

*Adrian
Scot*



DEPARTMENT OF MINES SOUTH AUSTRALIA

GEOLOGICAL SURVEY
ENGINEERING DIVISION

SATELLITE TRACKING STATION - CEDUNA
GEOLOGICAL INVESTIGATIONS - PROGRESS REPORT NO. L
FEASIBILITY/DESIGN STAGE
Out of Hundreds, Co. Way

Client: The Overseas Telecommunications Commission (Australia)

by

W.R.P. Boucaut
Senior Geologist
Engineering Geology Section

and

I.S. Rowan
Geophysicist
Exploration Geophysics Section

DM.454/68

21st May, 1968

66/138

SATELLITE TRACKING STATION - CEDUNA
GEOLOGICAL INVESTIGATIONS - PROGRESS REPORT No.1

FEASIBILITY/DESIGN STAGE

Out of Hundreds, Co. Way

Client: The Overseas Telecommunications Commission (Australia)

by

W.R.P. BOUCAUT
SENIOR GEOLOGIST
ENGINEERING GEOLOGY SECTION
and

I.S. ROWAN
GEOPHYSICIST
EXPLORATION GEOPHYSICS SECTION

<u>CONTENTS</u>	<u>PAGE</u>
INTRODUCTION	1
SITE GEOLOGY	1
Topography	1
Soil Types	2
Rock Types	2
Geological Structures	3
Weathering	3
Groundwater	4
SEISMIC REFRACTION SURVEY	4
General	4
Results	5
DISCUSSION	6
Antenna Structure	6
Other Buildings	6
REFERENCES	6
APPENDIX A Logs of Diamond Drill Holes and Explanatory Notes	8
APPENDIX B Logs of Pits	11

<u>Fig. No.</u>	<u>Title</u>	<u>Plan No.</u>
1	Satellite Tracking Station - Ceduna, Geological Plan and Isopach Map of Seismic Refraction Interpretation.	68-398
2	Satellite Tracking Station - Ceduna, Results of Seismic Refraction Lines A & C	56586
3	Satellite Tracking Stations - Ceduna Results of Seismic Refraction Lines B & D	56587
4	Satellite Tracking Station - Ceduna Results of Seismic Refraction Lines E & F.	56588

DEPARTMENT OF MINES
SOUTH AUSTRALIA

Rep. Bk. No. 66/138
G.S. No. 3999
D.M. No. 454/68

SATELLITE TRACKING STATION - CEDUNA
GEOLOGICAL INVESTIGATIONS - PROGRESS REPORT NO. 1
FEASIBILITY/DESIGN STAGE
Out of Hundreds, Co. Way

Client: The Overseas Telecommunications Commission (Australia)

INTRODUCTION

A request to investigate foundation conditions for a proposed Satellite Tracking Station about 19 miles northeast of Ceduna was received in a letter dated 5th April, 1968 from The Overseas Telecommunications Commission.

The main structure, as yet undefined, may occupy an area of about 60 ft. diameter and will support a solid dish of about 100ft. diameter, capable of rotation in any direction.

Various single story buildings are to be erected over an area of about 200 ft. by 100 ft. immediately south of the main structure.

Investigations carried out at the site have included

-an inspection of the area with officers of the Commission to determine possible sites for the station
-a seismic refraction survey (Figs. 1 to 4)
-four diamond drill holes (DH 1 to 4) totalling 106 ft. (Appendix A)
-three test pits, 5 to 5.5 ft. deep, (Appendix B)

SITE GEOLOGY

Topography

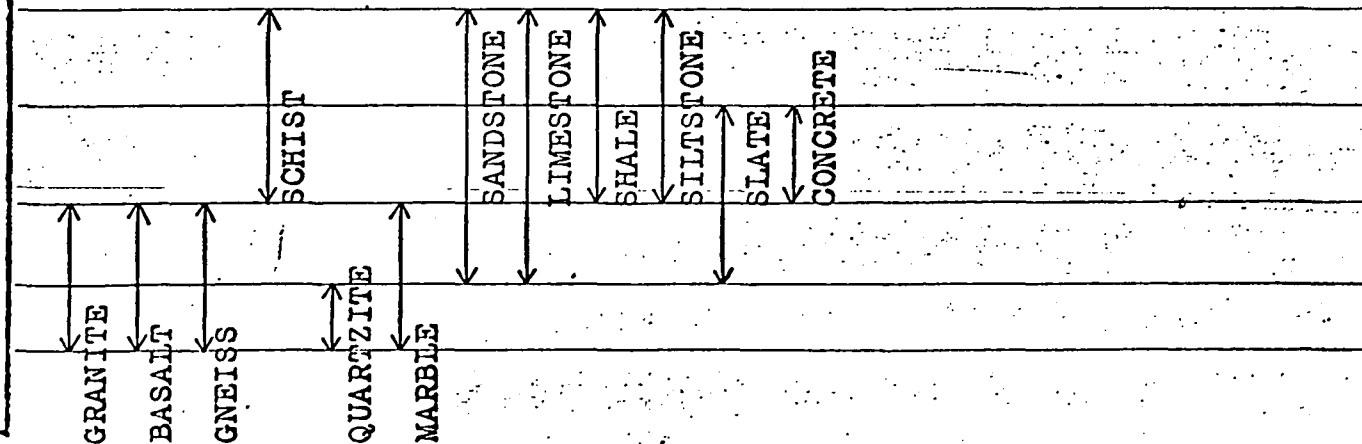
The site is in an area of northwest-southeast trending gently rolling sand dunes. They are up to 50 ft. high and from a few hundred feet to 0.5 miles apart with flat ground between. The site chosen

TABLE 1

CLASSIFICATION OF CLAY SOILS AND ROCK SUBSTANCES
BY UNCONFINED COMPRESSIVE STRENGTH.

	TERM	ABBREVIATION	UNCONFINED COMPRESSIVE STRENGTH (Kg/sq.cm)
SOIL PROPERTIES Disintegrates, or can be remolded, in water.	Very soft (clay)	VS	less than 0.25
	Soft "	S	0.25 - 0.5
	Medium "	M	0.5 - 1.0
	Stiff "	St	1.0 - 2.0
	Very Stiff "	V.St	2.0 - 4.0
	Extremely stiff clay	E.St	greater than 4.0
Boundary Zone Substances			
ROCK PROPERTIES Do not disintegrate, cannot be remolded, in water.	Very weak (rock)	VWR	less than 70
	Weak "	WR	70 - 200
	Medium Strong "	MSR	200 - 700
	Strong "	SR	700 - 1800
	Very strong "	VSR	greater than 1800

Range of strengths of some Common Rock Substances, in the Fresh State



for the Station is on the broad, nearly flat crest of one of these dunes.

The dunes are normally covered by dense low scrub, but in the chosen area the scrub has been removed and cereal crops planted.

