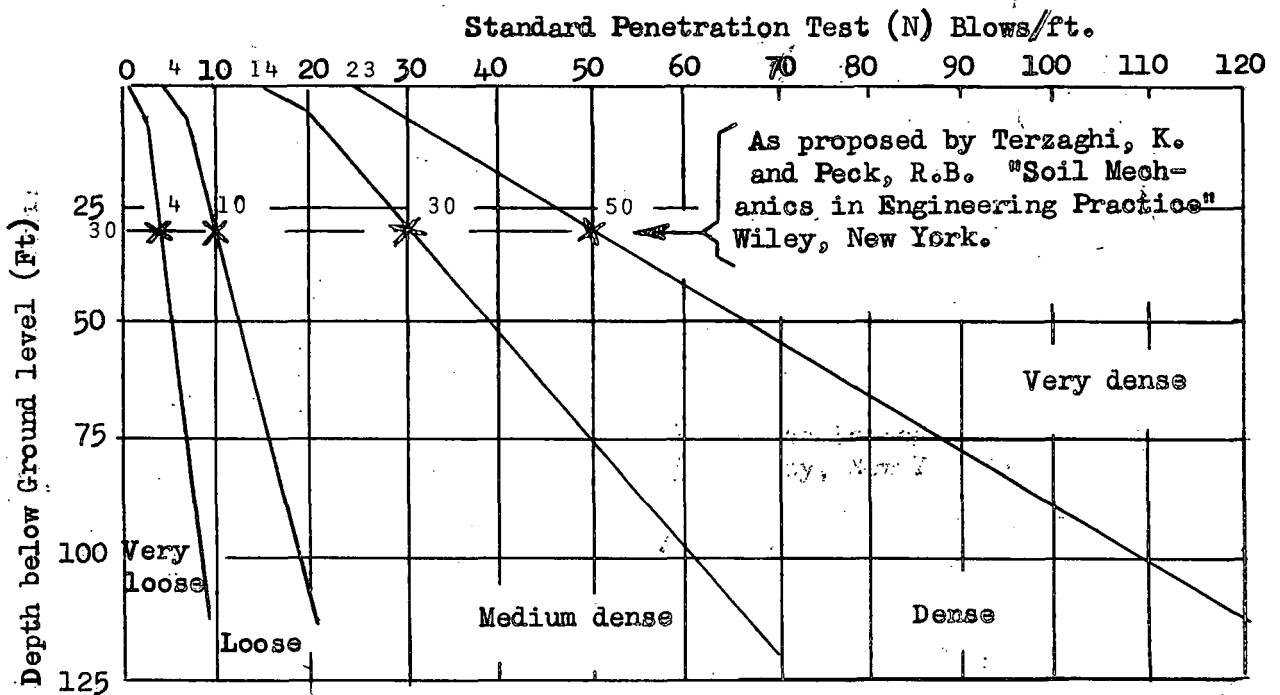
COMPACTNESS	SYMBOL	N
Loose	LS	0 to 8
Moderately compact	MC	8 to 15
Compact	C	15 to 30
Very Compact	VC	greater than 30

3. SANDS

CLASSIFICATION OF SANDS BY STANDARD PENETRATION TEST

The relative density of granular soils has been judged from the results of Standard Penetration Tests carried out by the procedure described by Terzaghi and Peck (1948) bearing in mind the limitations of the method as discussed by Gibbs and Holtz (1957). At all times the water in the drill hole was kept at the level of surrounding groundwater.

EFFECT OF OVERBURDEN PRESSURE ON STANDARD PENETRATION TEST



Based on Gibbs, H.J. & Holtz, W.G. (1957) "Research on Determining the Density of Sands by Spoon Penetration Testing" Vol. I Proc. 4th Int. Conf. SM & FE, London.

REFERENCES

- TERZAGHI, K., and PECK, 1948. "Soil Mechanics in Engineering Practice". Wiley. New York.
- GIBBS, H.T. and HOLTZ, W.G., 1957. Research on Determining the Density of Sands by Spoon Penetration Testing. Proc. 4th Inter. Conf. SM & FE, London, Vol. 9.

ENGINEERING
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SECTION

BRADY
TAYLOR
CHALK
BYRON

CABLE-TOOL DRILLING
SAMPLING TOOL ASSEMBLY
WITH "S" SERIES SHOES

FIG. 1

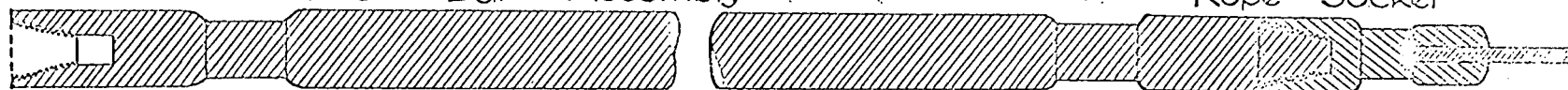
DEPARTMENT OF MINES - SOUTH AFRICA

"S" Series Percussion Sampling Tool Assembly



Sinker Bar Assembly

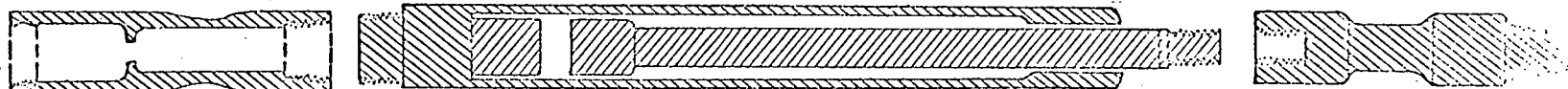
Rope Socket



Vacuum Head

Sampling Tool

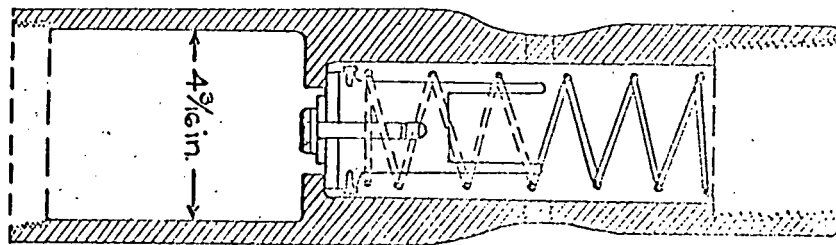
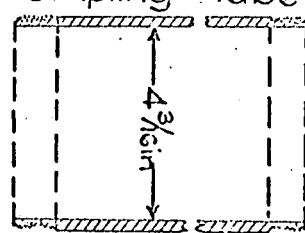
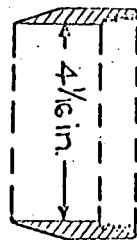
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"A" Shoe

Sampling Tube

Vacuum Head



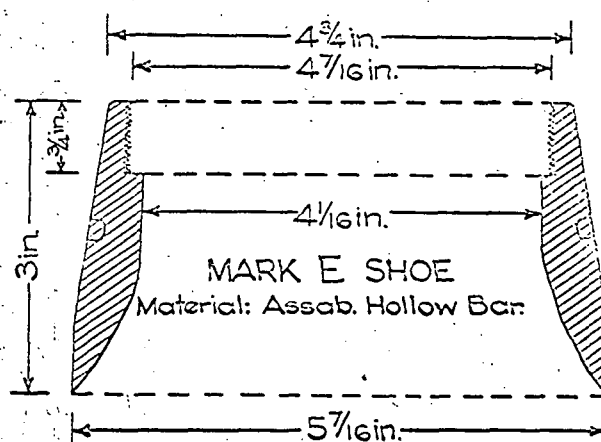
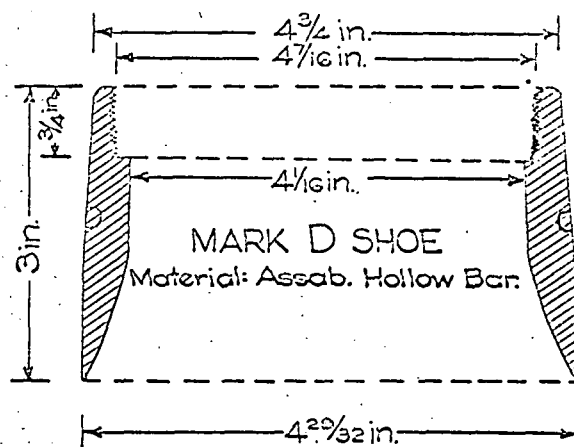
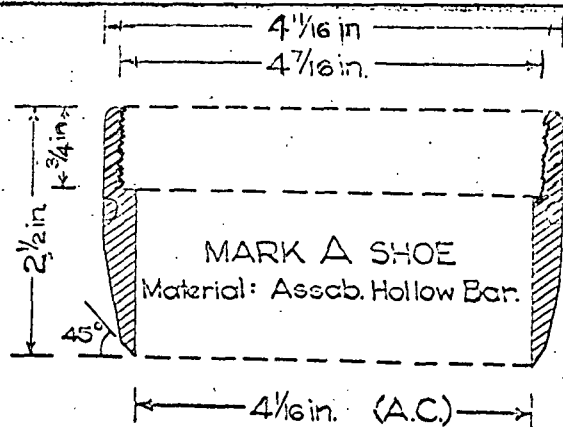
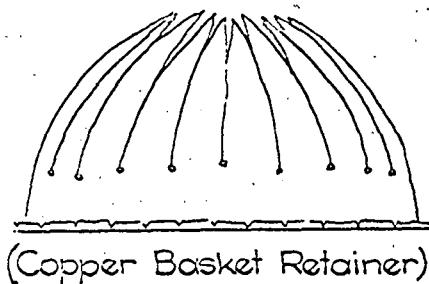
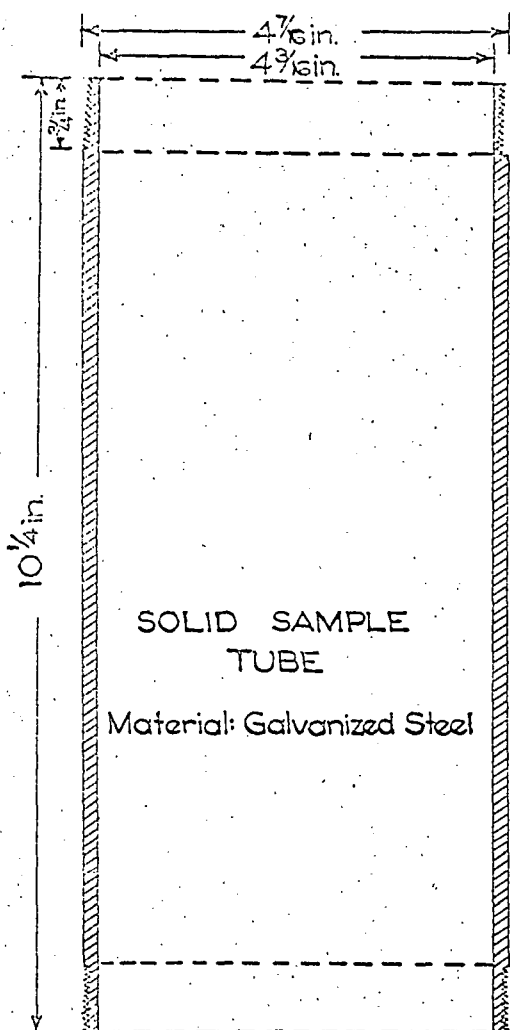


FIG: 2

DEPARTMENT OF MINES — SOUTH AUSTRALIA

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Drm D.H.D.

Tcd AMB

Chd. L.V.W.

Exd. J.W.P.

