

RB 66/72

COMMONWEALTH LAW COURTS BUILDING
GEOLOGICAL INVESTIGATIONS - PROGRESS REPORT NO. 1

DESIGN STAGE

TA 453, Hd. Adelaide

Client: Commonwealth Department of Works

by

J.H. FRYAR

GEOLOGIST

ENGINEERING GEOLOGY SECTION

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	" 4	

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2.	" " " " " 4.

12th March, 1968

Rept. Bk. No. 66/72
G.S. 3934
D.M. 1725/67

Rept. Bk. No. 66/72
G.S. 3934
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INTRODUCTION

Foundation investigations for the proposed Commonwealth Law Courts Building were requested in a letter dated 3rd November, 1967, from the Acting Director of Works.

The proposed building will be about 130ft. long and 100ft. wide. No other details are available but it is understood that it will be multi-storey.

Footing types under consideration include bored piers and a shallow raft structure.

The investigation consisted of four cable tool drill holes, each about 60ft. deep at the corners of the proposed building (Fig. 1). The holes were tube sampled throughout, with sealed tube samples taken in Hole C.H.3 at intervals of about 5ft. The holes were extended to 92ft., 70.5ft., 70ft. and 64ft. respectively by diamond drilling.

Geological logs of the holes are included as Figs. 3 to 6. Levels shown on the logs are based on contours shown on the City of Adelaide 4 chains to 1 inch plan, and are accurate only to the nearest foot.

Results of holes CH1 and CH4 are summarized on Fig. 2.

This report discusses the results of the drilling and also includes relevant information gained from drill holes and excavations elsewhere in the city area.

SITE GEOLOGY

Soil and Rock Types

The results of the drilling in the four holes indicate a rather uniform near-horizontal succession. This is summarised in Table 1:

TABLE 1

Depth (ft.) From To		Age	Unit	Description
0	2			FILL material, sand and gravel mixture.
2	6	PLEISTOCENE TO RECENT	HINDMARSH CLAY	CLAY SOIL, medium plasticity, containing slight amounts of lime sand and gravel.
6	30			CLAY SOIL, high plasticity, mainly grey, red brown, and ochre yellow mottled, with considerable amounts of earthy lime in pockets.
30	39			SAND, coarse to medium, with excess clay fines. Consists mainly of quartz grains. Yellow and red brown, mottled.
39	52			CLAY SOIL, high plasticity, grey-green with yellow mottling, limy near its base.
----- Erosion Surface -----				
52	82	UPPER PLIOCENE	HALLETT COVE SANDSTONE	SANDSTONE, fine grained, fossiliferous, white to buff with calcareous cement. The upper 5 to 10ft. occurs as lime gravel with approximately 40% clay in pockets sometimes completely surrounding sandstone fragments. Several solution cavities up to 50mm. size occur parallel to joints or to indistinct horizontal bedding. Limonite staining occurs near its base.
82	92	OLIGOCENE	PORT WILLUNGA BEDS	SILT SOIL, low plasticity, grading from ochre yellow to dark grey. Fossiliferous, gaseonitic, limy.

From other drill holes in the site area, particularly at the Adelaide Central Market (Ref. 1) and at the P.M.G. Communications Building, Waymouth Street (Ref. 2) it is known that Pt. Willunga beds are underlain by Blanche Point Marls of Oligocene age to depths up to 150ft.

Holes 1, 2 and 3 suggest two distinct layers in the Hallett Cove Sandstone, as follows:-

Layer A

This is the upper 5 to 10ft. and consists of about 50% strong white calcareous sandstone separated by pockets of firm clay up to 10cm. diameter and few clay beds up to 5cm. thick. There are also some vertical joints present.

This layer probably represents a weathered zone at the top of the sandstone.

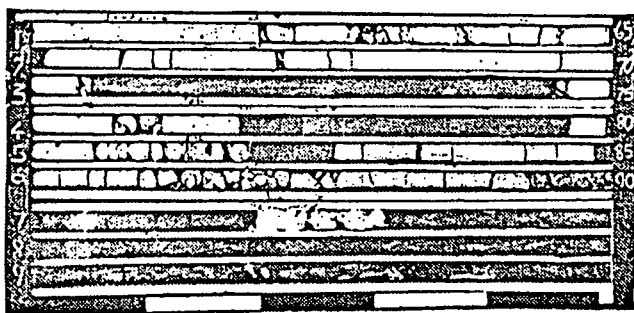
Layer B

This is about 20ft. thick and is dense calcareous sandstone containing irregular solution cavities and numerous near-horizontal fractures. There are also a few near-vertical cemented joints. Solution cavities are concentrated along these joint planes. Weak limestone gravel bands also occur, ranging in thickness from 0.5 to 2ft. Core losses probably represent cavities in the limestone or lenses of sandy material washed away during drilling.