Soil Types

The ground surface over much of the site is covered by a red brown (limy) sand, mainly fine grained, calcareous and containing numerous rounded to subangular calcrete gravel fragments 0.1 to 0.5ft. in diameter. The test pits constructed on the site indicate that this material is about 0.5ft. thick and has been disturbed by cultivation. It overlies a layer of calcrete about 0.5ft. thick. This is a nodular material consisting of "cores" of sandy limestone surrounded by "shells" composed of concentric layers of laminar limestone. Several "shells", representing different generations of calcrete formation may often be recognized. Calcrete is probably formed by the precipitation of calcium carbonate in soil. The calcrete fragments have the properties of a strong rock (Table 1) but the mass usually tends to be composed of discreet strong nodules in a matrix of weak lime-sand. The horizon varies in composition laterally, and in places may be completely absent (Pit 3).

Underlying the calcrete is a further layer of red brown limy sand, with some calcrete gravel fragments. This material extends to depths of 5.5ft. below ground surface in Pits 2 and 3, and to 5.0ft. in Pit 1. A further calcrete horizon was exposed in the floor of Pit 1 at a depth of 5ft.

The soil and calcrete horizons were penetrated by holes DH2 to 4 to depths of 9 to 11ft. Most of the soil was washed away in the drilling water and only fragments of calcrete were recovered.

Rock Type

About 300ft. southeast of the site chosen on the dune crest, granite outcrops over an area of about 100ft. diameter (Fig.1) The outcrop is generally 8ft. lower in elevation than the crest of the dune.

The granite is medium to coarse grained, and is essentially

composed of quartz and felspar with few micas. It is pink to grey in colour when fresh.

Geological Structures

Although the granite is a strong and compact rock in small samples, the mass has been weakened to varying degrees by geological defects such as joints, and crushed and weathered seams.

Three sets of joints are exposed in the outcrop.

Set. 1 Strike N 105° E, dip 75° N, spaced 5 to 20ft. apart and persistent up to 20ft.

Set 2 Strike N 15° E, dip 80° E, spaced 20 to 30ft. apart, and persistent up to 40ft.

Set 3 Near horizontal.

Joints of Set 1 and Set 2 are mainly tightly closed and strongly cemented. Joints of Set 3 are commonly weakly cemented or uncemented and open to 5mm.

Joints in the granite recovered during drilling had similar dips to those of Sets 1 to 3, but in holes DH 2 to 4 they were spaced less than 0.5ft. and were weakly cemented or uncemented with weathering products and limonite staining on joint faces.

Weathering

Weathering is defined here as disintegration of rock due to the action of surface agencies and associated phenomena. Two distinct types of weathering occur, namely mechanical and chemical.

Mechanical Weathering. This is mechanical failure of the rock due to the opening up of joints near the ground surface, and is believed to be initiated by the redistribution and release of stresses within the rock mass when overburden is removed by erosion.

In rocks which contain few joints of tectonic origin, irregular joints almost parallel to the ground surface tend to form as part of the mechanical weathering process. Keislinger (Ref. 1) describes these as Bankung or exfoliation joints. They are clearly formed by failure of the rock across its fabric. It is probable that the flat lying joints (Set 3) in outcrop and in

in the drilling cores are of this type.

Chemical Weathering. This is decomposition of the rock material due largely to the action of groundwaters. The rock materials are virtually impermeable, and the groundwater has gained access to the rock mass mainly by percolating along joints or seams. The degrees of chemical weathering are defined according to the physical properties of the weathering products as set out in Table 2.

The granite in outcrop and recovered during drilling of hole DH1 is fresh to slightly weathered. In holes DH2 to 4 the granite immediately below the topsoil is moderately to highly weathered. Holes DH3 and DH4 passed into fresh granite at depths of 21 and 14 ft. below the ground surface respectively but hole DH2 did not pass into fresh granite until 48ft. below the ground surface.

Groundwater

Following periods of rain, water accumulates in holes in the surface of the granite outcrop.

Groundwater was present in Holes DH2 and DH3 at 38ft. and 17ft. respectively, while in Hole DH2, 25ft. deep, no water was measured. The irregularity of the groundwater surface as indicated by these measurements is probably due to disconnected or partly connected bodies of water within the rock mass. This is typical of groundwater in jointed rocks (Ref. 2).

SEISMIC REFRACTION SURVEY

General

A shallow seismic refraction survey was carried out between 2nd and 8th April, 1968, under the direction of Mr. I. Rowan, Exploration Geophysics Section to determine depths to granite in the site area.

The S.I.E.P. 19,12 channel recording equipment mounted in a Land Rover was used for the survey. Four 900ft. spreads were completed along lines A, B, C and D, and eight more detailed 330ft. spreads were completed along part of lines A and B and

TABLE 2
WEATHERING PRODUCTS OF ROCK MATERIALS

TERM	ABBREVIATION ON DRAWINGS	DESCRIPTION
Fresh	Fr.	The rock shows no discolouration, loss of strength, or any other effect due to weathering.
Slightly Weathered	SW	The rock is slightly discoloured, but not noticeably lower in strength than the fresh rock.
Moderately Weathered	MW	The rock is discoloured and noticeably weakened, but 2 inch diameter drill cores cannot usually be broken up by hand, across the rock fabric.
Highly Weathered	HW	The rock is usually discoloured and weakened to such an extent that 2 inch diameter cores can be broken up readily by hand, across the rock fabric. Wet strength usually much lower than dry strength.
Completely Weathered	CW	The rock is discoloured and is entirely changed to a soil, but the original fabric of the rock is mostly preserved. The properties of the soil depend upon the composition and structure of the parent rock.

along lines E and F (Fig. 1).

Results

The quality of the records was good with the exception of a few isolated cases where the records were marred by crossfeed in the recorder. The seismic interpretation for the various lines are shown on Figs. 2, 3 and 4. As the ground surface levels are not known the depths given are depths below the surface at that particular point.

The seismic velocity through the soil and calcrete varies locally from 1,200ft/sec. to 3,000 ft/sec. The lower velocities probably correspond to the loosely compacted surface material, and the higher velocities probably indicate greater thicknesses of calcrete^{or}/perhaps highly weathered granite. Drill holes along line F, where the indicated thickness of this layer was 20 to 25 ft., showed 10ft. of soil and calcrete overlying several feet of highly weathered granite. An average velocity of 2,250ft./sec. for this layer was used in depth calculations.

The seismic velocity through the underlying material was between 16,500 ft/sec. and 19,000 ft./sec. The drill holes along line F showed the material with this seismic velocity to be fresh granite. The velocity of 14,000ft/sec. recorded on lines E and F may correspond to areas of more weathered granite such as was indicated by hole DH 2. At the intersection of lines E and A a lower velocity (7,000 ft/sec.) was recorded. This may indicate that more highly weathered granite is present, or that, if fresh granite is present, it dips steeply to the west at this point. This may also be the cause of the lack of interpretable records along the western end of line A.

An isopach plan using contours for the thickness of the low seismic velocity layer is shown in Fig. 1. Seemingly anomalous single depth values have been omitted as the contouring is only approximate with this sparse coverage. These results show that north-west of the outcrop depths to the granite are less than south or east where the granite seems to dip away relatively steeply.