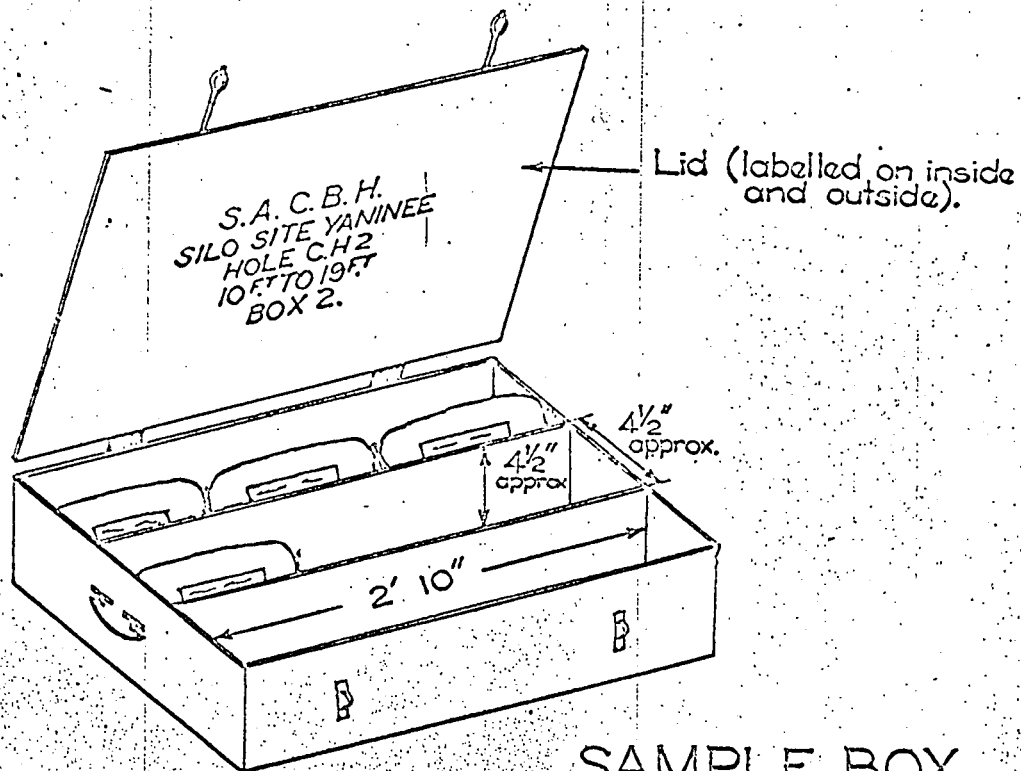
CABLE-TOOL DRILLING
S SERIES SAMPLING
TUBE AND SHOTS

SCALE: NOT TO SCALE

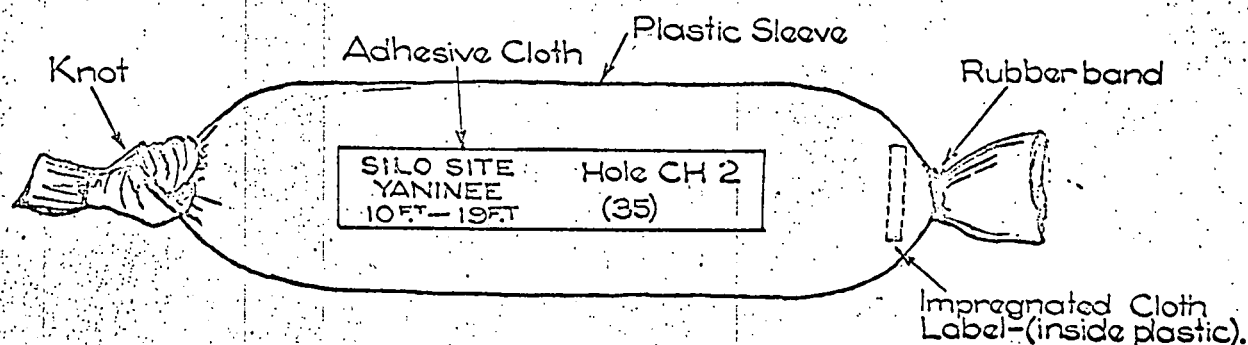
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Date: 21 Oct '69

SENIOR GEOLOGIST



SAMPLE BOX



EXTRUDED SAMPLE
SEALED IN PLASTIC SLEEVE.

FIG: 3

ENGINEERING DIVISION	DEPARTMENT OF MINES - SOUTH AUSTRALIA	Scale:
Compiled: W.R.P.B		Date: 17 Dec 1960
Drawn: Gkd.	CABLE TOOL DRILLING LABELLING AND BOXING OF EXTRUDED SAMPLES	Dwg No 37580 MB

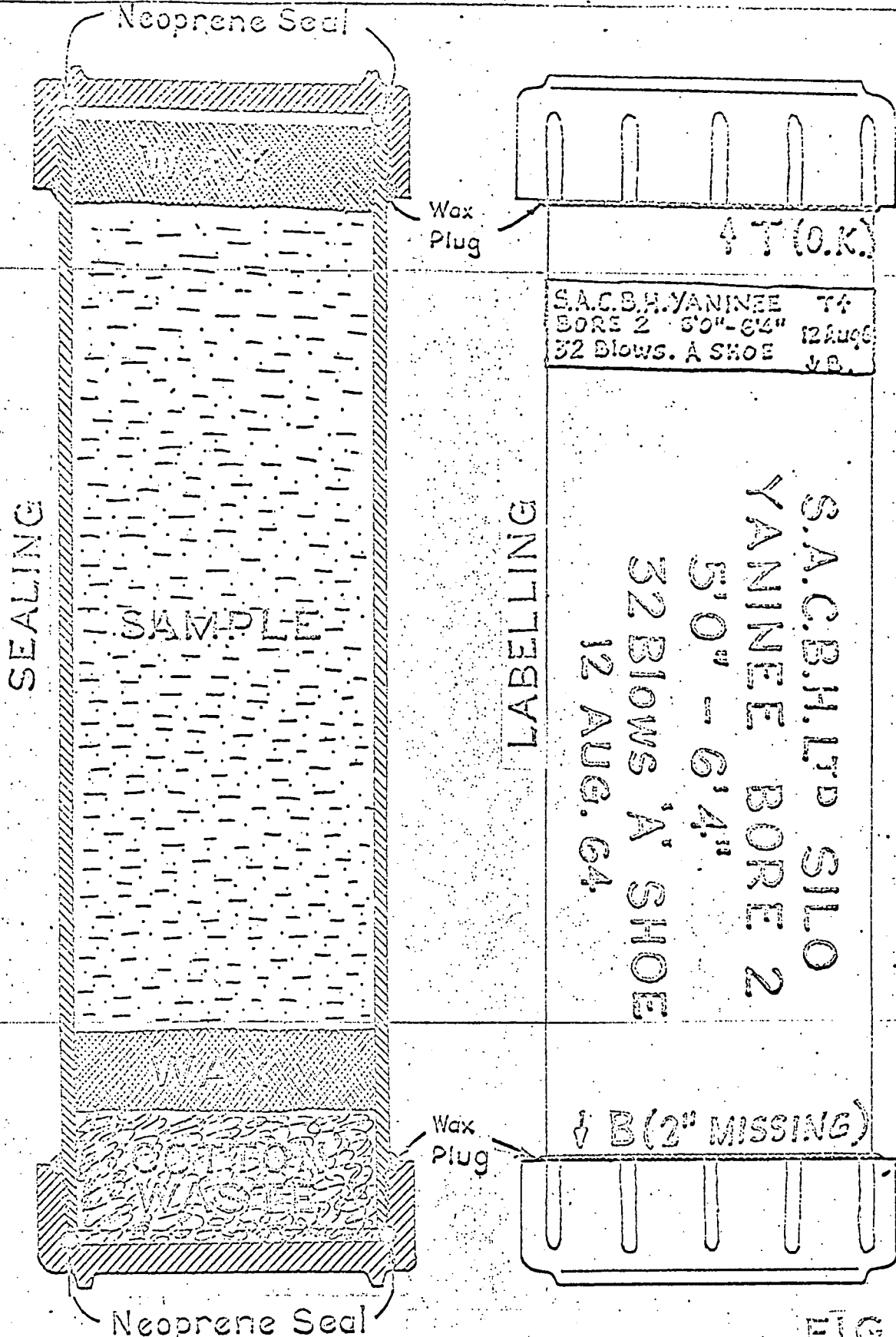


FIG. 4

DEPARTMENT OF MINES — SOUTH AUSTRALIA

ENGINEERING SERVICES SECTION	Drn. R.D.S.	SEALED TUBE SAMPLE	SCALE: NOT TO SCALE
	Tcd. R.D.S.		S 4419 MB
DIRECTOR OF MINES	Ckd. A.H.S.		DATE: 29 JUNE 65
	Exd.		