Groundwater

The main groundwater table was cut within the Hallett Cove Sandstone at about 58ft. below ground level. In CH2, at 41ft., water was cut at the base of the sand horizon within the Hindmarsh Clay. Dampness was recorded in the other holes at about

PLATE I



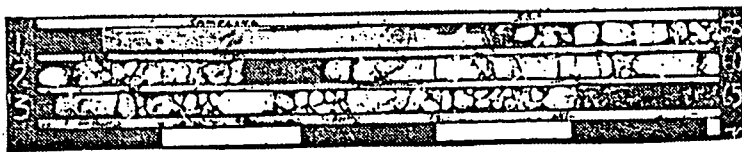
30559.

COMMONWEALTH LAW COURTS BUILDING

DIAMOND DRILL CORE

HOLE C.H. 1

PLATE 2



30560.

COMMONWEALTH LAW COURTS BUILDING

DIAMOND DRILL CORE

HOLE C.H. 4

35ft. (within the sandy horizon) but there was no inflow. Minor quantities of water were noticed in the upper clay horizons between 6 and 18ft. below the ground surface. This water was probably from a "perched" water table associated with "gilgai" structures in the expansive clays as discussed below.

DISCUSSION

Hindmarsh Clay

Drilling results and experience at other sites, suggest that these clays are fairly uniform in lateral extent (Fig. 2). They are highly plastic (CH) soils, mainly very stiff, at moisture contents close to the plastic limit.

The clay is intersected by two sets of near-vertical joints roughly at right angles to each other, spaced about 0.2ft. apart, which tend to divide the mass into a series of vertical columns. Other less regular low dipping (30° to 45°) joints occasionally occur.

The upper 6 to 18ft. of clay is limy and is considerably weaker than the underlying clay. It is probable that the upper surface of the clay is irregular and deformed into a series of mounds and depressions (gilgai structures, Ref. 3). These depressions often contain pools of perched groundwater.

The sand and clayey sand horizon about 30ft. below the ground surface is dense to medium dense.

Hallett Cove Sandstone

Hole 1, the only hole to penetrate to the base of the formation, showed a total thickness of Hallett Cove Sandstone Formation of 29ft.

Observations at other sites in the city and as indicated by the variations between each drill hole at the site, especially Hole 4, show that the sandstone is extremely variable both laterally and vertically. The rock substance ranges from very weak to strong (Table 2), and the rock mass may be weakened by joints, solution cavities, clay seams or pockets, or lenticular beds of sand.

The upper surface of the sandstone is also highly irregular in shape. Buried erosional pinnacles and sink holes are common. One large sinkhole approximately 20ft. in diameter was discovered during pier boring at the Public Library Site, Kintore Avenue, (Ref. 4).

CONCLUSIONS

The following conclusions are based on drilling results, and experience elsewhere in the city area:-

Pier Foundations

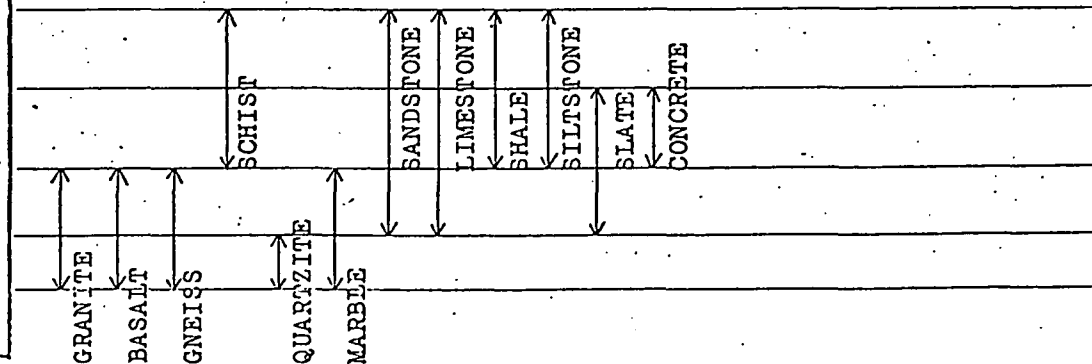
- Auger drilling is feasible through the Hindmarsh Clay Formation. Casing is generally required as any water cut can weaken the clays and cause collapse (Ref. 2).
- Belling in Layer A by machine has not been possible and hand mining methods have been used.
- Piers could be founded in the Hallett Cove Sandstone Formation, probably at the top of Layer B. The presence and thickness of sandstone suitable for pier foundations should be confirmed at each pier site either by drilling before construction starts or by visual examination and drilling at each pier base during construction (Ref. 5).

TABLE 2

	TERM	ABBREVIATION	UNCONFINED COMPRESSIVE STRENGTH (Kg/cm ²)*
SOIL PROPERTIES Disintegrates, or can be remolded, in water.	Very soft (clay)	VS	less than 0.25
	Soft "	S	0.25 - 0.5
	Medium "	M	0.5 - 1.0
	Stiff "	St	1.0 - 2.0
	Very Stiff "	V.St	2.0 - 4.0
	Extremely stiff clay	E.St	greater than 4.0
Boundary Zone Substances			
ROCK PROPERTIES Do not disintegrate, cannot be remolded, in water.	Very weak (rock)	VWR	less than 70
	Weak "	WR	70 - 200
	Medium Strong "	MSR	200 - 700
	Strong "	SR	700 - 1800
	Very strong "	VSR	greater than 1800

*The rock substances are fresh (unaffected by decomposition) samples, tested to A.S. Ref. H. For rocks showing planar anisotropy, the long axis of the samples is normal to the fabric planes.