DISCUSSION

Antenna Structure

In the fresh state granite is generally a medium strong to very strong rock (Table 1), but its strength decreases with increase in weathering. The change from rock to soil properties is gradational but is placed nominally between the highly and completely weathered states.

It is probable that nearly horizontal Bankung joints, which usually contain soil from infill or from weathering of the adjacent rock, are persistent in the granite over most of the area. The open joint at a depth of 22ft. in hole DH 4 (13ft. below top of granite) is probably a Bankung joint. The core losses at 16 and 31 ft. in hole DH 2 probably represent zones of weaker rock such as weathered zones, possibly associated with Bankung joints, or crushed seams.

General Buildings

The soils as exposed in the pits would not be subject to seasonal shrinkage and swelling movements of any significance and would be well drained.

Bearing capacity is not high due to the limy material present, which becomes soft and loses strength if allowed to become saturated, but experience has shown that for normal domestic type foundations, footings of the following dimensions are usually adequate.

EXTERNAL WALLS	:	2 ft. wide by 1ft. deep
INTERNAL WALLS	:	1.5ft. wide by 1ft. deep

Concrete raft type foundations also have proved to be satisfactory. Artificial consolidation of the soil by flooding and compaction with a vibrating roller is generally advised before footings are placed.

The following precautions are normally recommended for building constructed on these soils.

...All surplus surface water, downhill drainage, roof run-off etc. be carried well away from the footings in properly constructed drains of adequate capacity. Lawns and gardens should be kept well clear of the foundation area to prevent damage resulting from over-watering.

...A wide concrete, or heavy asphalt paving surround be placed around the building. This is designed to minimise soil moisture variations beneath the footings. If a concrete paving is used, it should be bonded to the footing by means of a bitumastic compound.

W.R.P. BOUCAUT
SENIOR GEOLOGIST
ENGINEERING GEOLOGY SECTION

WRPB:RW:OB:CL
22.5.68

I.S. ROWAN
GEOPHYSICIST
EXPLORATION GEOPHYSICS SECTION

REFERENCES

1. KEISLINGER, A. 1960. Residual Stress and Relaxation in Rocks. Proc. Int. Geological Congress, Copenhagen. Session 21 Part XVIII, pp. 270 - 276
2. TERZAGHI, K. 1962 Stability of steep slopes on Hard Unweathered Rock. Geotechnique. Vol. 12, No.4 pp. 251 - 270

APPENDIX A

LOGS OF DIAMOND DRILL HOLES AND
EXPLANATORY NOTES

APPENDIX A

LOGS OF DIAMOND DRILL HOLES

AND EXPLANATORY NOTES

NOTES ON DRILLING PROCEDURES

Equipment

The core sizes are as follows:-

<u>Symbol</u>	<u>Nominal Diameter of Cores</u>	
	<u>Inches</u>	<u>Centimeters</u>
NXC (NX casing)	2.8	7.2
NMLC	2.0	5.1

The NMLC cores were obtained with "M" type stationery inner tube core barrels fitted with bottom discharge bits. The inner tubes were of the split type, ensuring minimum disturbance of the core during removal from the barrel.

Storing and Marking of Core

Cores are stored in wooden boxes, each compartment of which is designed to contain five feet of core. The internal length of each compartment is actually five feet one inch, to allow for 100 per cent core recovery, when roughness of the ends of the core, and small inaccuracies in measurement when breaking it to fit the box, make it difficult to fit five feet of core in a compartment of exactly that length. The boxes are marked with consecutive compartment numbers at one end, and the drilled depths from the surface in feet at the other.

The cores were boxed in this manner at the drill sites, the core being placed in its appropriate place in the box as soon as it was extracted from the core barrel. The bottom of each lift was marked with paint immediately it was placed in the box, and a corresponding paint mark was made on the side of the core box. The measured depth of the hole in feet from the surface was painted on the side of the core box. Core losses where their position was not known, were assumed to occur at the bottom of each lift. Timber blocks cut to the correct length were used to indicate core not recovered.

The cores were stored in the P.M.G. Works Yard, Ceduna.

NOTES ON DETAILS SHOWN ON LOG SHEETS

At the head of the sheet general details of the hole are given.

The logs are plotted on a scale of 1 inch = 10 feet (1:120). In the column headed "Log", places where core was obtained are shown by stippling, and the traces of joints, and seams are shown diagramatically by means of symbols.

Places where core was lost are shown by blank spaces. The descriptions given on the log sheet refer only to material recovered as core. Core is lost by the material being ground or washed away during the drilling process; it may usually be inferred

that such material is relatively weak. Near the surface this weakness may arise from the unconsolidated nature of soils or weathered rocks. At greater depths the weakness may arise from weathering confined to narrow seams in otherwise fresh rock, or from sheared, crushed or jointed rock. It cannot always be assumed that the material not recovered is weak, since even solid rock core may be ground away and lost during drilling operations.

The core size symbol is shown on the log sheets under the heading "Core Size".

The drilling lifts and the approximate percentage core recovery are shown in the column "lift Core Recovery".

The degree of chemical decomposition or weathering of the rock material recovered is given in the column "Weathering" by means of a histogram-type plot.

The "Fracture Log" column shows the degree of fracturing of the core by means of histogram-type plot. Degree of fracturing means the degree to which the rock has mechanically broken up along geological defects such as joints, or seams.

In the column "Structures", angles shown on joints, bedding or other geological structures are the angles they make with the plane at 90° to the axis of the core.

DEPARTMENT OF MINES — SOUTH AUSTRALIA

PROJECT TRACKING STATION

LOG OF DIAMOND DRILL HOLE

HOLE No. D. H. 1

SERIAL No

CEDUNA

SECTION C^o WAY

FEATURE FOUNDATIONS

CO-ORDINATES

LOCATION DINAH ROCK HOLE

ANGLE FROM HORIZONTAL 90°

DIRECTION

R.L. Surface

R.L. Collar

Datum

FT.

FT.

DESCRIPTION OF CORE

WEATHERING

CORE SIZE, DEPTH

LOG

FRACTURE LOG

STRUCTURES

LIFT, CORE LOSS

WATER LEVEL

CASING

DRILL WATER LOSS

WATER PRESSURE TESTS

LUCEON'S

10

20

30

40

50

60

70

80

90

100

5

10

50

100

DATE

21 Apr 68

Not Tested

GRANITE

Ironstained

Medium to coarse grained; mainly fresh, lightly iron-stained in top 2ft, with some decay of feldspars.

NX

NM

LC

Joint, near horizontal, irregular, tight, lightly ironstained.

END OF HOLE 525ft.

WEATHERING

ENGINEERING GEOLOGY SECTION

FR — Fresh

SW — Slightly weathered

MW — Moderately "

HW — Highly "

CW — Completely "

DRILL No 16

TYPE

DRILLER Ken Rogers

START 9 Apr 68

FINISH 2 Apr 68

LOGGED BY J.A.C.P.