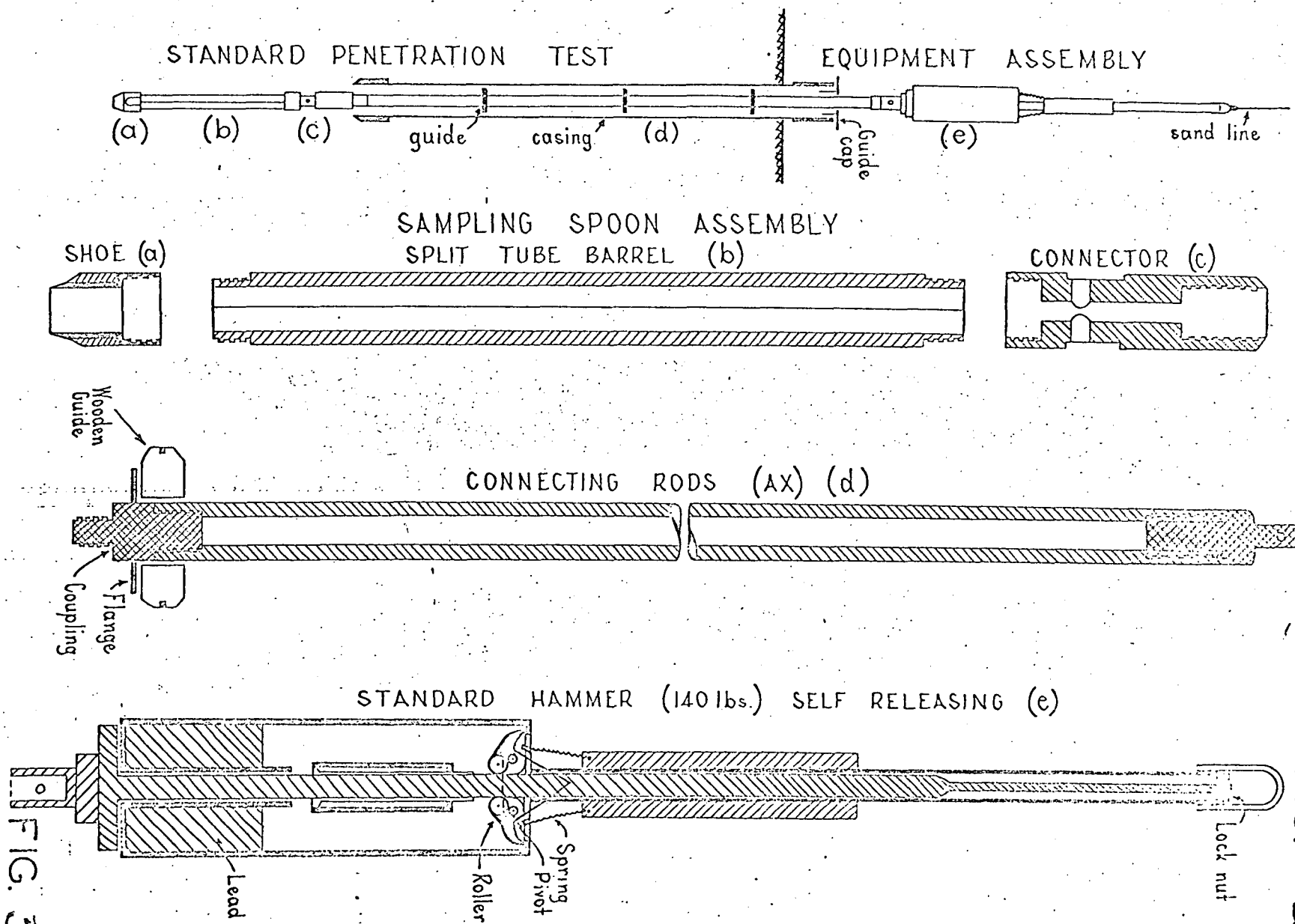


FIG. 5

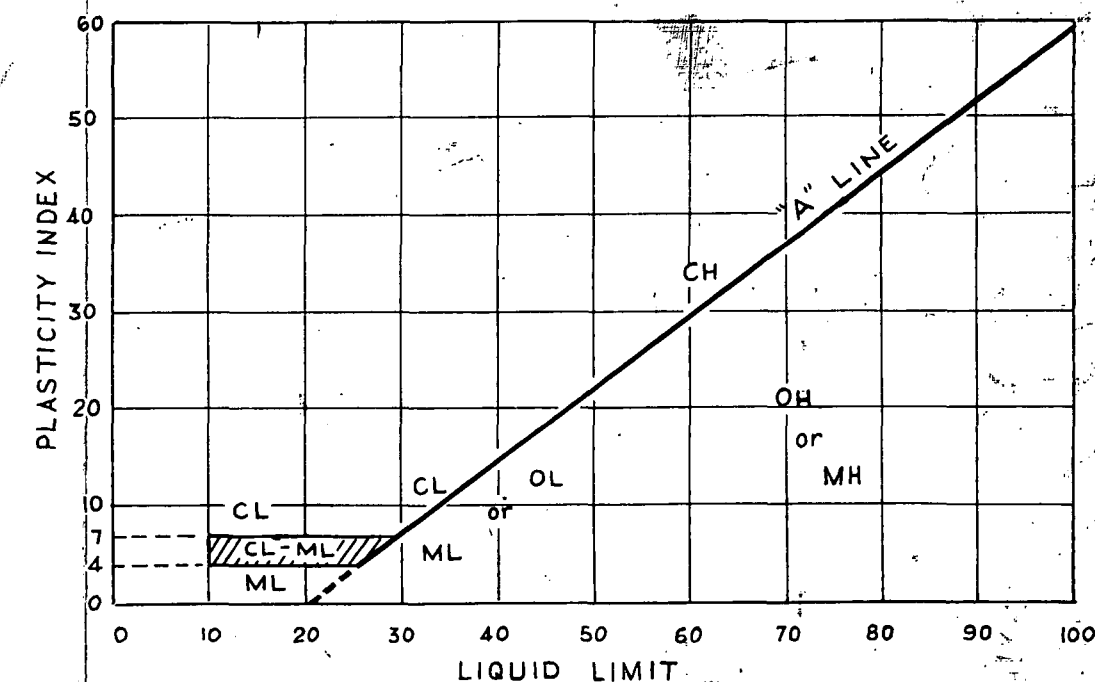
FIG. 5

ENGINEERING DIVISION Compiled: W.R.P.B. Dn. R.H. Ckd.	DEPARTMENT OF MINES - SOUTH AUSTRALIA STANDARD PENETRATION TEST EQUIPMENT	Scale: Not to scale Date: 22 Dec 1969 Drg. No. S4420 MB
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FIELD IDENTIFICATION PROCEDURES (Excluding particles larger than 0.25 ft. and basing fractions on estimated weights)				GROUP SYMBOL	GROUP NAME and typical materials	LABORATORY CLASSIFICATION CRITERIA			
COARSE GRAINED SOILS More than 50% of material is larger than No. 200 B.S. Sieve size.	GRAVELS More than 50% of the coarse fraction is larger than 2 mm (retained on B.S.7 sieve)	CLEAN GRAVELS Little or no fines	Wide range in grain size and substantial amounts of all intermediate particle sizes.	GW	GRAVEL, well graded; gravel sand mixtures, little or no fines	Coarse grained soils classified on basis of percentage of fines, as follows PERCENT OF FINES GRAVELS Less than 5 More than 12 5 to 12 SANDS SW, SP SM, SC Borderline cases, use 2 symbols	$C_u = \frac{D_{60}}{D_{10}}$ Greater than 4 $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ Between one and 3		
			Predominantly one size, or a range of sizes, with some intermediate sizes missing.	GP	GRAVEL, poorly graded; gravel sand mixtures, little or no fines.		Not meeting all gradation requirements for GW		
		DIRTY GRAVELS Appreciable amount of fines	Non-plastic fines - for identification see ML below.	GM	GRAVEL, excess silty fines; poorly graded gravel-sand-silt mixtures		Atterberg limits below "A" line or PI less than 4		Above "A" line with PI between 4 and 7 are borderline cases requiring use of dual symbols
			Plastic fines - for identification see CL below	GC	GRAVEL, excess clayey fines; poorly graded gravel-sand-clay mixtures		Atterberg limits above "A" line with PI greater than 7		
	SANDS More than 50% of the coarse fraction is smaller than 2 mm (passing B.S.7 sieve)	CLEAN SANDS Little or no fines.	Wide range in grain sizes, and substantial amounts of all intermediate particle sizes	SW	SAND, well graded; well graded sands, gravelly sands, little or no fines.		Not meeting all gradation requirements for SW		
			Predominantly one size or a range of sizes, with some intermediate sizes missing.	SP	SAND, poorly graded; poorly graded sands gravelly sands, little or no fines		Atterberg limits below "A" line or PI less than 4		
		DIRTY SANDS Appreciable amount of fines	Non plastic fines - for identification see ML below	SM	SAND, excess silty fines; poorly graded sand-silt mixtures		Atterberg limits above "A" line with PI greater than 7		
			Plastic fines - for identification see CL below	SC	SAND excess clayey fines; poorly graded sand-clay mixtures.		Atterberg limits above "A" line with PI greater than 7		

FINE GRAINED SOILS More than 50% of material is smaller than No. 200 B.S. sieve size.	FIELD INVESTIGATION PROCEDURES on fraction smaller than 0.4 mm. (passing B.S.36 sieve)							GROUP SYMBOL	GROUP NAME (and typical materials)
	SILTS AND CLAYS Liquid limit less than 50	SOIL CAST (wet soil)	SOIL THREAD	SHINE	DILATANCY	ODOUR	DRY STRENGTH		
		Forms fragile cast. Cracks form when kneaded while moist	Thick crumbly thread; easily broken	None to very dull	Distinct	Not significant	None to slight	ML	SILT SOIL, low plasticity; inorganic silts and very fine silty or clayey sands, rock flour.
		Cast may be handled freely without breaking. Can be kneaded moist without cracking. Material adheres to the hand.	Thread can be pointed as fine as a lead pencil, but is fragile.	Moderate	None to slight	Not significant	Moderate	CL	CLAY SOIL, low plasticity; inorganic clays of low to medium plasticity, gravelly clays, sandy clays silty clays, lean clays
	SILTS AND CLAYS Liquid limit more than 50	Cast fragile to cohesive material will adhere somewhat to the hand.	Soft, weak thread.	None to very dull	Slight to distinct	Decayed organic matter	Low	OL	ORGANIC SOIL, low plasticity; organic silts and silt clays of low plasticity
		Moderately plastic and cohesive. Material adheres somewhat to the hand	Weak to medium thread. May be crumbly.	Dull	None to slight	Not significant	Moderate. Powdered soil feels floury	MH	SILT SOIL, high plasticity; inorganic silts, micaceous or diatomaceous fine sandy or silty soils elastic silts.
		Very plastic and cohesive. Material very sticky to the hand. Greasy to touch.	Very tough thread. Can be rolled to a pin point.	Very glossy	None	Strong earthy.	High to very high. Cannot be powdered by finger pressure.	CH	CLAY SOIL, high plasticity; inorganic clays of high plasticity, fat clays
		Plastic and cohesive. Feels slightly spongy. Greasy to touch.	Weak to medium thread. Often soft and fibrous	Moderate to very glossy	None	Decayed organic matter	Moderate to high. Powdered soil may be fibrous.	OH	ORGANIC SOIL, high plasticity; organic clays of medium to high plasticity.
HIGHLY ORGANIC SOILS		Readily identified by colour, odour, spongy feel and frequently by fibrous texture.						PT	PEATY SOIL; Peat and other highly organic soils.

GRAIN SIZE CURVE to be used to identify soil fractions	PLASTICITY INDEX		LIQUID LIMIT		PLASTICITY CHART FOR LABORATORY CLASSIFICATION OF FINE GRAINED SOILS	
	60 50 40 30 20 10 7 4 0		0 10 20 30 40 50 60 70 80 90 100			



PLASTICITY CHART
FOR LABORATORY CLASSIFICATION OF FINE GRAINED SOILS

NOTE : BOUNDARY CLASSIFICATIONS : Soils possessing characteristics of two groups are shown as a combination of two group symbols, e.g. GW - GC, well graded gravel with clay binder.

Based on "The Unified Soil Classification System"
United States Department of the Interior
Bureau of Reclamation "Earth Manual"
First Edition, Denver COLORADO 1960

FIG. 6

PROJECT GOVERNMENT OFFICES
BUILDING

DEPARTMENT OF MINES SOUTH AUSTRALIA

LOG OF CABLE TOOL HOLE

TOWN AGRE 335

FEATURE FOUNDATION INVESTIGATION

HUNDRED ADELAIDE

LOCATION VICTORIA SQUARE WEST

CO-ORDS

HOLE CH 1

SERIAL No.

R.L. Surface: FT.

R.L. Collar: FT.

Datum

GEOLOGICAL NOTES
AND CLASSIFICATION

R.L. (FEET)
DEPTH

GRAPHIC
LOG

GROUP
SYMBOL

SOIL DESCRIPTION

GROUP NAME

Unified Soil Classification, U.S.B.R.
Earth Manual 2nd Edition 1966

WATER
LEVEL

CEILING

MOISTURE
CONTENT

Consistency

Compact Density

FIELD TEST DATA

BLOWS
PER FOOT
20 40 60 80

SOILTEST
P.T.R. METER
Units *
1 2 3 4

Calcareous

Silt is highly calcareous
Nodules are ferruginous
and calcareous.

ML-
SM

SILT SOIL, low plasticity,
very sandy (40%), light
brown.

CL

CLAY SOIL, low plasticity,
very sandy, grey. Contains
patches of silt (ML) and
some weakly cemented
nodules.

Sand content is 20 to 40%

CH

CLAY SOIL, high plasticity,
grey, mottled red-brown
and yellow-brown.
Contains few sand grains.

Decomposed rootlet,
1 m.m. diameter.

Decomposed rootlet
2 m.m. diameter.

Fines are
ferruginous.

SP
to
SC

SAND, poorly graded, fine
to coarse grained, 10 to
15% clayey fines, brown.

CH

CLAY SOIL, high plasticity,
grey, mottled yellow brown.
Few sand grains.