Range of strengths of some Common Rock Substances, in the Fresh State



.... Cement grouting has been used in an attempt to strengthen the Hallett Cove Sandstone. Generally it has been of doubtful value due to clay and fine sand in joints and cavities, preventing the penetration of grout. The grout takes can vary considerably depending on the number of solution cavities and fissures intersected.

Raft Foundations

The clays at depths greater than 18ft. below ground surface, are generally very stiff, and it should be possible to utilize them for a raft foundation. At higher levels there is likely to be a greater degree of lateral variation in type of soil, moisture content and strength.

Basement Excavation

This could be readily carried out in the Hindmarsh Clay Formation by mechanical equipment. The joints control the overall strength of the clay, which may be significantly lower than that obtained in the laboratory by testing of small clay samples (Ref. 6). In most cases of failure of excavation walls in the city, failure has occurred along pre-existing joint planes. However in most of these failures water has been allowed to seep into the joints thus weakening the clay mass further. Seepage of perched water from gilgai structures is often a major cause of these failures. If the basement of the proposed building lies below the upper surface of the mottled clays, provision for permanent drainage of perched water is desirable.

FURTHER INVESTIGATION

If a basement is proposed it is considered desirable to explore the site further by means of several backhoe trenches up to 18ft. deep. These give a good sampling of near surface gilgai structures, with associated groundwater problems.