DATE 21 Apr 68

TRACED R.H.

CHECKED WATB

FRACTURE LOG

1 4 16 64

12 5 3/4 2 1/2

Natural fractures per foot of core

Equivalent diameter in inches

5578

PROJECT TRACKING STATION

LOG OF DIAMOND DRILL HOLE

SERIAL No

.CEDUNA

SECTION

C° WAY

FEATURE FOUNDATIONS

CO-ORDINATES

R.L. Surface

FT.

LOCATION, DINAH, ROCK HOLE

ANGLE FROM HORIZONTAL. 90°

R.L. Collor

FT

DIRECTION

Datum

[illegible]

WEATHERING

FR — Fresh
SW — Slightly weathered
MW — Moderately “
HW — Highly “
CW — Completely “

FRACTURE LOG

1 4 16 64 — Natural fractures per
 — foot of core
 12 3 1 1/4 1/16 — Equivalent diameter
 in inches

ENGINEERING GEOLOGY SECTION

DRILL No. 16
TYPE
DRILLER Van Rooyen
START 20. Dec. 68
FINISH 11. Apr. 69

LOGGED BY
J.A.C.P.
DATE 24 Apr 68
TRACED R.H.
CHECKED W.B.B.

SHEET. 1. OF 1. DRG. No S6579 Dc

PROJECT TRACKING STATION

LOG OF DIAMOND DRILL HOLE

SERIAL No

CEDUNA

SECTION

C° WAY

FEATURE FOUNDATIONS

CO-ORDINATES

R.L. Surface

FT.

LOCATION. DINAH ROCK HOLE

ANGLE FROM HORIZONTAL. 90°

R.L. Collar

FT

DIRECTION

Datum

DESCRIPTION OF CORE	SW WEATH- ING	CORE SIZE, DEPTH LOG	FRACTURE LOG	STRUCTURES JOINTS, VEINS, SEAMS, SHEARED ZONES, CRUSHED ZONES	LIFT,	WATER LEVEL	CASING	WATER PRESSURE TESTS
					CORE LOSS %		DRILL PIPE LOSS %	
		R.L. FT.	1 4 16 64		5 50 10	DATE	0-100	10 20 30 40 80 120
CALCRETE in BAKARA SOIL	NO CORE	AX						
Very strongly calcareous cemented fine grained sand.	NO CORE	10'		11 to 14ft Joints spaced 5mm to 0.2 ft, near horizontal, irregular, limonite stained, clay & limonite coatings.		24 Apr '68		Not Tested
GRANITE.	NM/C	20'		Joint, dip 70°, slightly rough, regular, up to 1mm. calcareous cement. Joint, limonite stained, and contains earthy limonite coating. Several Joints, near horizontal limonite stained, irregular, up to 0.2 ft. MW rock adjacent. Joints, tight, limonite cemented.				
Medium to coarse grained grains mainly 2 to 5mm. essentially quartz and feldspar, few mica (biotite). When weathered feldspar decays to kaolin and other clay minerals, rock is whitish with yellow-brown staining. When fresh, rock is finely mottled, pinkish-gray.				END OF HOLE 25ft.				
		30'						
		40'						
		50'						
		60'						
		70'						
		80'						
		90'						
		100'						

WEATHERING

FR -- Fresh
SW -- Slightly weathered
MW -- Moderately "
HW -- Highly "
CW -- Completely "

FRACTURE LOG

Figure 1 is a schematic diagram of a core with natural fractures. The core is represented as a horizontal cylinder with a diameter of 12 inches. Natural fractures are shown as vertical lines intersecting the core. A legend indicates that the solid lines represent 'Natural fractures per foot of core' and the dashed lines represent 'Equivalent diameter in inches'.

ENGINEERING GEOLOGY SECTION

DRILL No 16
TYPE
DRILLER Van Rooyen
START 18 Apr '68
FINISH 17 Apr '68

LOGGED BY
V.A.G.P.
DATE 24 Nov 88
TRACED R.H.
CHECKED WATB

SHEET. 1. OF 1.	DRG. No	86580	πc
-----------------	---------	-------	----

LOG OF DIAMOND DRILL HOLE

Co. WAY

R.L. Surface

ANGLE FROM HORIZONTAL 90°

R.L. Collar

DIRECTION

Datum

WEATHERING		ENGINEERING GEOLOGY SECTION	
FR - Fresh		DRILL NO : 10	LOGGED BY
SW - Slightly weathered		TYPE	JAEF/HWS
MW - Moderately ..		DRILLER Von Rooyen	DATE 24 APR '68
HW - Highly ..		START 23 APR '68	TRACED J.A.W.
CW - Completely ..		FINISH 25 APR '68	CHECKED L.V.W.
FRACTURE LOG		SHEET . OF . DRG. No. \$6599	
1 4 15 64	Natural fractures per foot of core		
12 3 1/4 1/8	Equivalent diameter (in inches)		

APPENDIX B

LOGS OF PITS

APPENDIX B

Three test pits up to 5.5 ft. deep were excavated using a truck mounted power auger drill. These were put down in the area of the general buildings to the south of the main antenna structure. Logs of the pits are attached. Their location is shown on Fig. 1.

LOG OF PIT

PROJECT SATELLITE TRACKING STATION, CEDUNA

LOCATION DINAH ROCK HOLES

LANDFORM DUNES

RELIEF GENTLY ROLLING

MICRORELIEF CULTIVATED PADDOCK

DRAINAGE External RAPID

Internal

Direction of fall.

Surface Absorption MODERATE

SURFACE VEGETATION Type WHEAT CROP

SAMPLE NUMBER	SOIL/ROCK HORIZON	R.L. (Feet)	DEPTH (Feet)	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification U.S.B.R. Earth Manual 1st Ed. Rev 1963	OTHER GEOLOGICAL PEDOLOGICAL DESCRIPTION	SOIL/ROCK STRUCTURE	WATER LEVEL	MOISTURE CONTENT	Consistency Compaction Pct Density	SOILTEST PENETRO-METER UNITS*
					SP	SAND, poorly graded, fine grained, up to 20% silt and clay fines. Mainly 5% GRAVEL, rounded fragments to 0.2ft. size. Sand is quartz and calcareous fragments, gravel is CALCRETE.						
					GP							
			1									
						GRAVEL, poorly graded, fragments mainly angular, 0.1 to 0.5ft. size, often cemented into blocks up 1.5ft. across. Contains up to 20% SAND, fine grained, calcareous, between blocks and in pockets. Gravel is CALCRETE.						
			2									
					SP	SAND, poorly graded, fine grained, up to 15% silty fines, grades to GRAVEL well graded (approx 40 to 70%) rounded fragments to 0.5ft. size. Generally weakly to strongly cemented near top, becoming less cemented with depth - near 4ft. sand is loose friable. Several patches of weak white earthy lime silt to 1.5ft. diameter. Sand is quartz and calcareous grains, gravel is CALCRETE.		Granular. Calcrete is moderately strong rock.				
			3		GW							
			4									
					GP	GRAVEL, poorly graded, fragments from 0.5 to 1ft. size. Gravel is CALCRETE (as from 0.5 to 1.0ft. depth).						
			5									
						BASE OF PIT 5.0ft.						
			6									
			7									
			8									

REMARKS

* These values refer to clay soils only and provide an indication of their consistency.