PLEISTOCENE
HINDMARSH CLAY

TYPE OF SAMPLE

A shoe (SA)
D " (SD)
E " (SE)
G " (SG)
Sealed Tube
A Shoe - SAL
Standard Pen-
etration Test - SPT

Water level (date)
WC
Water cut

CONSISTENCY
(Clays)
VS. - Very Soft
S - Soft
F - Firm
St. - Stiff
V. St. - Very Stiff
H. - Hard

COMPACTNESS
(Sands)
LS - Loose
MC - Moderately
Compact
C - Compact
VC - Very
Compact

RELATIVE
DENSITY (Sands)
VL - Very Loose
L - Loose
MD - Medium
Dense
D - Dense
VD - Very Dense

MOISTURE
CONTENT
H - Humid
D - Damp
M - Moist
W - Wet
S - Saturated
LL - Liquid Limit
PL - Plastic Limit

ENGINEERING GEOLOGY SECTION

DRILL No. 1
TYPE RUSTON
DRILLER D.R. PHILLIPS
START 25 Feb. 70
FINISH 26 Feb. 70

LOGGED BY
J.P.T.
DATE 8 Apr. 70
TRACED SLT.
CHECKED R.H.

SHEET 1 OF 4

DRG No. S7744

11x6

PF H° S6676 MB

GEOLOGICAL NOTES AND CLASSIFICATION	R.L. (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	MOISTURE CONTENT	CONSISTENCY	FIELD TEST DATA			
								BLOWS PER FOOT		SOILTEST P.T.R. METER	
								20	40	60	80
								1	2	3	4
PLEISTOCENE HINDMARSH CLAY	35		CH	As above with silty patches as shown, weakly cemented in part.							
	40			Mottling mainly red-brown, some black spots.							
	45			Up to 10% gravel, where shown.							
	50			Contains up to 20% fine grained sand.							
UPPER PLEISTOCENE HALLET COVE SANDSTONE	55			HOLE DIAMOND - DRILLED TO 79.0 FT. see separate sheet for detailed log							
	60										
	65										
	70										

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Sands)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
					DRILL No.	LOGGED BY
A - shoe (SA)	VS. - Very Soft	LS - Loose	VL - Very Loose	H - Humid	1	JPT
D - " (SD)	S - Soft	MC - Moderately Compact	L - Loose	D - Damp	RUSTON	DATE 8 Apr. '70
E - " (SE)	F - Firm	C - Compact	MD - Medium Dense	M - Moist	DRILLER D.R. PHILLIPS	TRACED S.L.T.
G - " (SG)	St. - Stiff	VC - Very Compact	D - Dense	W - Wet	START 25 Feb '70	CHECKED R.H.
Saturated Tube	V. St. - Very Stiff	VD - Very Dense	VD - Very Dense	S - Saturated	FINISH 26 Feb '70	
A Shoe - SAT	H. - Hard		LI - Liquid Limit	LI - Liquid Limit		
Standard Penetration Test			PL - Plastic Limit	PL - Plastic Limit		
	* These values refer to clay soils only and provide an indication of their consistency.				SHEET 2. of 4.	DRG No. 87744 - 1

GEOLOGICAL NOTES AND CLASSIFICATION	R.L. (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	CEMENT MOISTURE CONTENT	Consistency Combed Density	FIELD TEST DATA			
								BLOWS PER FOOT		SOIL TEST	
								20	40	60	80
								P.T. RATER Units			
								1	2	3	4
UPPER PLIOCENE HALLET COVE SANDSTONE											
				END OF DIAMOND DRILL HOLE							
Calcareous, glauconitic, abundant shelly fossil fragments and impressions. Weathering has altered most of the glauconite to limonite, bleached.	80										
	85		MH	SILT SOIL, high plasticity, sandy, clayey. Grey with brownish mottling. Weakly cemented bands and patches as shown. Sand grains form up to 30%							
	90			Mainly dark grey.							
Sand consists of quartz, glauconite and shell fragments. Minor mica flakes. Calcareous.	95		SM (SC)	SAND, excess fines (20%) light grey. Sand is fine to medium grained.							
	100										
	105		SP	SAND, poorly graded, few fines, fine to medium grained.							

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
					DRILL No.	LOGGED BY
A shoe (SA)	VS. — Very Soft	LS — Loose	VL — Very Loose	H — Humid	1	J.P.T.
D " (SD)	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	TYPE RUSTON	DATE 8 Apr. '70
E " (SE)	F — Firm		MD — Medium Dense	M — Moist	DRILLER D.R. PHILLIPS	TRACED S.L.T.
G " (SG)	St. — Stiff	C — Compact	D — Dense	W — Wet	START 18 Mar. '70	CHECKED R.H.
Sealed Tube — A Shoe — SAL	V. St. — Very Stiff	VC — Very Compact	VD — Very Dense	S — Saturated	FINISH 21 Mar. '70	
Standard Penetration Test	H. — Hard			LL — Liquid Limit		
	* These values refer to clay soils only and provide an indication of their consistency.			PL — Plastic Limit	SHEET 3 OF 4	DRG No. S7744-b HaG

PROJECT GOVERNMENT OFFICES
BUILDING

DEPARTMENT OF MINES - SOUTH AUSTRALIA
LOG OF CABLE TOOL HOLE

TOWN ACRE 335

HOLE	CH 1
SERIAL No.	
R.L. Surface	FT.
R.L. Collar	FT.
Datum	

FEATURE FOUNDATION INVESTIGATION
LOCATION VICTORIA SQUARE WEST

HUNDRED ADELAIDE

CO-ORDS

GEOLOGICAL NOTES AND CLASSIFICATION	R.L. (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	MOISTURE CONTENT	Consistency Compact, Dense	FIELD TEST DATA			
								BLOWS PER FOOT		SOIL TEST UNIT	
								20	40	60	80
UPPER EOCENE BLANCHE POINT MARLS. Slightly calcareous. Micaceous (?)	110		SP	as above		S	MD				
	115		MH	SILT SOIL, high plasticity, some fine grained sand. Dark grey/ black.							
END OF HOLE 119.5 FT.											

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS. — Very Soft	LS — Loose	VL — Very Loose	H — Humid	DRILL No. 1	LOGGED BY
D " (SD)	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	TYPE RUSTON	JPT
E " (SE)	F — Firm	C — Compact	MD — Medium Dense	M — Moist	DRILLER D.R. PHILLIPS	DATE 8 Apr. '70
G " (SG)	St. — Stiff	V.C. — Very Compact	D — Dense	W — Wet	START 18 Mar. '70	TRACED S.L.T.
Sealed Tube - A Shoe - SAL	V. St. — Very Stiff	H. — Hard	VD — Very Dense	S — Saturated	FINISH 21 Mar. '70	CHECKED R.H.
Standard Penetration Test - SPT	* These values refer to clay soils only and provide an indication of their consistency.			LL — Liquid Limit	SHEET 4 OF 4	DRG No. S7744-c
				PL — Plastic Limit		Ha6

PROJECT. BUILDING.

LOG OF DIAMOND DRILL HOLE

FEATURE. FOUNDATION. INVESTIGATION

TOWN ACRE 335 HUNDRED ADELAIDE.

CO-ORDINATES

LOCATION VICTORIA SQUARE WEST

ANGLE FROM HORIZ. 90°

DIRECTION

DESCRIPTION OF CORE

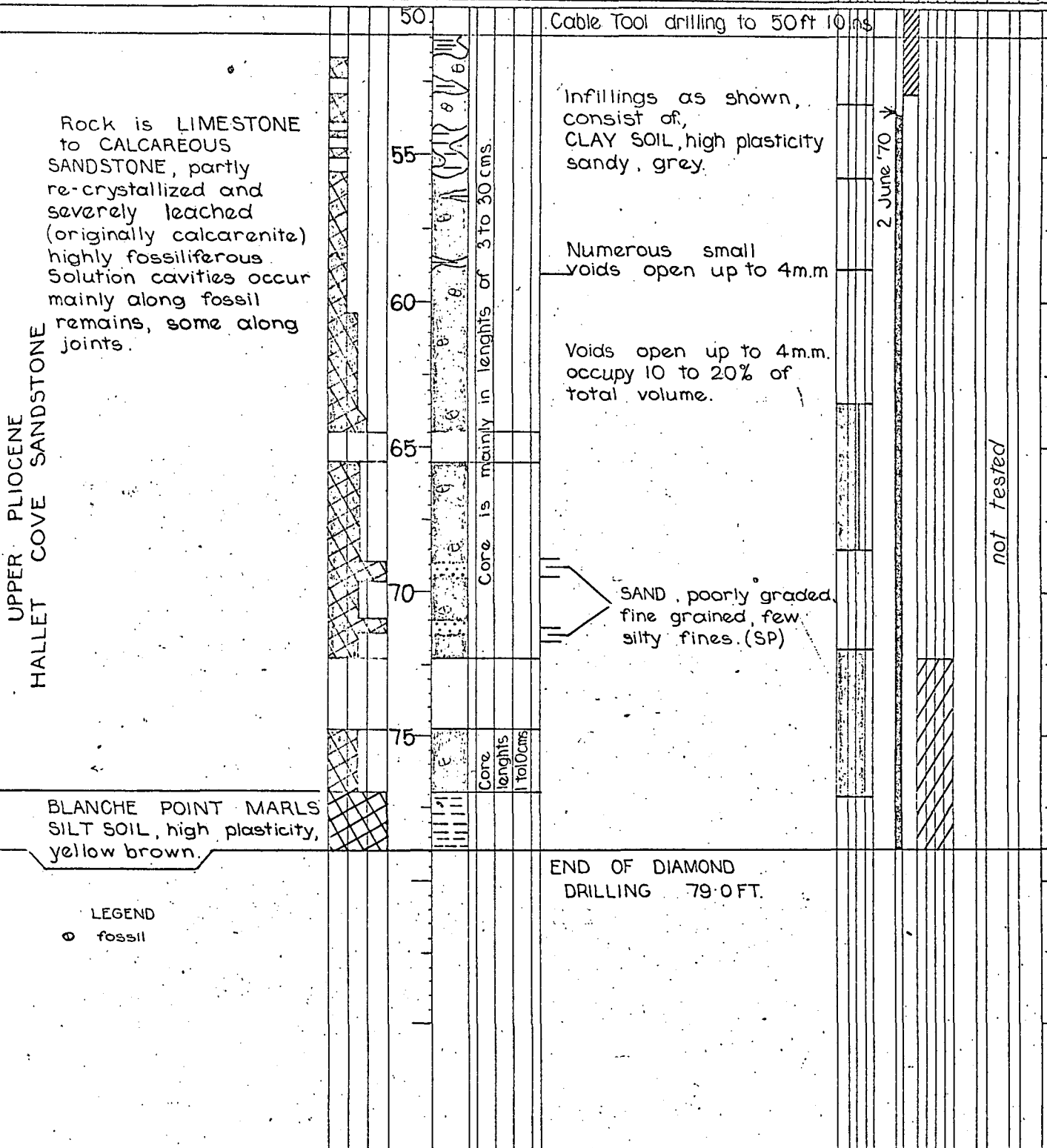
③
GROUP
SYMBOL①
STRENGTH
TERMCORE
SIZE
DEPTH
L
R.F.T.③
FRACTURE
LOG



STRUCTURES

LIFT
CORE
LOSS %WATER
LEVELCASING
LOSS
LOSS %WATER
PRESSURE
TESTS
LUGGONS

④

DATE



ROCK SUBSTANCE		FRACTURE LOG		ENGINEERING GEOLOGY SECTION	
① STRENGTH TERM VS-Very Strong S-Strong MS-Medium Strong W-Weak VW-Very Weak SO-Soil properties	CONDITION TERM  Fresh Decomposed Weathered Altered Not Applicable	③	 Natural fractures per foot of core Equivalent length of core in inches	LOGGED J.P.T.	
		④	(3.5) Maximum effective pressure (bars) reached during test. Min. = Minimum value.	DATE 8 Apr. '70 TRACED SLT CHECKED R.H.	
② Substances with soil properties remoulded and classified by Unified System		SHEET 1 OF 1		DRG. No. S7744-d HAG	