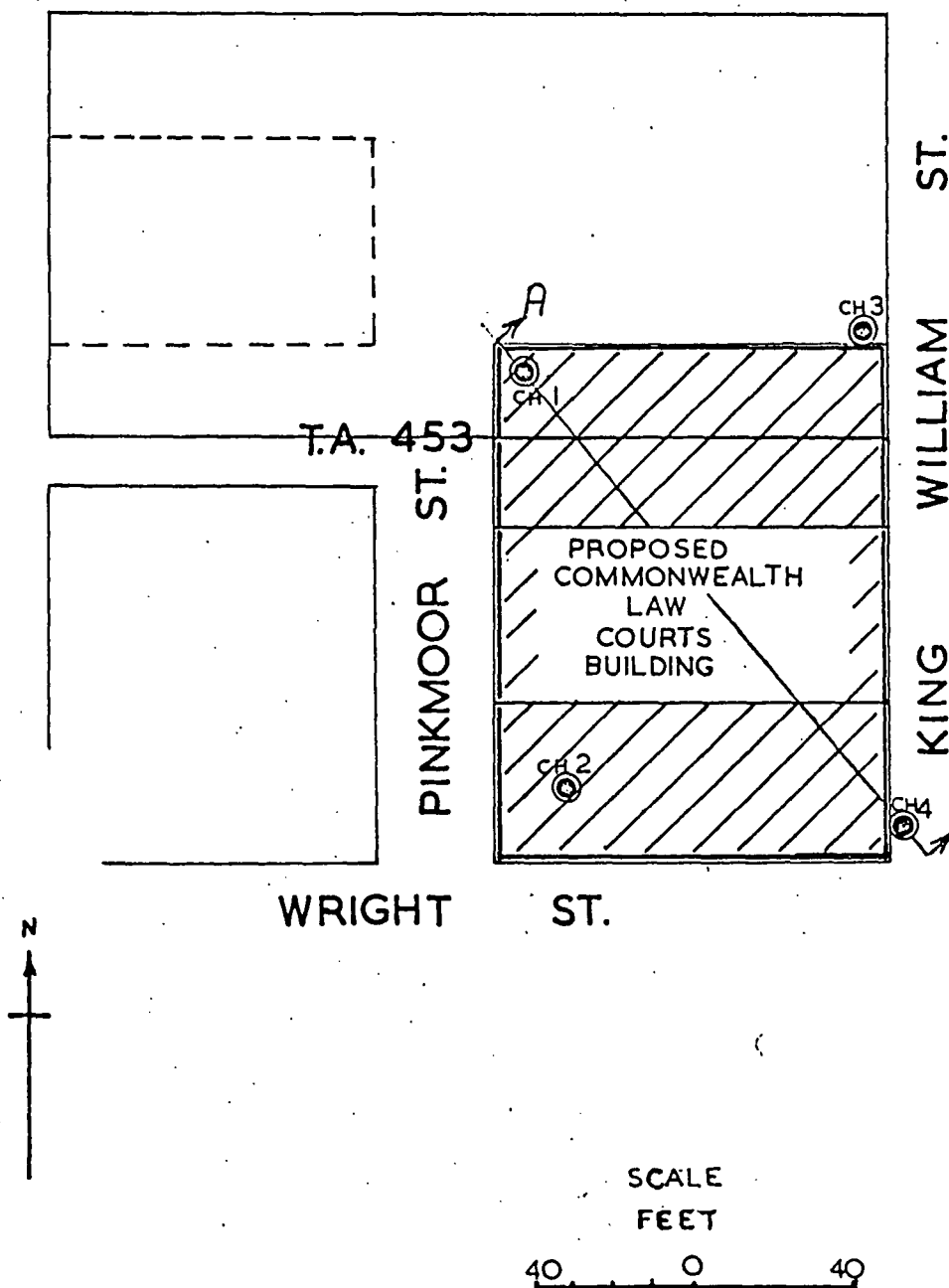
Joanne H. Fryar

JHF:SMA
12.3.1968

J.H. FRYAR
GEOLOGIST
ENGINEERING GEOLOGY SECTION

REFERENCES

1. STEEL, R.D. 1964. Adelaide Central Market Reconstruction Geological Investigations, Progress Report No.1, S.A. Dept. of Mines Rept. Bk. No. 59/53.
2. ALLCHURCH, P.D. 1966. Proposed Communications Building Geological Investigations, Progress Report No.1, S.A. Dept. of Mines, Rept. Bk. No. 62/31.
3. ALLCHURCH, P.D. Gilgai Structures in "mottled clays" beneath the City of Adelaide. Geol. Survey S. Aust. Quart. Geol. Notes No. 14.
4. ALLCHURCH, P.D. Karst Topography of the Hallett Cove Sandstone in the Adelaide City Area. Geol. Surv. S.Aust. Quart. Geol. Notes No. 24.
5. AMERICAN STANDARD BUILDING CODE, Requirements for Excavations and Foundations, Appendix, p. 18, clause 2.1 A.S.C.E. Manuals of Engineering Practise, No. 32. (A.S.A. Document A56.1-1952).
6. PROF. SKEMPTON AND LA ROCHELLE. The Bradwell Slip; A short-term failure in London Clay. Geotechnique Vol. XV No. 3 September, 1965.



For Section A-A' see Dwg. No. 68-192

FIG 1

DEPARTMENT OF MINES — SOUTH AUSTRALIA

ENG. GEOL. SECTION	Drn J.H.F.	COMMONWEALTH LAW COURTS BUILDING LOCALITY PLAN AND LOCATION OF DRILL HOLES	SCALE: AS SHOWN
	Tcd. R.T.		S 6416
	Ckd. W.H.F.		Ha 6
	Exd. D.I.		DATE: 5 March '68

SERIAL NO 303/75

LOCATION *WELLINGTON SQUARE (S.E. Corner)*

CO-ORDS

Detung *A.H.D.*

WATER LEVELS	MOISTURE CONTENT	CONSISTENCY (Clays)	COMPACTNESS (Silt)	RELATIVE DENSITY (Sand)	TYPE OF SAMPLE	* These values refer to clay soils only and provide an indication of their consistency.	
 Water level 'date'	H -- Humid	VS -- Very Soft	LS -- Loose	Vt -- Very Loose	OPEN TUBE	ENGINEERING GEOLOGY SECTION	
	D -- Damp	S -- Soft	MC -- Moderately Compact	L -- Loose	 A Shoe	DRILL NO. 23 TYPE DM 500 DRILLER A. Sturak START 15.7.74	LOGGED BY R.S.R. DATE 15.7.74 TRACED D.W.W.
	M -- Moist	F -- Firm		MD -- Medium Dense	 D Shoe		
	W -- Wet	St -- Stiff	C -- Compact		SEALED TUBE WITH NUMBER		
	S -- Saturated	V St -- Very Stiff	VC -- Very Compact	D -- Dense	 A Shoe		

LOG OF CABLE TOOL HOLE

HOLE NO. C.H.B

SERIAL NO 303/75

PROJECT ADELAIDE SUBWAY STUDY

FEATURE RECONNAISSANCE

LOCATION WELLINGTON SQUARE (S.E. Corner)

SEC Res. HD YATALA

CO-ORDS

El Collar

El Surface AG

Datum A.H.D.

GEOLOGICAL NOTES AND CLASSIFICATION	EL.	DEPTH m	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	MOISTURE CONTENT	CONSISTENCY	COMPACTNESS	FIELD TEST DATA
										blows PER 30 cm
Carisbrooke Sand.				CH						34(12.12.11)
				SC	Sand excess clay fines. Fine to medium sand. Red brown & yellow colour.					26(9.10.7)
				SM	Less clay. Silty Brown to off-white colour.					25(7.10.7)
					Mainly fine sand to silt.					25(8.8.9)
										27(8.10.9)
										27(9.9.9)
										18(7.6.5)
										37(8.16.13)
										33(5.12.16)
										75(28.32.15)
Blanche Point Marl.				GP /SM	Poorly graded gravel with fine to medium grained silty sand. Gravel fragments 2-5 cm. Colour red brown to light brown colour.					40(14.14.12)
										57(15.20.16)
				SC	Sand excess clay fines. Green colour. Grey clay/silt with some sand.					57(26.13.18)

WATER LEVELS

MOISTURE CONTENT

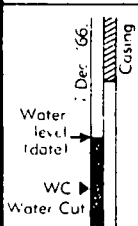
CONSISTENCY (Clays)

COMPACTNESS (Silt)

RELATIVE DENSITY (Sands)

TYPE OF SAMPLE

* These values refer to clay soils only, and provide an indication of their consistency.