CLASSIFICATION	CONSISTENCY (CLAY)	COMPACTNESS (SILT)	RELATIVE DENSITY (SAND)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION
Great Soil Group	VS — Very Soft	Ls — Loose	VL — Very Loose	H — Humid	PLANT TYPE
Subgroup	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	LOGGED
REFERENCE	F — Firm	C — Compact	MD — Medium Dense	M — Moist	J.A.C.P.
Fuller	St — Stiff	VC — Very Compact	D — Dense	W — Wet	DATE 24.4.68
Moist	V. St — Very Stiff		VD — Very Dense	S — Saturated	TRACED
Water level (date)	H — Hard			LL — Liquid Limit	CHECKED
Water cut				PL — Plastic Limit	

PROJECT SATELLITE TRACKING STATION - CEDUNA

LOCATION . DINAH ROCK HOLES

SECTION . . . ~~HUNDRED~~ CO. WAY

LANDFORM. DUNES

RELIEF GENTLY ROLLING

Direction of foll.

MICRORELIEF CULTIVATED Paddock

DRAINAGE External RAPID

Internal

Surface Absorption MODERATE

SURFACE VEGETATION Type WHEAT CROP

Granular.
Calcrete is
moderately
strong rock

BASE OF PIT 5.5.ft.

REMARKS

* These values refer to clay soils only and provide an indication of their consistency.

CLASSIFICATION		7 May 67 Water level (date) WE > Water cut	CONSISTENCY (CLAY)	COMPACTNESS (SILT)	RELATIVE DENSITY (SAND)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
Great Soil Group	VS — Very Soft		LS — Loose	VL — Very Loose	H — Humid	PLANT TYPE START FINISH	LOGGED J.A.C.P. DATE 21 June 68 TRACED CHANGED	
Subgroup	S — Soft		MC — Moderately Compact	L — Loose	D — Damp			
	F — Firm		C — Compact	MD — Medium Dense	M — Moist			
REFERENCE			VC — Very Compact	D — Dense	W — Wet			
Fuller				VD — Very Dense	S — Saturated			
Moist					LL — Liquid Limit			
Plastic					PL — Plastic Limit			
						SHEET 4 OF 1 DRG NO. 36582 Do		

56109 MB

LOG OF PIT

PROJECT SATELLITE TRACKING STATION - CEDUNA

LOCATION DINAH ROCK HOLES

SECTION

HUNDRED CO. WAY

LANDFORM DUNES

RELIEF GENTLY ROLLING

Direction of fall.

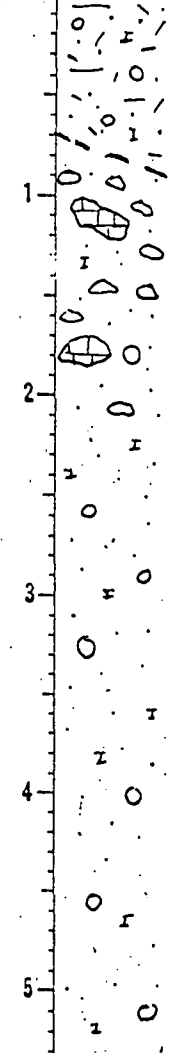
MICRORELIEF CULTIVATED Paddock

DRAINAGE External RAPID

Internal

Surface Absorption MODERATE

SURFACE VEGETATION Type WEAT CROP

SAMPLE NUMBER	SOIL/ROCK HORIZON	R.L. (feet) DEPTH (feet)	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification U.S.B.R. Earth Manual 1st Ed. Rev 1963	OTHER GEOLOGICAL PEDOLOGICAL DESCRIPTION	SOIL/ROCK STRUCTURE	WATER LEVEL	MOISTURE CONTENT Consistency Comp. Density	SOILTEST PENETROMETER UNITS *
		1		SP	SAND, poorly graded, fine grained-up to 15% silt and clay fines, and 5 to 25% GRAVEL, as rounded fragments, 2 to 20mm diameter. Grey brown, friable. Sand is quartz, calcareous fragments, Gravel is CALCRETE		Granular to poorly clddy			
		2		SP/GW	Increased GRAVEL content to 25 to 30% occasionally weakly to moderately cemented into boulders up to 0.6ft. dia.		Calcrete is moderately strong rock.			
		3		SP	Decreasing GRAVEL content to 10 to 20%. Fragments mainly 2 to 5mm. diameter.					
		4								
		5								
		6			BASE OF PIT 5.5ft.					
		7								
		8								

REMARKS

* These values refer to clay soils only and provide an indication of their consistency.

CLASSIFICATION	CONSISTENCY (CLAY)	COMPACTNESS (SILT)	RELATIVE DENSITY (SAND)	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION
Great Soil Group	VS — Very Soft	LS — Loose	VL — Very Loose	H — Humid	PLANT TYPE
Subgroup	S — Soft	MC — Moderately Compact	L — Loose	D — Damp	LOGGED
REFERENCE	F — Firm	C — Compact	MD — Medium Dense	M — Moist	DATE
Fuller	St — Stiff	VC — Very Compact	D — Dense	W — Wet	THATCHED
Map	V. St — Very Stiff		VD — Very Dense	S — Saturated	CHURCH
Pr. to	H — Hard			LL — Liquid Limit	
				PL — Plastic Limit	

J.A.C.P.