GEOLOGICAL NOTES AND CLASSIFICATION				SOIL DESCRIPTION		FIELD TEST DATA											
R.L. (FEET) DEPTH				GROUP NAME		BLOWS PER FOOT		SOIL TEST									
GRAPHIC LOG				Unified Soil Classification, U.S.B.R.		20 40 60 80		Units									
GROUP SYMBOL				Earth Manual 2nd Edition 1966				1 2 3 4									
Silt patches are calcareous				CH		CLAY SOIL, high plasticity, sandy. Grey with red-brown and some yellow-brown mottling. Some patches of white silt as shown. Sand content decreases from approx. 20% at top to less than 5% at 25 ft.		Firm		N=7							
										N=9							
										N=12							
Numerous rootlets less than 0.5 m.m. diam.				CH				Stiff									
Rootlet, partly decomposed, 1m.m. diam.				CH				Very Stiff									
Dyke of clayey sand, 2m.m. wide.				SC to SP		SAND, poorly graded, 10 to 25% clayey fines. Sand is mainly medium grained. Pale grey with yellow-brown and red-brown patches.		Humid Dence									

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silts)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
					DRILL No. 1 TYPE RUSTON DRILLER D.R. PHILLIPS START 20 Feb. '70 FINISH 24 Feb. '70 SHEET 1. OF 3	LOGGED BY J.P.T. DATE 26 Feb. '70 TRACED S.L.T. CHECKED R.H. DRG No. S7745 Ha6
A shoe (SA) D. " (SD) E. " (SE) G. " (SG) Sealed Tube - A Shoe - SAL Standard Penetration Test - SPT	VS. — Very Soft S — Soft F — Firm St. — Stiff V. St. — Very Stiff H. — Hard	LS — Loose MC — Moderately Compact C — Compact VC — Very Compact	VL — Very Loose L — Loose MD — Medium Dense D — Dense VD — Very Dense	H — Humid D — Damp M — Moist W — Wet S — Saturated LL — Liquid Limit PL — Plastic Limit		

PROJECT GOVERNMENT OFFICE LOG OF CABLE TOOL HOLE

HOLE CH 2

SERIAL No.

BUILDING

TOWN ACRE 335

FEATURE FOUNDATIONS

HUNDRED. ADELAIDE

LOCATION. VICTORIA SQUARE WEST

CO-ORDS

R.L. Surface, FT.

R.L. Collar, FT.

Datum.

GEOLOGICAL NOTES
AND CLASSIFICATIONR.L. (FEET)
DEPTHGRAPHIC
LOGGROUP
SYMBOL

SOIL DESCRIPTION

GROUP NAME

Unified Soil Classification, U.S.B.R.
Earth Manual 2nd Edition 1966WATER
LEVELCEMENT
MOISTURE
CONTENTConsistency
Compact; Durable

FIELD TEST DATA

BLOWS
PER FOOT
20 40 60 80SOIL TEST
P.T. METER
Units: %
1 2 3 4PLEISTOCENE
HINDMARSH CLAY FORMATION

Slickensided joint.

40

CH

Silty patches
are calcareous.

45

Silty patches, weakly to
moderately cemented in
part, white.

50

55

60

65

70

Hole continued by
diamond drill to 79.2 ft.
For detailed log see
dwg. no. S7745-C

2 June '70

V

UPPER PLIOCENE
HALLET COVE SANDSTONE

TYPE OF SAMPLE

A shoe (SA)
D " (SD)
E " (SE)
G " (SG)
Sealed Tube -
A Shoe -SAL
Standard Pen-
etration Test-SPTWater level, (date)
Water cut
Casing
Dec '66CONSISTENCY
(Clays)VS. — Very Soft
S — Soft
F — Firm
St. — Stiff
V. St. — Very Stiff
H. — HardCOMPACTNESS
(Silt)Ls — Loose
MC — Moderately
Compact
C — Compact
VC — Very
CompactRELATIVE
DENSITY (Sands)VL — Very Loose
L — Loose
MD — Medium
Dense
D — Dense
VD — Very DenseMOISTURE
CONTENTH — Humid
D — Damp
M — Moist
W — Wet
S — Saturated
LL — Liquid Limit
PL — Plastic Limit

ENGINEERING GEOLOGY SECTION

DRILL No. J
TYPE RUSTON
DRILLER D.R. PHILLIPS
START 20 Feb '70
FINISH 24 Feb '70LOGGED BY
J.P.T.
DATE 23 Feb '70
TRACED S.L.T.
CHECKED R.H.

SHEET 2 OF 3

DRG No. S7745-a
Hag* These values refer to clay soils only and
provide an indication of their consistency.

PROJECT GOVERNMENT OFFICE LOG OF CABLE TOOL HOLE

HOLE CH 2

SERIAL No.

BUILDING

TOWN ACRE 335

FEATURE FOUNDATIONS

HUNDRED, ADELAIDE

R.L. Surface FT.

LOCATION VICTORIA SQUARE WEST

CO-ORDS

R.L. Collar FT.

Datum

GEOLOGICAL NOTES
AND CLASSIFICATIONR.L. (FEET)
DEPTHGRAPHIC
LOGGROUP
SYMBOLSOIL DESCRIPTION
GROUP NAMEUnified Soil Classification, U.S.B.R.
Earth Manual 2nd Edition 1966WATER
LEVELMOISTURE
CONTENT

Consistency

Compaction

FIELD TEST DATA

BLOWS
PER FOOTSOILTEST
PRIMETER
Units *

20 40 60 80

1 2 3 4

UPPER PLIOCENE
HALLET COVE
SANDSTONE

75

END OF DIAMOND DRILL HOLE

79.2 FT

UPPER EOCENE
BLANCHE POINT
MARLSWeathered zone.
LeachedSand consists of
quartz, glauconite
and minor mica.
Fines are highly
calcareous.
Numerous fossils.

80

85

90

95

100

MH

SILT SOIL, high plasticity,
Sandy. Pale grey-green.
Contains strongly
cemented bands as
shown.

Mainly dark grey

SM-MH

SAND, excess silt.
Mainly fine grained.
Grey to yellow brown.

MC > PL

MC >> PL

Moderately compact

MD

TYPE OF SAMPLE

A shoe (SA)
D " (SD)
E " (SE)
G " (SG)
Sealed Tube -
A Shoe - SAL
Standard Pen-
etration, Test-SPTWater level,
(date) 7 Dec '66
Water cut
CasingCONSISTENCY
(Clays)VS. — Very Soft
S — Soft
F — Firm
St. — Stiff
V. St. — Very Stiff
H. — HardCOMPACTNESS
(Silt)Ls. — Loose
MC — Moderately
Compact
C — Compact
VC — Very
CompactRELATIVE
DENSITY (Sands)VL — Very Loose
L — Loose
MD — Medium
Dense
D — Dense
VD — Very DenseMOISTURE
CONTENTH — Humid
D — Damp
M — Moist
W — Wet
S — Saturated
LL — Liquid Limit
PL — Plastic Limit

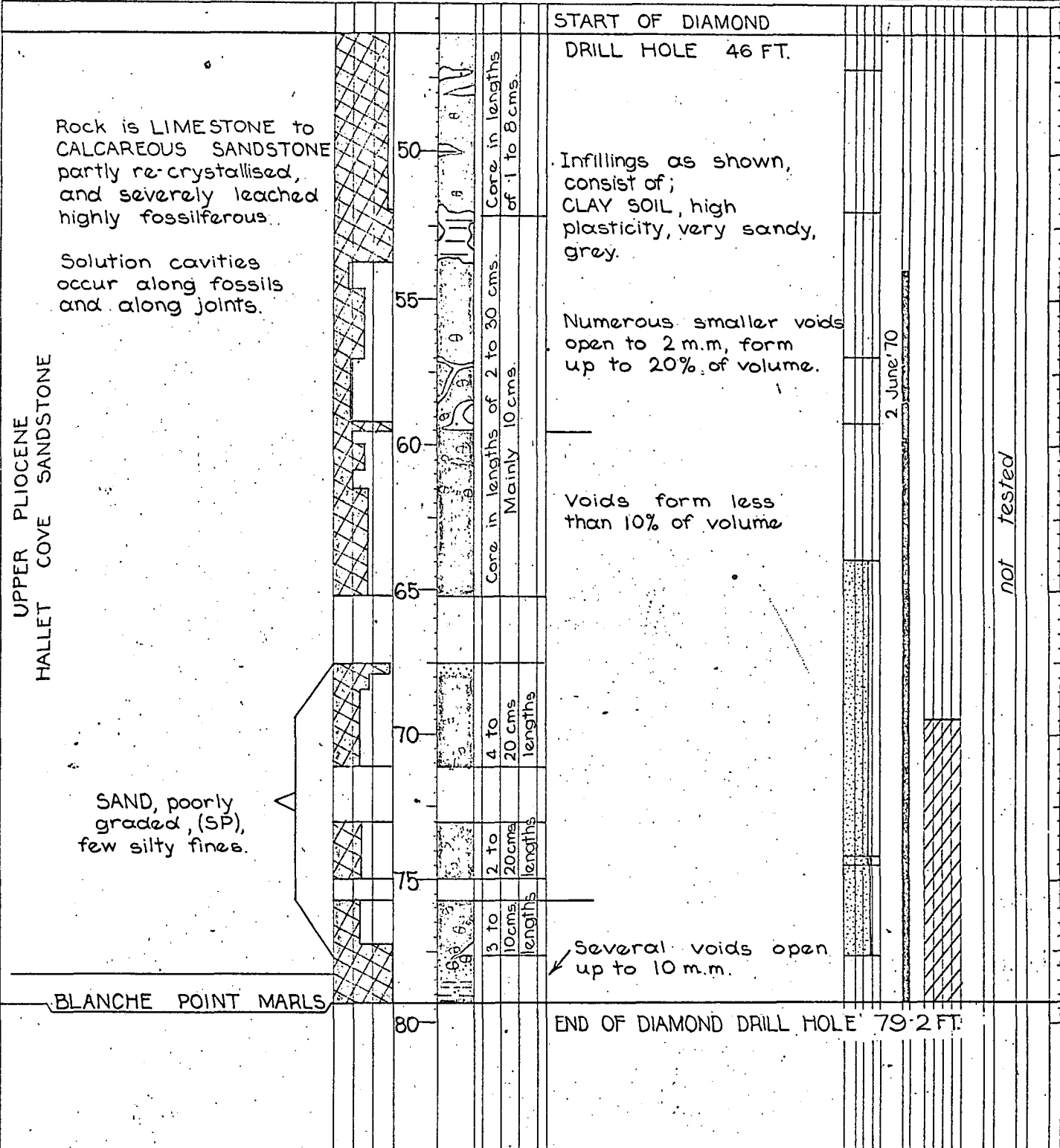
ENGINEERING GEOLOGY SECTION

DRILL No. 1
TYPE RUSTON
DRILLER D.R. PHILLIPS
START 13 Mar '70
FINISH 13 Mar '70LOGGED BY
JPT
DATE 26 Feb '70
TRACED S.L.T.
CHECKED R.H.

SHEET 3 OF 3

DRG No. 7745-b
Rag* These values refer to clay soils only and
provide an indication of their consistency.