H - Humid
D - Damp
M - Moist
W - Wet
S - Saturated
LL - Liquid Limit
PL - Plastic Limit

VS - Very Soft
S - Soft
F - Firm
St - Stiff
V St - Very Stiff
H - Hard

Ls - Loose
MC - Moderately Compact
C - Compact
VC - Very Compact

VL - Very Loose
L - Loose
MD - Medium Dense
D - Dense
VD - Very Dense

OPEN TUBE
A Shoe
D Shoe
SEALED TUBE WITH NUMBER
STANDARD PENETRATION TESTS
9 (2.3.4)
Total blows for 0.3m (in 0.1m increments)

ENGINEERING GEOLOGY SECTION

DRILL NO 23
TYPE D.M. 500
DRILLER A. Sturak
START
FINISH

LOGGED BY R.S.R.
DATE
TRACED D.W.W.
CHECKED A.F.

SHEET 2 OF 4. DRG NO S11030a Ha5

LOG OF CABLE TOOL HOLE

HOLE NO. C.H.B

SERIAL NO 303/75

PROJECT ADELAIDE SUBWAY STUDY

FEATURE RECONNAISSANCE

LOCATION WELLINGTON SQUARE (S.E. Corner)

SEC Res. HD YATALA

CO-ORDS

El Collar m

El Surface 46 m

Datum A.H.D

GEOLOGICAL NOTES AND CLASSIFICATION	EL	DEPTH m	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	Casing MOISTURE CONTENT	Consistency	Compr. Density	FIELD TEST DATA	
										BLOWS PER 30 cm	SG. TEST PENET. (DIAMETER)
										4 8 16 32 64	
Undifferentiated Tortachilla L/s and Blanche Point Transitional Marl.		21		ML /CL	Material mostly has soil consistency.						72(26.24.22)
		22									
		23									
		24			Light yellowish colour. Material mostly soil properties some harder cemented bands.						
		25		ML /CL	Light yellow brown with some glauconite. Some harder bands						
South Maslin Sands Upper Weathered Horizons?		27		CL	Sandy clay, low plasticity. Bright maroon colour.						
		28		SC	Sand excess clay fines Fine sand. Mottled orange, maroon & grey colour. Some hard Fe cemented lumps						
		29			Fine sand. Less clay. Grey green colour.						2(6.8.7)
		30									

WATER LEVELS	MOISTURE CONTENT	CONSISTENCY (Clays)	COMPACTNESS (Sils)	RELATIVE DENSITY (Sands)	TYPE OF SAMPLE	* These values refer to clay soils only, and provide an indication of their consistency.	
	H --- Humid D --- Damp M --- Moist W --- Wet S --- Saturated LL --- Liquid Limit PL --- Plastic Limit	VS --- Very Soft S --- Soft F --- Firm St --- Stiff V St --- Very Stiff H --- Hard	Ls --- Loose MC --- Moderately Compact C --- Compact VC --- Very Compact	VL --- Very Loose L --- Loose MD --- Medium Dense D --- Dense VD --- Very Dense	OPEN TUBE A Shce D Shce SEALED TUBE WITH NUMBER STANDARD PENETRATION TESTS Total blows for 0.3m	ENGINEERING GEOLOGY SECTION	
Water level (date) WC Water Cut	DRILL NO 23 TYPE DM 300 DRILLER A. Sturak START FINISH					LOGGED BY R.S.R. DATE TRACED D.W.W. CHECKED A.F.	
	SHEET 3 OF 4					DRG NO S110306 Ha5	

LOG OF CABLE TOOL HOLE

HOLE NO. *C.H.8*SERIAL NO *303/75*PROJECT *ADELAIDE SUBWAY STUDY*FEATURE *RECONNAISSANCE*LOCATION *WELLINGTON SQUARE (S.E. Corner)*SEC *Res.* HD *YATALA*

CO-ORDS

El. Collar m

El. Surface *46* mDatum *A.H.D.*

GEOLOGICAL NOTES AND CLASSIFICATION	EI	DEPTH m	GRAPHIC LOG	GROUP SYMBOL	SOIL DESCRIPTION GROUP NAME Unified Soil Classification, U.S.B.R. Earth Manual 2nd Edition 1966	WATER LEVEL	GROUND LEVEL	MOISTURE CONTENT	CONSISTENCY	COMPACTNESS	FIELD TEST DATA	
											BLOWS PER 30 cm	TEST PE. 150mm Ea Unit *
<i>South Maslin Sands</i> <i>Grey to Black Fine sands</i>				<i>SC</i>	<i>Dark grey colour.</i>						<i>21(6.07)</i>	
		<i>31</i>									<i>22(7.7.0)</i>	
		<i>32</i>									<i>15(3.46)</i>	
					<i>Black colour. Fine sand.</i>						<i>31(8.112)</i>	
		<i>33</i>									<i>29(10.8.11)</i>	
		<i>34</i>									<i>27(7.11.9)</i>	
											<i>33(13.9.11)</i>	
		<i>35</i>			<i>END OF HOLE 34.80m</i>							