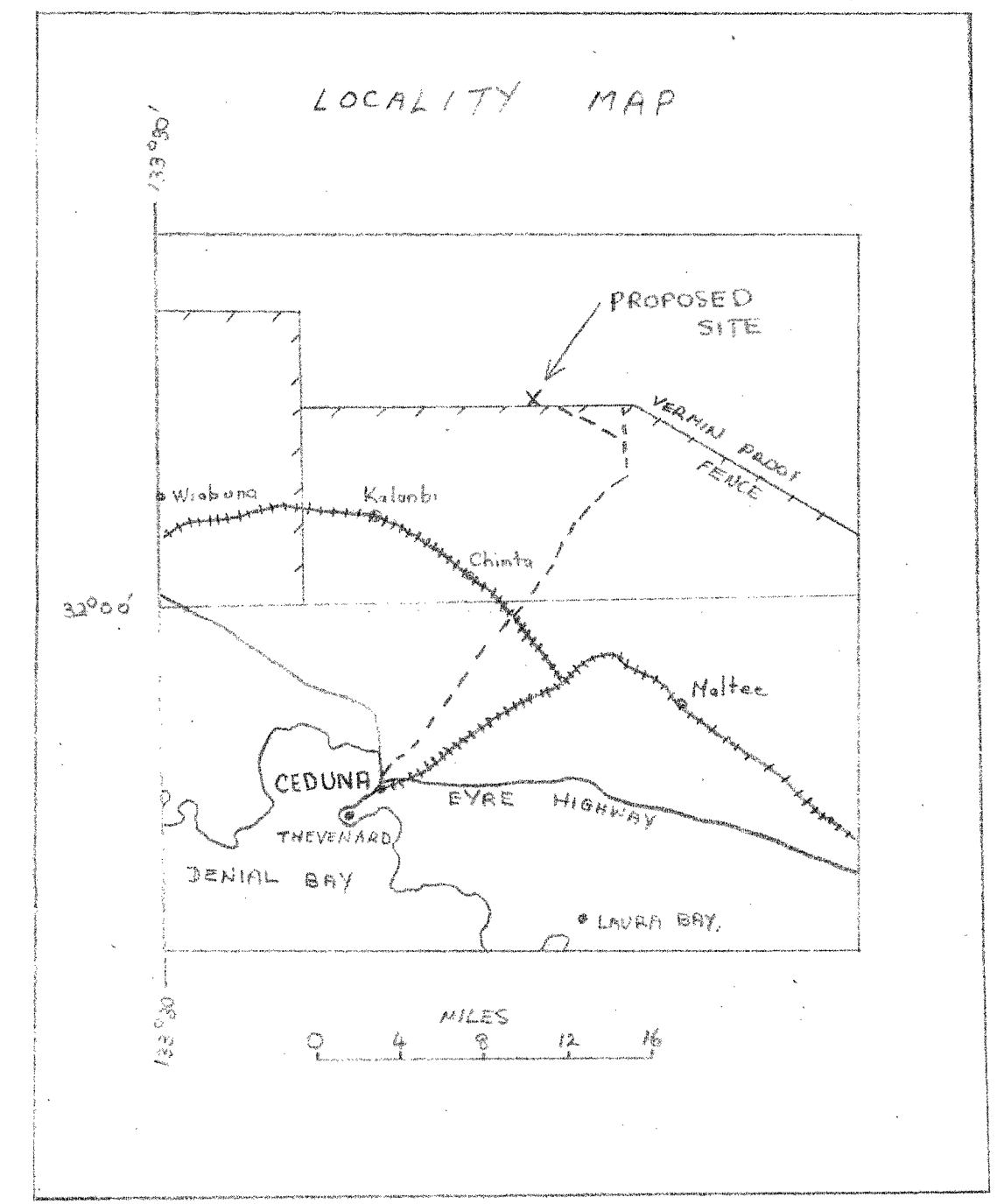
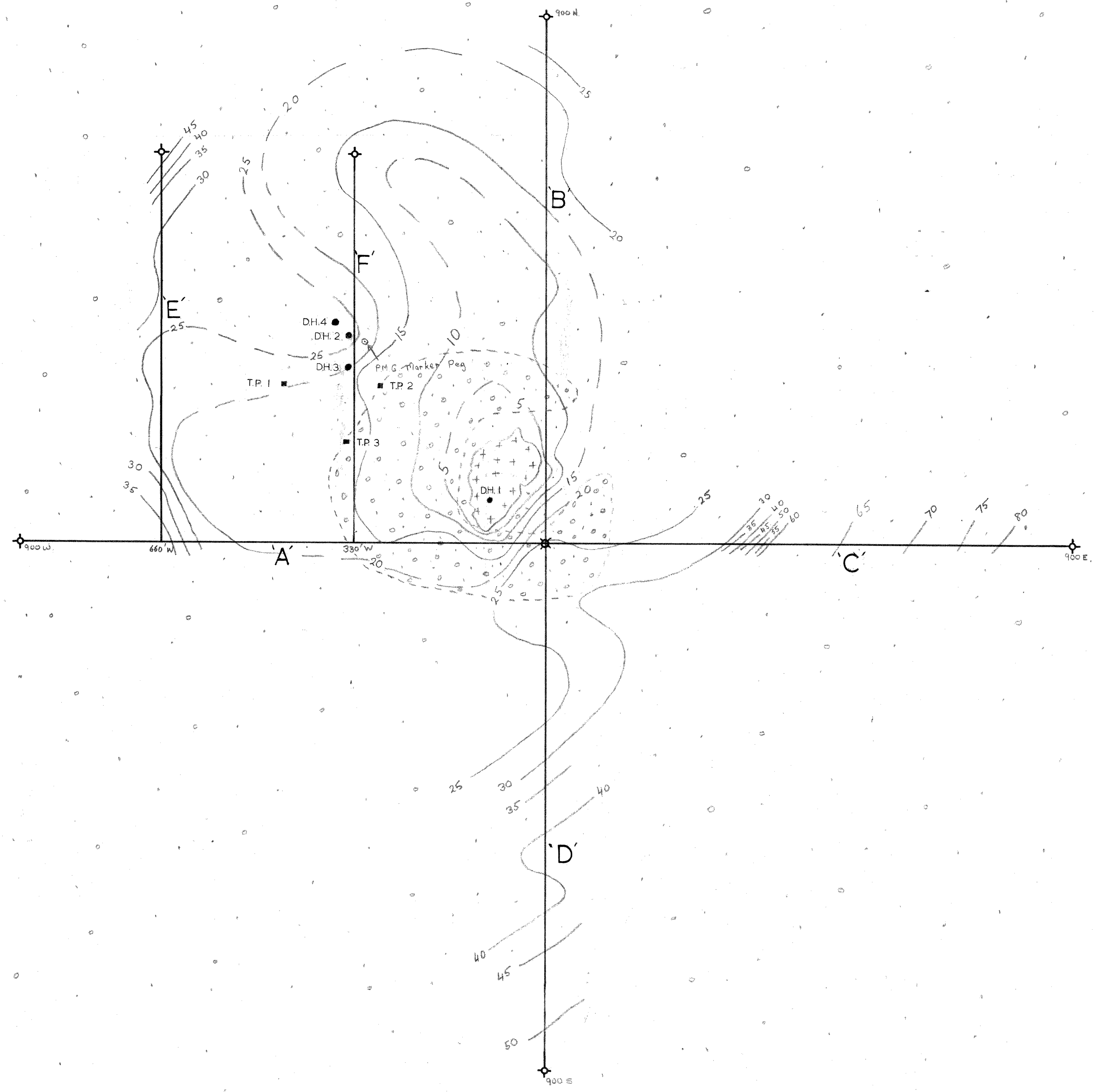
DATE

THATCHED

CHURCH

SHEET 1 OF 1

6583



LEGEND

-  TOPSOIL, red brown sand, with CALCRETE gravel to 0.5 ft diameter
-  GRANITE
-  DH.1 Diamond Drill Hole No 1
-  TP.1 Test pit No 1
-  25 Contour showing depth in feet of top of high velocity layer below ground surface
-  SEISMIC TRAVERSE A

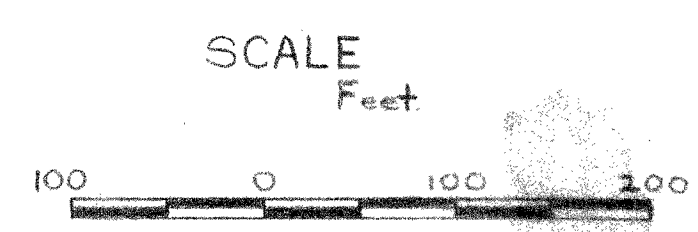


FIG. 1.