DESCRIPTION OF CORE	GROUP SYMBOL	STRENGTH TERM	CORE SIZE DEPTH	LOG	FRACTURE LOG	STRUCTURES	LIFT CORE LOSS %	WATER LEVEL	CASING	DRILL WATER LOSS %	WATER PRESSURE TESTS LUGFONS
	VS	MS	WN	WV	WV		10	5 50	DATE	0 100	0 5 5 10 50



① ROCK SUBSTANCE STRENGTH TERM VS-Very Strong S-Strong MS-Medium Strong W-Weak VW-Very Weak SO-Soil properties CONDITION TERM Fresh Decomposed Weathered Altered Not Applicable	③ FRACTURE LOG 1 4 16 64 Natural fractures per foot of core 12 3 3 3 Equivalent length of core in inches 4 16 ④ (3.5) Maximum effective pressure (bars) reached during test. Min. = Minimum value.	ENGINEERING GEOLOGY SECTION DRILL No. 2 TYPE MINDRILL E1000 DRILLERASHMONEIT START 5 Mar. '70 FINISH 7 Mar. '70 LOGGED J.P.T. DATE 26 Feb. '70 TRACED S.L.T. CHECKED R.H. SHEET 1. OF 1. DRG. No. S7745-C HOG
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PROJECT . GOVERNMENT . OFFICE

HOLE CH 3

LOG OF CABLE TOOL HOLE

SERIAL No.

R.L. Surface.

FT.

R.L. Collar

FT.

Datum.

BUILDING

TOWN ACRE . 335

FEATURE . FOUNDATIONS.

HUNDRED . ADELAIDE

LOCATION . VICTORIA SQUARE WEST . S.E. corner

CO-ORDS

GEOLOGICAL NOTES AND CLASSIFICATION	R.L. (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL CEILING	MOISTURE CONTENT	Consistency	Correct Density	FIELD TEST DATA				
									BLOWS PER FOOT 20 40 60 80			SOILTEST P.T.METER Units # 1 2 3 4	
PLEISTOCENE HINDMARSH CLAY					Logged from prices from sealed tube shoes.					F	N=7		
	SAND is quartz SILT is lime	5	CL	CLAY SOIL, low plasticity, sandy, silty (lime), grey-brown contains patches of silt and cemented nodules.									
	decreasing lime content.	10	CH	CLAY SOIL, high plasticity, grey to red-brown mottled Bright sheens on structural units.									
	little or no calcareous fines.	20	CH										
	iron stained in part minor local lime patches	30	SP- SC	SAND, poorly graded, with clayey fines to SAND excess clayey fines, grey to red brown									
	35			As from 7 to 29 ft						St			

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS. — Very Soft	LS — Loose	VL — Very Loose	H — Humid	DRILL No. 1	LOGGED BY W. Boucaut
D " (SD)	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	TYPE RUSTON	DATE 21 Apr '70
E " (SE)	F — Firm	C — Compact	MD — Medium Dense	M — Moist	DRILLER D. PHILLIPS	TRACED SLT
G " (SG)	St. — Stiff	V.C — Very Compact	D — Dense	W — Wet	START 17 Feb '70	CHECKED R.H.
Sealed Tube - A Shoe - SAL	V.St. — Very Stiff	VD — Very Dense	LL — Liquid Limit	S — Saturated	FINISH 19 Feb '70	
Standard Penetration Test - SPT	H. — Hard		PL — Plastic Limit		SHEET 1 OF 3	DRG. No. S7746
	* These values refer to clay soils only and provide an indication of their consistency.					Ha6

PROJECT GOVERNMENT OFFICE
BUILDING
FEATURE FOUNDATIONS
LOCATION VICTORIA SQUARE WEST

DEPARTMENT OF MINES SOUTH AUSTRALIA
LOG OF CABLE TOOL HOLE

TOWN ACRE 335
HUNDRED ADELAIDE
CO-ORDS

HOLE CH 3
SERIAL No.
R.L. Surface FT.
R.L. Collar FT.
Datum

GEOLOGICAL NOTES AND CLASSIFICATION	R.L. (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL Casing	MOISTURE CONTENT	Consistency Compact/Dens.	FIELD TEST DATA	
								BLOWS PER FOOT 20 40 60 80	SOILTEST PARAMETER Units % 1 2 3 4
PLEISTOCENE HINDMARSH CLAY	40		CH	CLAY SOIL, high plasticity, grey to red brown mottled					
UPPER PLOICENE HALET COVE SANDSTONE ← EROSION SURFACE	45								
Silt is lime. Gravel/Sand is sandstone fragments.	50		ML GM	SILT SOIL, low plasticity, white, containing up to 50% SAND and GRAVEL fragments to 30 m.m. size					
	55			SILT SOIL, grading to ROCK (Sandstone) weak, increasing sand percentage					
	60			CONTINUED BY DIAMOND DRILL SEE DWG. NO. S7746-c					
	65								
	70								

TYPE OF SAMPLE	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sands)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
					DRILL No. 1 TYPE: RUSTON DRILLER D. PHILLIPS START 17 Feb '70 FINISH 19 Feb '70	LOGGED BY W. Boucaut DATE 21 Apr '70 TRACED SLT CHECKED R.H.
A shoe (SA) D " (SD) E " (SE) G " (SG) Sealed Tube - A Shoe -SAL Standard Pen- etration Test-SPT	VS. — Very Soft S — Soft F — Firm St. — Stiff V. St. — Very Stiff H. — Hard	LS — Loose MC — Moderately Compact C — Compact VC — Very Compact	VL — Very Loose L — Loose MD — Medium Dense D — Dense LL — Liquid Limit VD — Very Dense	H — Humid D — Damp M — Moist W — Wet S — Saturated LL — Liquid Limit PL — Plastic Limit	SHEET 2 OF 3	DRG No. S7746-a Hag

GEOLOGICAL NOTES AND CLASSIFICATION	R.L. (FEET) DEPTH	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL CEILING	MOISTURE CONTENT	Consistency	Compact. Density	FIELD TEST DATA			
									BLOWS PER FOOT	SOILTEST P'TR METER Units		
									20 40 60 80	1 2 3 4		
HALLET COVE SANDSTONE	75			54 TO 78FT DIAMOND DRILLING. SEE DWG. NO. S7746-c								
SAND, is quartz, glauconitic. Partly calcareous, shelly fossil fragment, to 20mm. size are common.	80		MH	SILT SOIL, high plasticity, estimated up to 30% SAND, medium grained. Grey, green, yellow mottled								
	85			Gradational to dark grey								
				Dark grey micaceous								
mica in sand sizes, (fine grained)	90			occasional red brown to grey, more sandy, pockets to 30mm. size and lenses to 5 mm wide.								
	95											
EOCENE BLANCHE POINT MARLS	100			END OF HOLE 98 FT.								

TYPE OF SAMPLE

A shoe (SA)

D. " (SD)

E " (SE)

G " (SG)

Sealed Tube - A Shoe -SAL

Standard Penetration Test-SPT

Water level, (date)

7 Dec '66

WC

Water cut

Consistency (Clays)

VS. — Very Soft

S — Soft

F — Firm

St. — Stiff

V. St. — Very Stiff

H. — Hard

Compactness (Sills)

Ls — Loose

MC — Moderately Compact

C — Compact

VC — Very Compact

Relative Density (Sands)

VL — Very Loose

L — Loose

MD — Medium Dense

D — Dense

VD — Very Dense

Moisture Content

H — Humid

D — Damp

M — Moist

W — Wet

S — Saturated

LL — Liquid Limit

PL — Plastic Limit

ENGINEERING GEOLOGY SECTION

DRILL No. 1

TYPE RUSTON

DRILLER D. PHILLIPS

START 10 Mar. '70

FINISH 11 Mar. '70

LOGGED BY W. Boucaut

DATE 21 Apr. '70

TRACED S.L.T.

CHECKED R.H.

SHEET 3 of 3

DRG No. S7746-b

Ha6

LOG OF DIAMOND DRILL HOLE

HOLE NO. CH 3

SERIAL No.

FEATURE. FOUNDATIONS

TOWN ACRE 335 HUNDRED ADELAIDE.

R. L. Surface . FT.

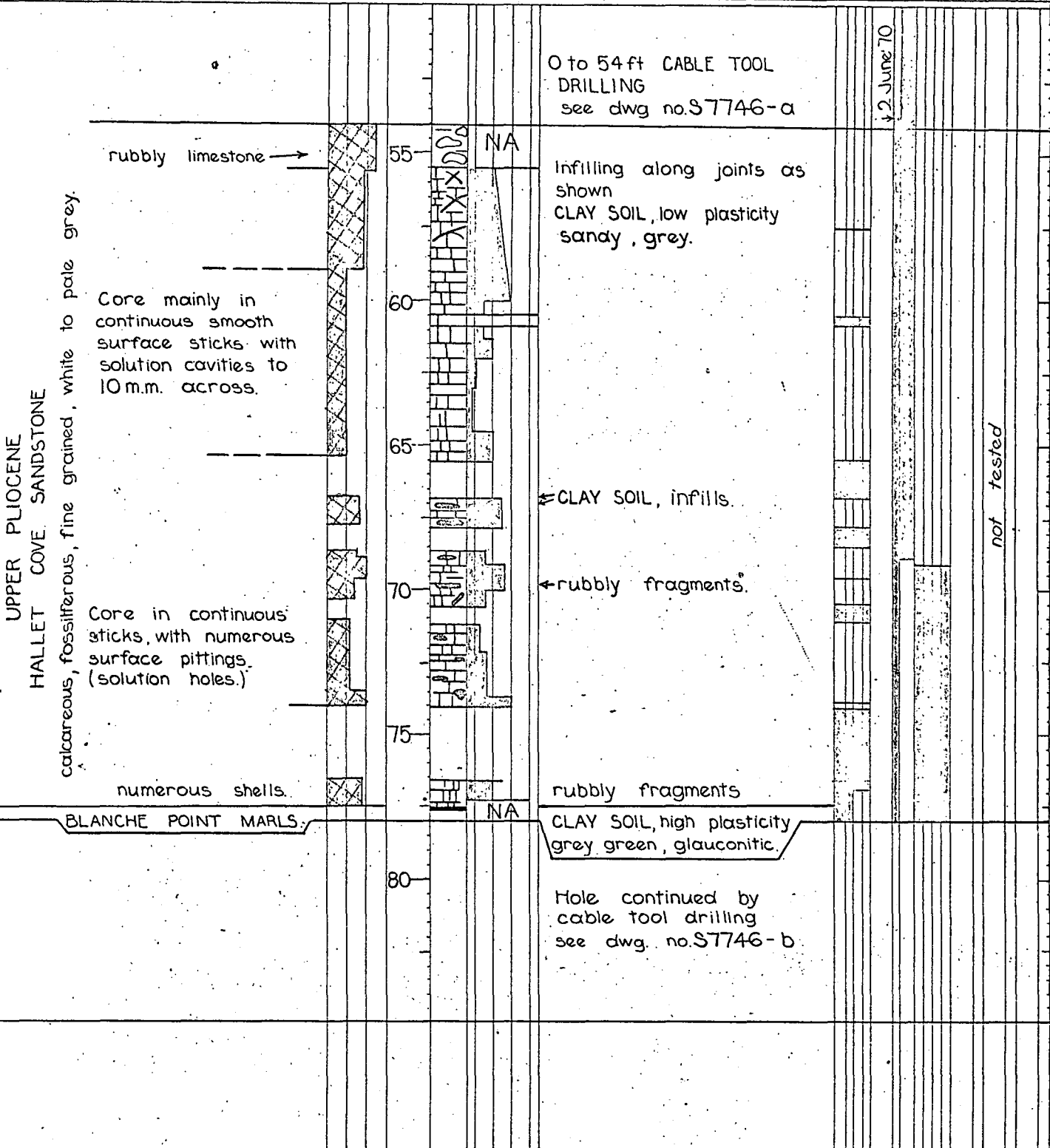
R. L. Collar . FT.