WATER LEVELS	MOISTURE CONTENT	CONSISTENCY (Clays)	COMPACTNESS (Sils)	RELATIVE DENSITY (Sands)	TYPE OF SAMPLE	* These values refer to clay soils only and provide an indication of their consistency.	
	H - Humid D - Damp M - Moist W - Wet S - Saturated LL - Liquid Limit PL - Plastic Limit	VS - Very Soft S - Soft F - Firm St - Stiff V St - Very Stiff H - Hard	Ls - Loose MC - Moderately Compact C - Compact VC - Very Compact	VL - Very Loose L - Loose MD - Medium Dense D - Dense VD - Very Dense	OPEN TUBE SEALED TUBE WITH NUMBER STANDARD PENETRATION TESTS Total blows for 0.3m (in 6.3m increments)	ENGINEERING GEOLOGY SECTION	
DRILL NO <i>23</i> TYPE <i>DM.500</i> DRILLER <i>A. Sturak</i> START FINISH <i>24.7.74</i>						LOGGED BY <i>R.S.R.</i>	
SHEET <i>4</i> OF <i>4</i>						CHECKED <i>A.F.</i>	DRG NO <i>S11030c Ha5</i>

LAW COURTS BUILDING

TOWN ACRE 453.

FEATURE FOUNDATIONS

CITY OF ADELAIDE

LOCATION CRN. KING WILLIAM AND WRIGHT STS.

COORDS.