DEPARTMENT OF MINES — SOUTH AUSTRALIA			
SATELLITE TRACKING STATION — CEDUNA			
GEOLOGICAL PLAN AND ISOPACH MAP OF			
SEISMIC REFRACTION INTERPRETATION			
ENGINEERING GEOLOGY SECTION	WALSH GEOLOGIST	Drm. N.T. Tcd. N.T.	SCALE: AS SHOWN
	W.B. Bourne SEN. GEOLOGIST	Ckd. W.A.B. Exd.	68-398 Dc.
Director of Mines			DATE: 9 MAY 68

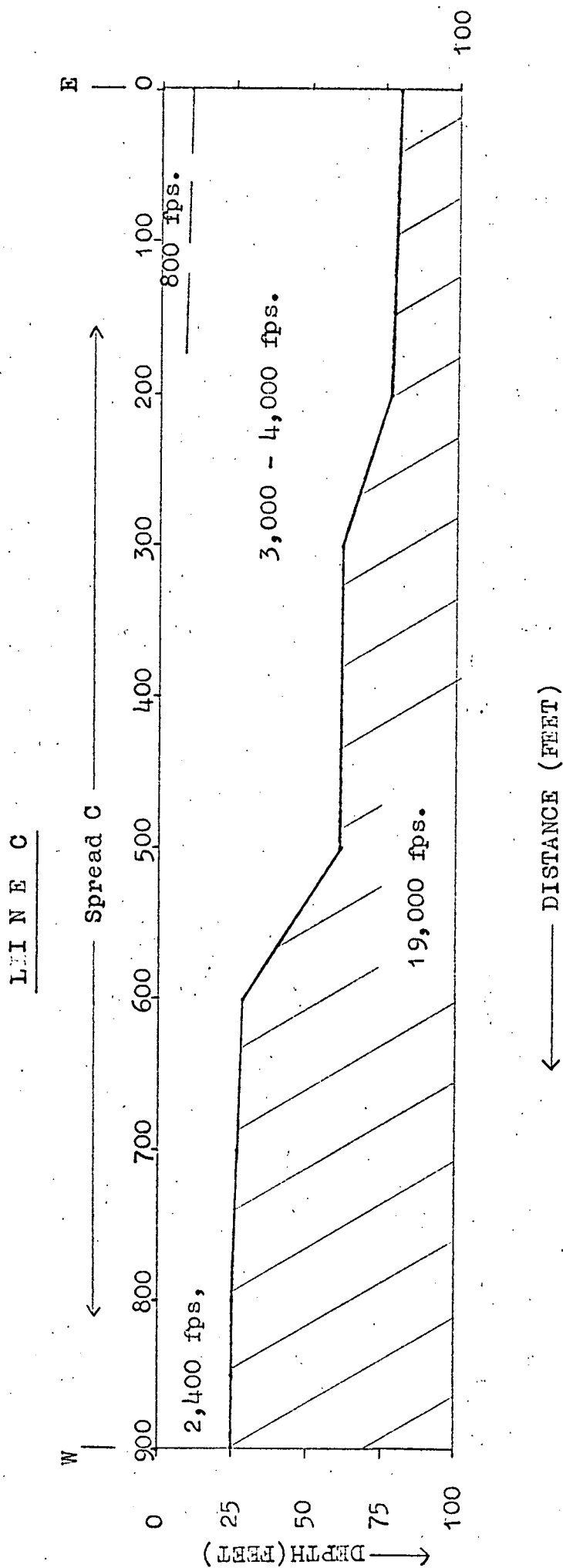
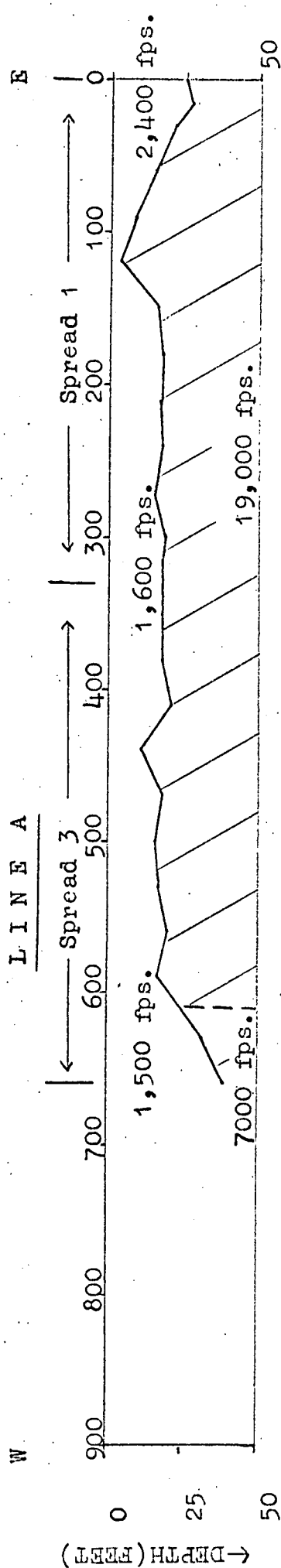


FIG 2

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Geophysicist

O.I.C.

Drn-IR

Tcd.

Ckd.

Exd.

SATELITE TRACKING STATION
CEDUNA

Results of Seismic Refraction
Lines A & C

SCALE: As Shown.