LOCATION VICTORIA SQUARE WEST.

CO-ORDINATES
ANGLE FROM HORIZ. 90° DIRECTION

Datum

DESCRIPTION OF CORE	② GROUP SYMBOL	① STRENGTH TERM	CORE SIZE DEPTH	LOG	③ FRACTURE LOG	STRUCTURES	LIFT CORE LOSS %	WATER LEVEL	CASING	DRILL WATER LOSS %	WATER PRESSURE TESTS LUGEONS	④
		VS MS S W VW SO	1 4 16 64		1 4 16 64		10 50	DATE	0 100	0.5 5 10 50		



① ROCK SUBSTANCE	③ FRACTURE LOG	ENGINEERING GEOLOGY SECTION
<p>VS-Very Strong S-Strong MS-Medium Strong W-Weak VW-Very Weak SO-Soil properties</p> <p>CONDITION TERM</p> <p>Fresh Decomposed Weathered Altered Not</p> <p>Applicable</p>	<p>1 4 16 64 Natural fractures per foot of core</p> <p>12 3 3 3 Equivalent length of core in inches</p> <p>4 16</p> <p>④ (3-5) Maximum effective pressure (bars) reached during test.</p> <p>Min. = Minimum value.</p>	<p>DRILL No. 24</p> <p>TYPE E.100</p> <p>DRILLER ASHMOREIT.</p> <p>START 21 Feb '70</p> <p>FINISH 22 Feb '70</p> <p>LOGGED W. BOUCAUT</p> <p>DATE 2 Apr '70</p> <p>TRACED S.L.T.</p> <p>CHECKED R.H.</p> <p>SHEET. 1 OF 1</p> <p>DRG. No. S7746-C</p>

PROJECT GOVERNMENT OFFICE

LOG OF CABLE TOOL HOLE

SERIAL No.

BUILDING

TOWN ACRE 335

R.L. Surface.

FT.

FEATURE FOUNDATIONS

HUNDRED. ADELAIDE

R.L. Collar

FT.

LOCATION VICTORIA SQUARE WEST

CO-ORDS

Datum

GEOLOGICAL NOTES
AND CLASSIFICATIONR.L. (FEET)
DEPTHGRAPHIC
LOGGROUP
SYMBOLSOIL DESCRIPTION
GROUP NAMEUnified Soil Classification, U.S.B.R.
Earth Manual 2nd Edition 1966WATER
LEVELMOISTURE
CONTENTConsistency
Compact, Dense

FIELD TEST DATA

BLOWS
PER FOOT
20 40 60 80SOILTEST
P.T.R. METER
Units *
1 2 3 4

Silt is calcareous

Numerous patches
of MnO₂PLEISTOCENE
HINDMARSH CLAY FORMATIONNumerous rootlets
along incipient
shrinkage cracks

Slickensided joint

SC-
ML SAND, excess clayey fines
(up to 50%), brown, with
patches of sandy SILT,
offwhite.CH CLAY SOIL, high plasticity,
silty. Light brown. Sand
dykes (SC) up to 3m.m. wideCH CLAY SOIL, high plasticity,
few sand grains (less
than 15%), light grey with
red-brown and yellow-
brown mottling.SC SAND, excess clayey fines
mainly pale grey, some
brown patches (weakly
cemented). SAND grains are
mainly medium grained.
Fines form up to 30%

CH CLAY SOIL, high plasticity

MC > PL
FirmMC > PL
Stiff

Very Stiff

Humid
Dense

V St

N = 6

N = 9

N = 14

TYPE OF SAMPLE

A shoe (SA)
D " (SD)
E " (SE)
G " (SG)
Sealed Tube -
A Shoe -SAL
Standard Pen-
etration Test-SPTWater level
(date)
7 Dec '66
WC
Water cutCONSISTENCY
(Clays)
VS. — Very Soft
S — Soft
F — Firm
St. — Stiff
V. St. — Very Stiff
H. — HardCOMPACTNESS
(Sills)
Ls — Loose
MC — Moderately
Compact
C — Compact
VC — Very
CompactRELATIVE
DENSITY (Sands)
VL — Very Loose
L — Loose
MD — Medium
Dense
D — Dense
VD — Very DenseMOISTURE
CONTENT
H — Humid
D — Damp
M — Moist
W — Wet
S — Saturated
LL — Liquid Limit
PL — Plastic Limit

ENGINEERING GEOLOGY SECTION

DRILL No. 1
TYPERUSTON BUCYRUS
DRILLER D. PHILLIPS
START 12 Feb '70
FINISH 16 Feb '70LOGGED BY
J.P.T.
DATE 30 Apr '70
TRACED S.L.T.
CHECKED R.H.

SHEET 1 OF 3

DRG No. S7747 Ha6

PROJECT GOVERNMENT OFFICE

BUILDING

FEATURE FOUNDATIONS

LOCATION VICTORIA SQUARE WEST

DEPARTMENT OF MINES SOUTH AUSTRALIA
LOG OF CABLE TOOL HOLE

TOWN ACRE 335

HUNDRED ADELAIDE

CO-ORDS

HOLE CH 4

SERIAL No.

R.L. Surface

FT.

R.L. Collar

FT.

Datum

GEOLOGICAL NOTES
AND CLASSIFICATIONR.L.
(FEET)
DEPTHGRAPHIC
LOGGROUP
SYMBOL

SOIL DESCRIPTION

GROUP NAME

Unified Soil Classification, U.S.B.R.
Earth Manual 2nd Edition 1966WATER
LEVELMOISTURE
CONTENT

Consistency

Compact Density

FIELD TEST DATA

BLOWS
PER FOOTSOILTEST
P.T. METER
UnitsPLEISTOCENE
HINDMARSH CLAY
FORMATION

Slickensided joint

Silt is highly
calcareousUPPER PLIOCENE
HALLET COVE SANDSTONECLAY SOIL, high plasticity,
few sand grains. Light
grey with yellow-brown
mottling.

CH

SAND, excess silty fines (up
to 30%), weakly cemented,
pale grey to white. Contains
numerous dykes of sandy
clay (CH+SC) up to 2cms. wide.

SM

Moderately cemented

HOLE CONTINUED BY
DIAMOND DRILL TO 76.7 FT.
for detailed log see
dwg. no.MC > PL
Very Stiff

Dense

2 June 70

TYPE OF SAMPLE

A shoe (SA)
D " (SD)
E " (SE)
G " (SG)
Sealed Tube
A Shoe -SAL
Standard Penetration Test-SPTWater level, (date)
7 Dec '66
WCM
Water cut

CONSISTENCY

VS. — Very Soft
S — Soft
F — Firm
St. — Stiff
V. St. — Very Stiff
H. — Hard

COMPACTNESS

Ls — Loose
MC — Moderately Compact
C — Compact
VC — Very Compact

RELATIVE DENSITY (Sands)

VL — Very Loose
L — Loose
MD — Medium Dense
D — Dense
VD — Very Dense

MOISTURE CONTENT

H — Humid
D — Damp
M — Moist
W — Wet
S — Saturated
LL — Liquid Limit
PL — Plastic Limit

ENGINEERING GEOLOGY SECTION

DRILL No. 1
TYPE RUSTON BUCYRUS
DRILLER D. PHILLIPS
START 12 Feb. 70
FINISH 16 Feb. 70
LOGGED BY J.P.T.
DATE 30 Apr. 70
TRACED S.L.T.
CHECKED R.H.
SHEET 2 OF 3
DRG No. S7747-a
Ha6* These values refer to clay soils only and
provide an indication of their consistency.

PF N° S6676 MB

BUILDING

TOWN ACRE 335

R.L. Surface

ET

R.L. Collar

ET

Datum


FOUNDATIONS

HUNDRED. ADELAIDE

LOCATION. VICTORIA. SQUARE. WEST

CO-ORDS

[illegible]

TYPE OF SAMPLE		CONSISTENCY (Clays)		COMPACTNESS (Silts)		RELATIVE DENSITY (Sands)		MOISTURE CONTENT		ENGINEERING GEOLOGY SECTION	
A shoe (SA)	 Water level, (date) → 7 Dec. '66 WCM Water cut	VS. — Very Soft	LS — Loose	VL — Very Loose	H — Humid	DRILL No. 1 TYPE RUSTIN DRILLER: D. PHILLIPS START 12 Feb. '70 FINISH 3 Mar. '70		LOGGED BY JPT DATE 30 Apr. '70 TRACED S.L.T. CHECKED R.H.			
D " (SD)		S — Soft	MC — Moderately Compact	L — Loose	D — Damp						
E " (SE)		F — Firm	C — Compact	MD — Medium Dense	M — Moist						
G " (SG)		St. — Stiff	V.C — Very Compact	D — Dense	W — Wet						
Sealed Tube — A Shoe —SAL		V. St. — Very Stiff	VD — Very Dense	S — Saturated							
Standard Penetration Test-SPT		H. — Hard			LL — Liquid Limit	PL — Plastic Limit	SHEET 3 of 3	DRG No. S7747-b	H-66		
		* These values refer to clay soils only and provide an indication of their consistency.									

DESCRIPTION OF CORE	GROUP SYMBOL	STRENGTH TERM	CORE SIZE DEPTH L R. FT.	LOG	FRACTURE LOG	STRUCTURES	LIFT CORE LOSS% 10 5 50	WATER LEVEL DATE	CASING DEPTH WATER LOSS% 0 100	WATER PRESSURE TESTS LUGGONS 0.5 5 10 50
UPPER PLIOCENE HALLET COVE SANDSTONE. Rock is LIMESTONE to CALCAREOUS SANDSTONE, partly re-crystallised and severely leached highly fossiliferous Solution cavities occur along fossils and along joints. Uncemented sand		VS	55	1 4 16 64	Core lengths from 2 to 5 cms lengths 20 cms.	START OF DIAMOND DRILL HOLE . 51 FT.	7 Feb 70			
					Core in lengths of 5 to 20 cms.	Rock with infillings as shown of, CLAY SOIL, high plasticity, grey, sandy. Numerous small voids open to 3mm., form up to 20% of volume in places Voids form less than 10% of volume.				
					5 to 20 cms	Patch of SAND (SP), poorly graded, fine grained, few silty fines 15 % voids				
					2 to 15 cms lengths					
					up to 3 cms lengths					
			80			END OF DIAMOND HOLE 76.6 FT. HOLE CONTINUED BY CABLE TOOL DRILL				
			85							

① ROCK SUBSTANCE
STRENGTH TERM
VS-Very Strong
S-Strong
MS-Medium Strong
W-Weak
VW-Very Weak
SO-Soil properties

CONDITION TERM
 Fresh
 Decomposed
 Weathered
 Altered
 Not
Applicable.