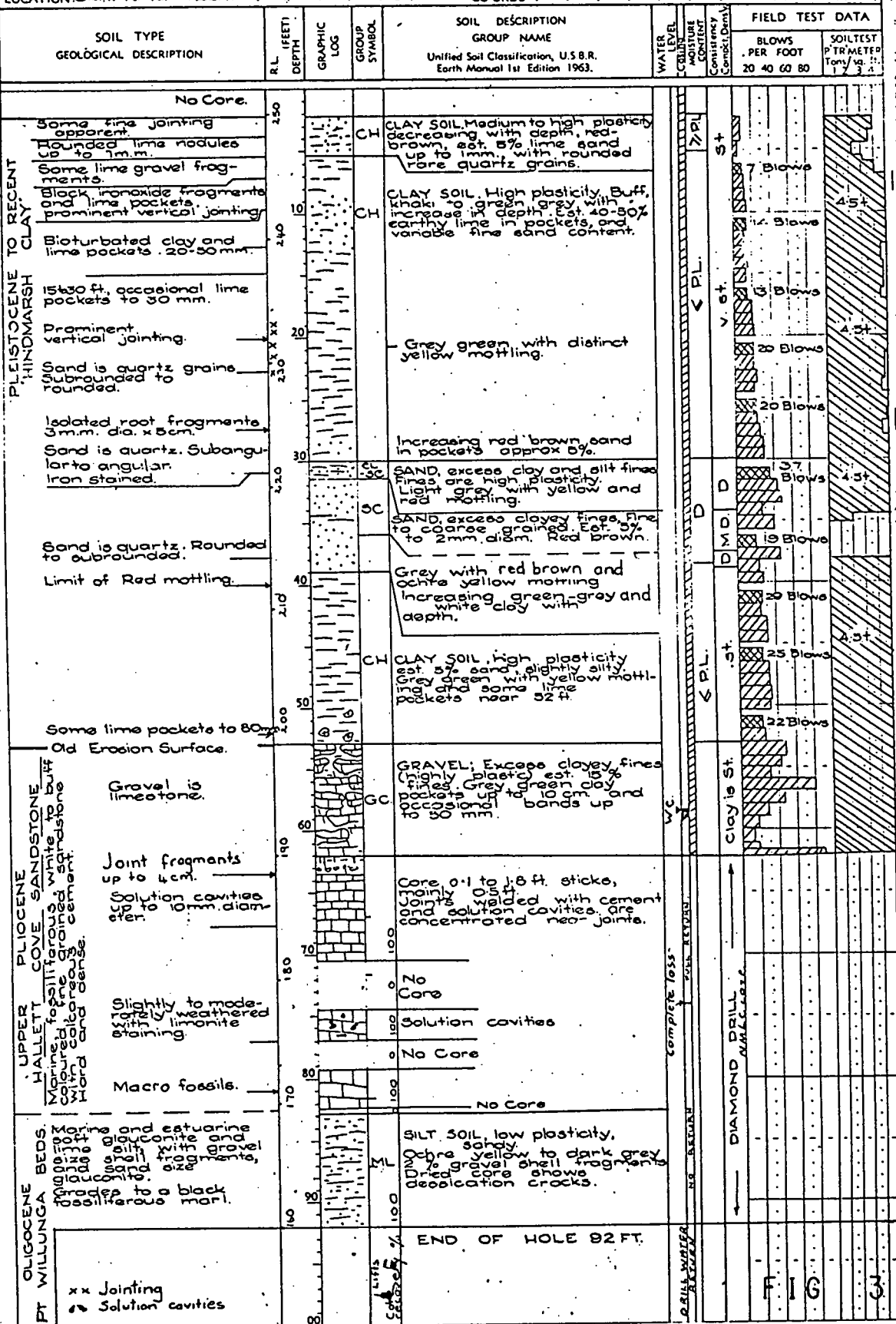


FIG 3

TYPE OF SAMPLE	CONSISTENCY	COMPACTNESS	RELATIVE DENSITY	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION	
A shoe (SA)	VS. - Very Soft	LS - Loose	VL - Very Loose	H - Humid	DRILL No. 8	LOGGED BY J. FRYAR
D - (SD)	S - Soft	MC - Moderately Compact	L - Loose	D - Damp	TYPE RUSTON	T. MOUNT
E - (SE)	SF - Soft to Firm	C - Compact	MD - Medium Dense	M - Moist	DRILLER DR. PHILIPS	DATE 10.1.68
G - (SG)	F - Firm	VC - Very Compact	D - Dense	W - Wet	START 22-12-67	TRACED L.M.D.
Sealed Tube:	F St. - Firm to Stiff		VD - Very Dense	S - Saturated	FINISH 4-1-68	CHECKED L.W.
A shoe - SAL	St. - Stiff			LL - Liquid Limit		
Standard Penetration Test - SPT	V.St. - Very Stiff			PL - Plastic Limit		
	H - Hard				SHEET 1 OF 1	DRG No. 56384

LOG OF CABLE TOOL HOLE

PROJECT COMMONWEALTH.

LAW COURTS BUILDING

TOWNACRE 453.

FEATURE FOUNDATIONS

CITY OF ADELAIDE

LOCATION CORN. KING WILLIAM AND WRIGHT STS.

COORDS.

HOLE NO. CH. 2.

SERIAL No.

R.L. Surface, 252.2 FT.

R.L. Collar FT.

Datum, M.S.L.