56586

De

DATE: MAY '68

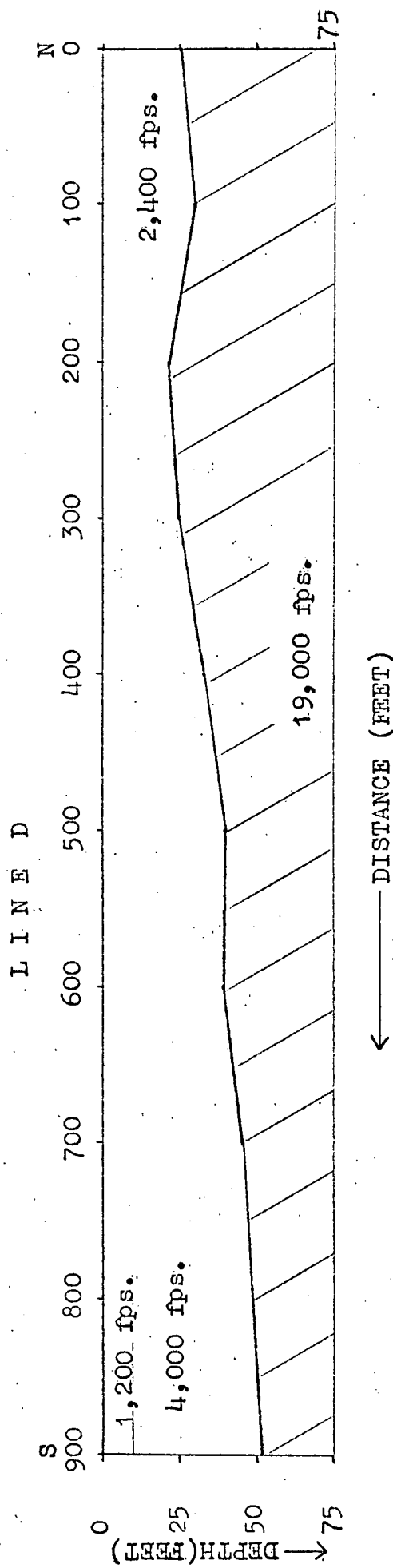
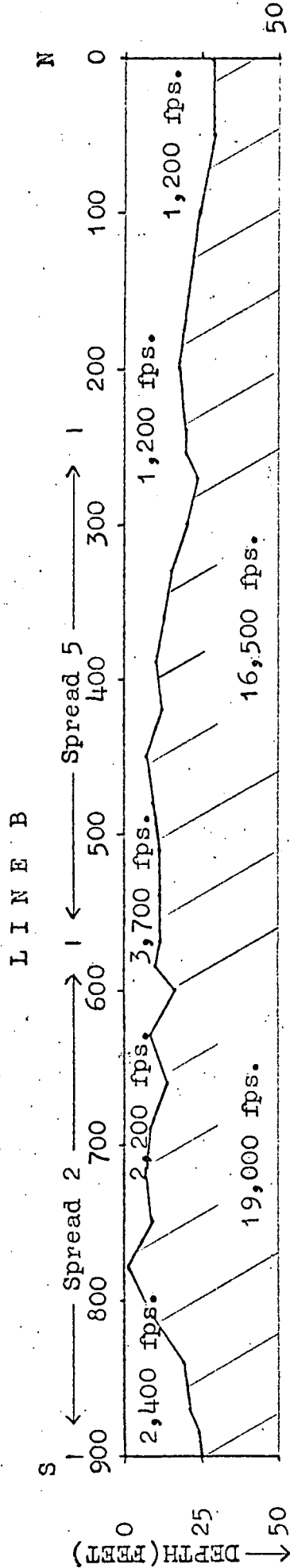


FIG 3

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Geophysicist.

Dr. R.

Tcd.

Ckd.

Exd.

SATELITE TRACKING STATION
CEDUNA
Results of Seismic Refraction
Lines B & D

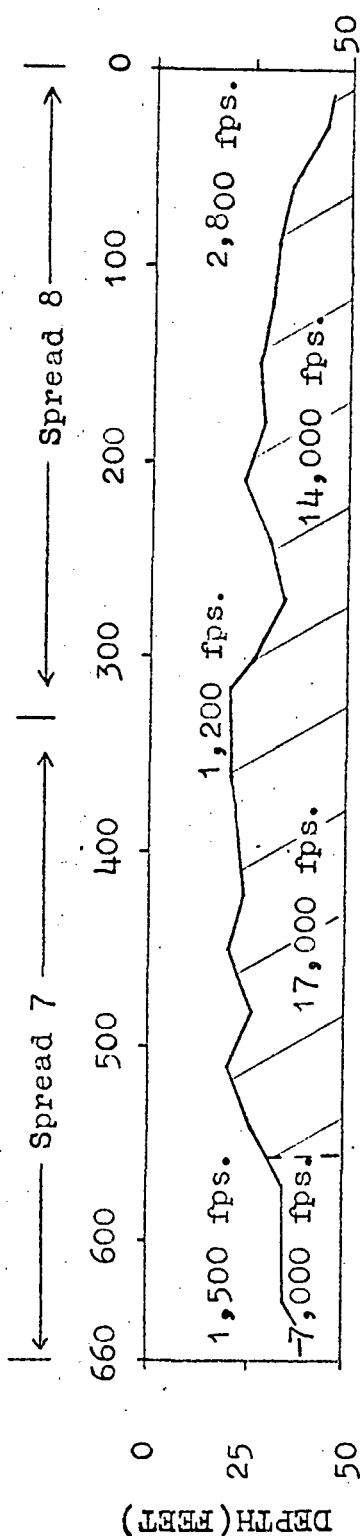
SCALE: As Shown.

S6587 DC

DATE: MAY '88

O.I.C.

LINE E



LINE F

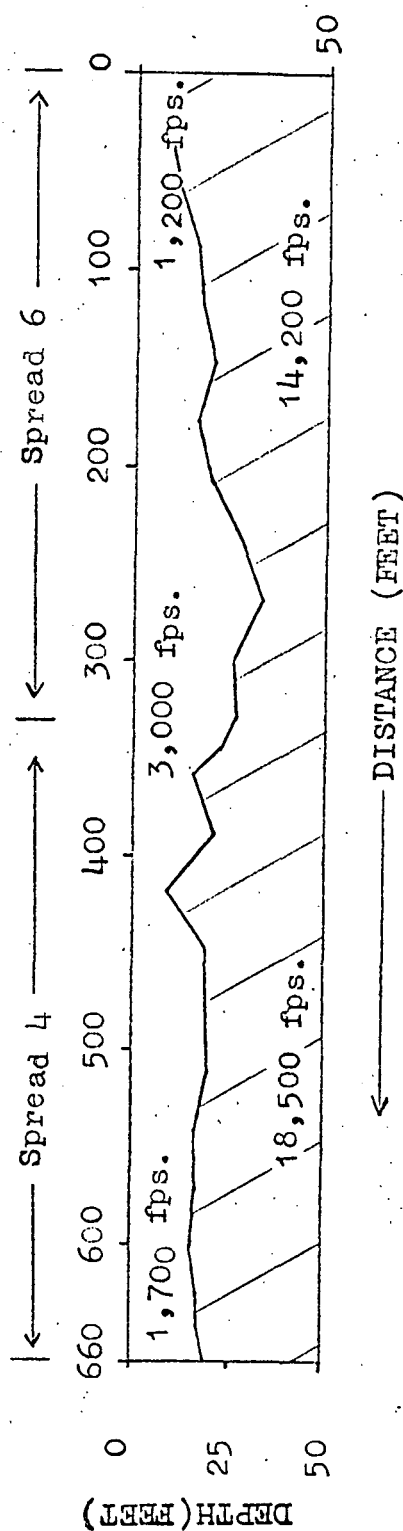


FIG 4

DEPARTMENT OF MINES — SOUTH AUSTRALIA

Geophysicist	Drn.	SATELITE TRACKING STATION CEDUNA Results of Seismic Refraction Lines E & F	SCALE: <i>As shown.</i>	
	Tcd.		56588 <i>Dc</i>	
O.I.C.	Ckd.			DATE: <i>MAY 68</i>
	Exd.			