③ FRACTURE LOG
1 4 16 64 Natural fractures per foot of core
 Equivalent length of core in inches
12 3 3 3 4 16

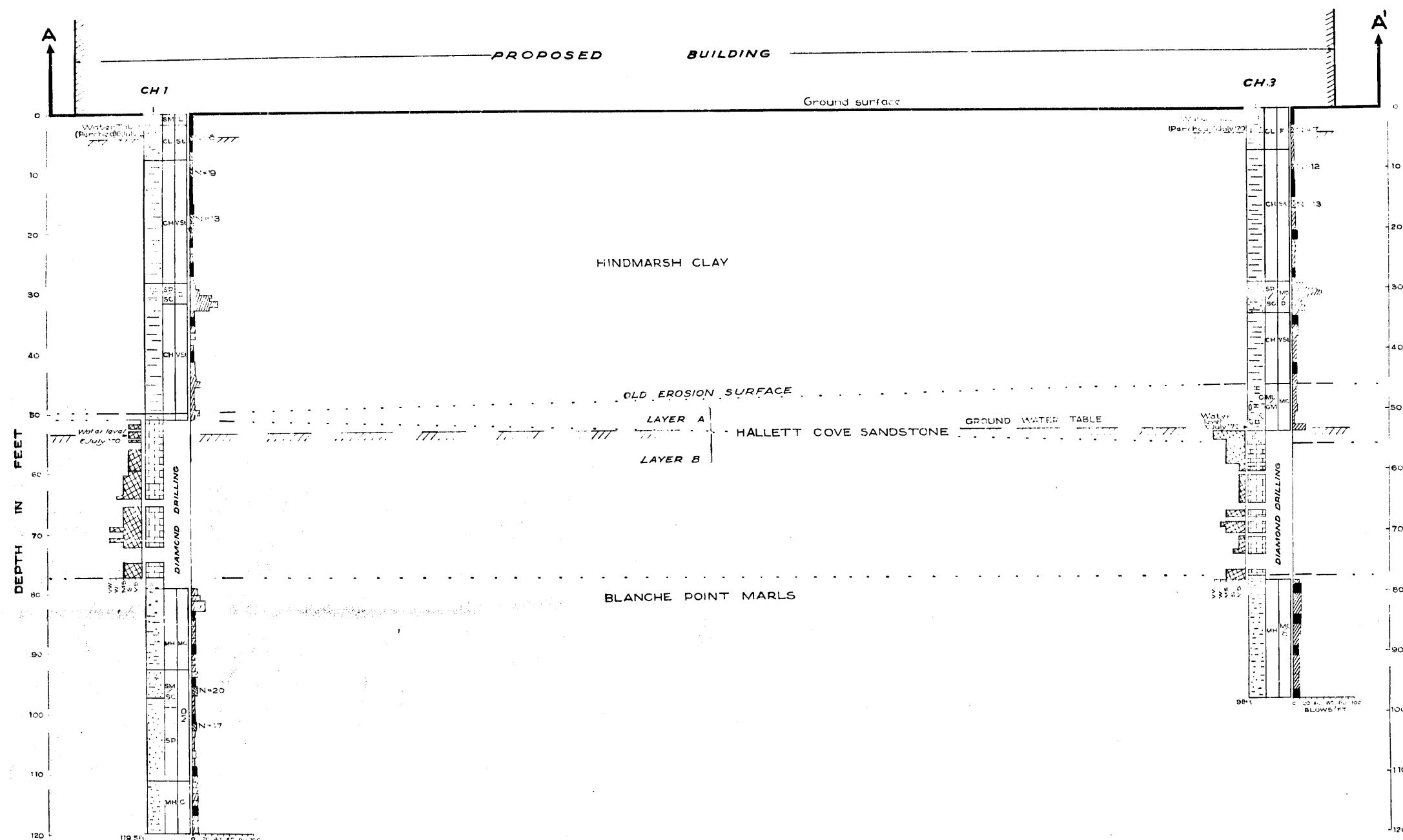
④ (3.5) Maximum effective pressure (bars) reached during test.
Min. = Minimum value.

ENGINEERING GEOLOGY SECTION
DRILL No. 24
TYPE MINDRILL E 1000
DRILLERASHMONEILT
START 18 Feb 70
FINISH 20 Feb 70

LOGGED J.P.T.
DATE 30 Apr 70
TRACED S.L.T.
CHECKED R.H.

SHEET 1 OF 1 DRG. No. S7747-C

DRILL LOGS



IN SOILS

GRAPHIC LOG

SW
CH
N-7 SPT Value

CLASSIFICATION

	Penetration rate	Type of Sample	Consistency (Clay)	Compactivity (Silt)	Relative Density (Sand)
Unified Soil Classification					
System Group					
Symbol					

IN ROCK

DIAMOND DRILLING

NO
GF
3-6 ft.

STRENGTH AND CONDITION

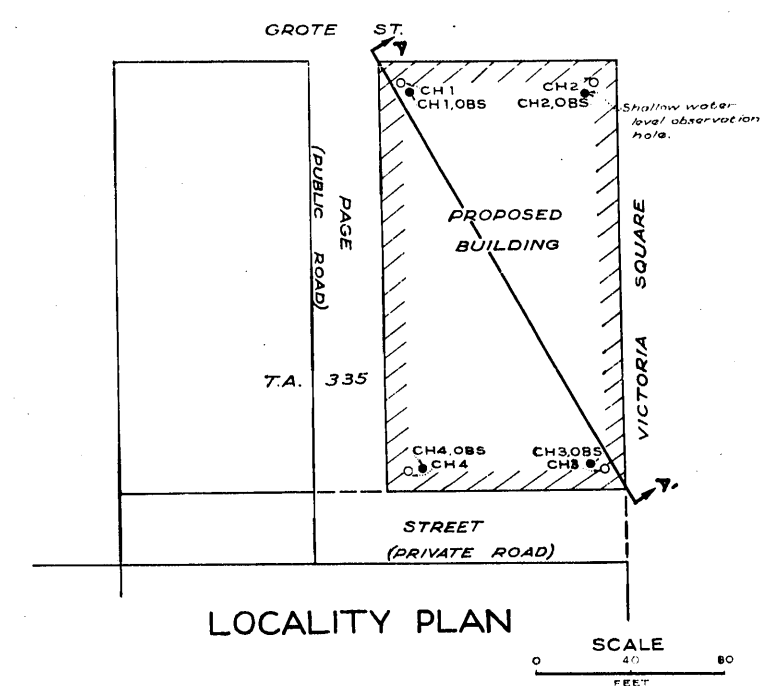
	Very weak	Weak	Medium strong	Strong	Very strong
VV					
W					
MS					
S					
VS					

LEGEND

▨ S Series A shoe (SA)

■ S Series A shoe, sealed for testing (SAL)

▩ Standard Penetration Test
N = N9 of blows/ft.



UNIFIED SOIL CLASSIFICATION (in part)		CONSISTENCY (CLAYS)	COMPACTNESS (SILTS)	RELATIVE DENSITY (SANDS)
	Soil Description	V Very soft	Ls Loose	VL Very loose
	CH CLAY SOIL, high plasticity	S Soft	MC Moderately compact	L Loose
	CL CLAY SOIL, low plasticity	F Firm	C Compact	MD Medium dense
	MH SILT SOIL, high plasticity	St Stiff	VC Very compact	D Dense
	SP SAND, poorly graded	VSt Very stiff		VD Very dense
	SC SAND, excess clayey fines	H Hard		
	SM SAND, excess silty fines			
	GM GRAVEL, excess silty fines, poorly graded gravel-sand-silt mixture			

SCALE IN FEET

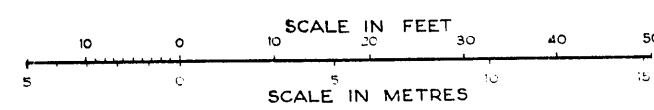


FIG. 1.

DEPARTMENT OF MINES — SOUTH AUSTRALIA			
GOVERNMENT OFFICE BUILDING			
T.A. 335 HQ ADELAIDE			
LOCALITY PLAN AND GEOLOGICAL SECTION			
ENGINEERING SECTION	GEOLOGY	Dra. W.F. R.R.	SCALE: As shown
	A. H. H. 100000 215970	Ed. G.H.	70-379 H06
Director of Mines: E.E.N. GREGORY		Ed. W.F. R.	
		Exd.	DATE: 3 June 70

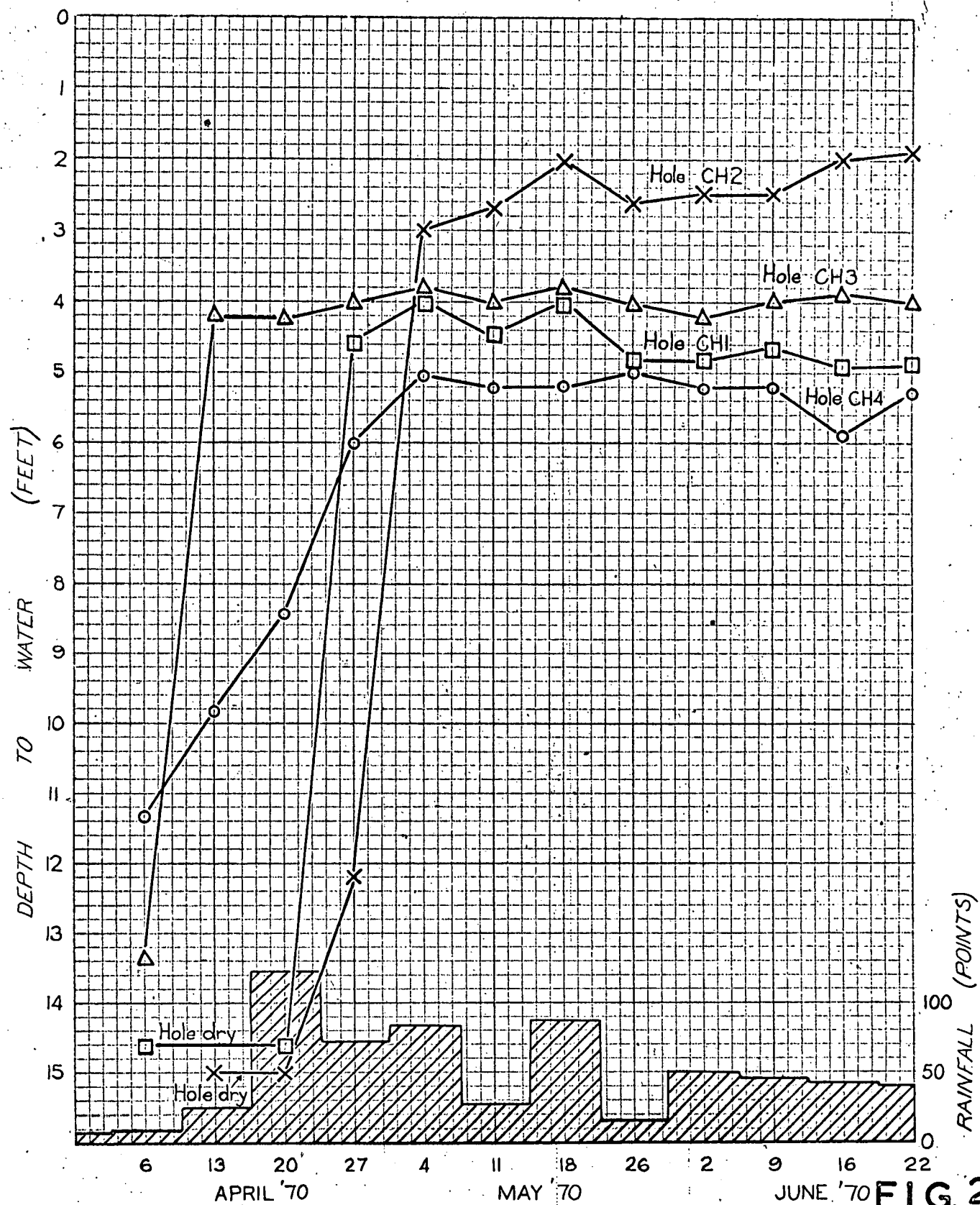


FIG. 2

DEPARTMENT OF MINES - SOUTH AUSTRALIA

Scale: As Shown.

Compiled: W.R.P.B.

**GOVERNMENT OFFICES
VICTORIA SQUARE WEST
OBSERVATION HOLES
WATER LEVEL MEASUREMENTS**

Date: 22 July 1970.

Drn. S.J.C. Ckd. L.V.W

Drg. No.
S7850
Ho 6