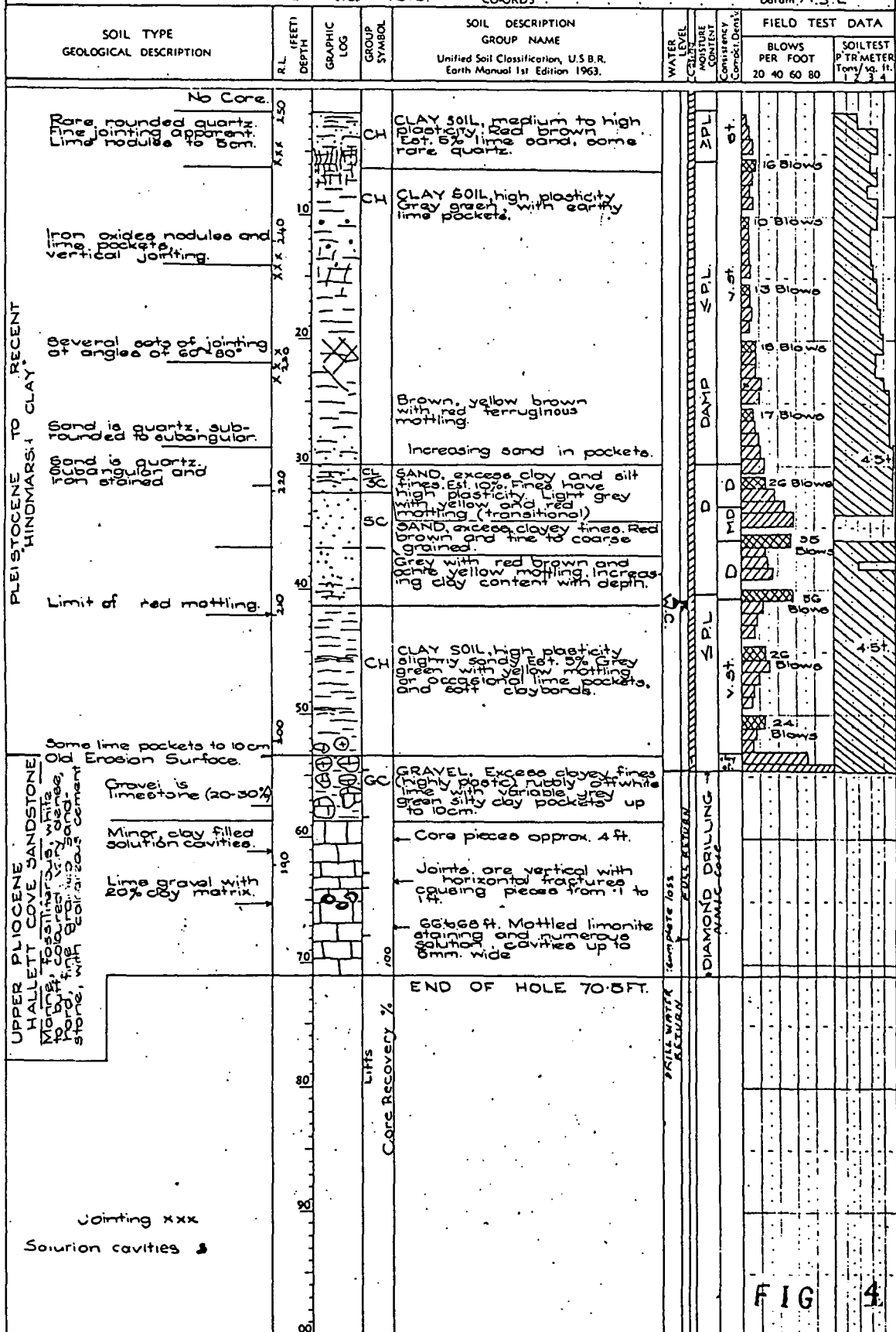


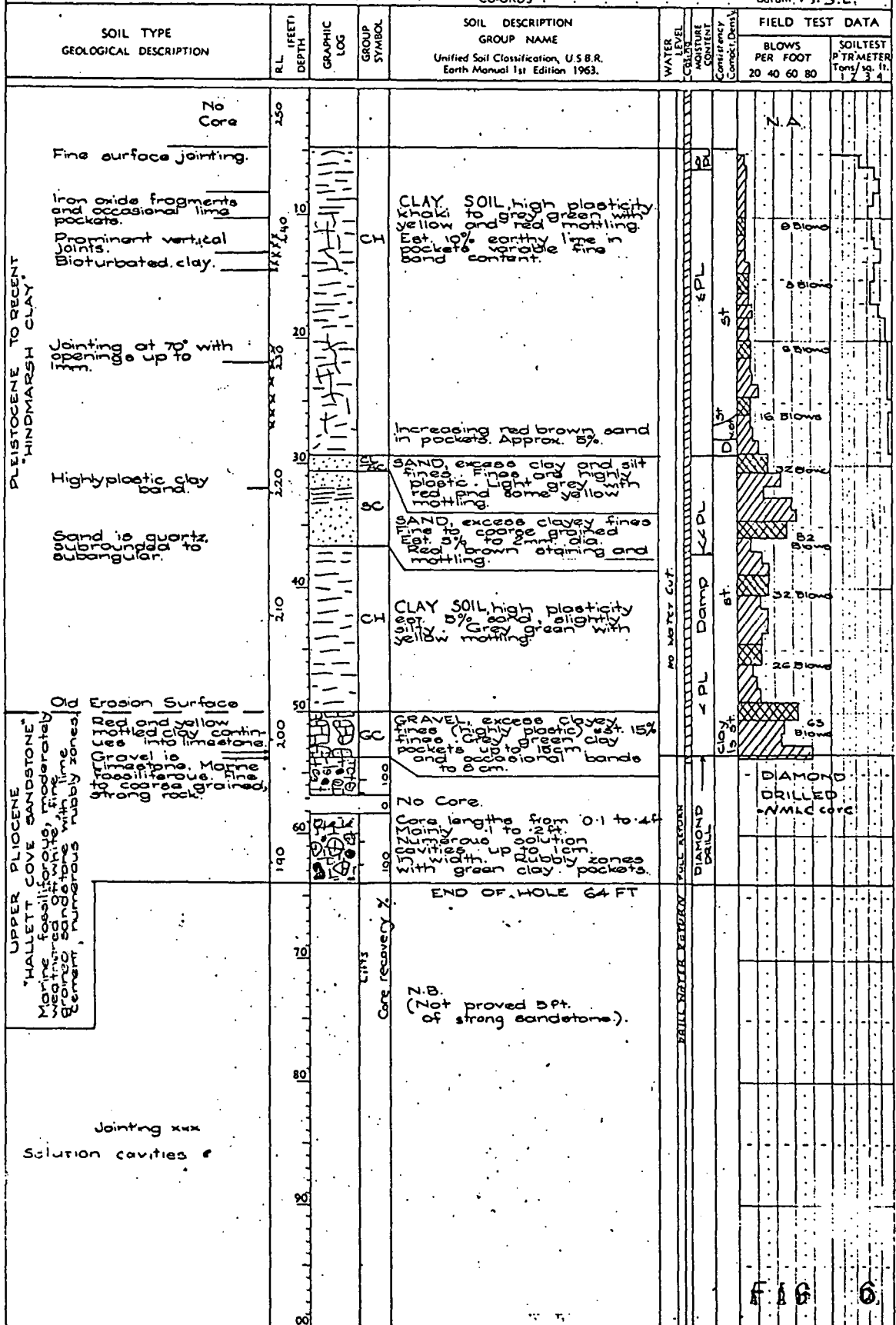
FIG 4

PROJECT COMMONWEALTH
LAW COURTS.
FEATURE FOUNDATION
LOCATION. ADELAIDE

LOG OF CABLE TOOL HOLE

TOWN ACRE 453.
CITY OF ADELAIDE.
CO-ORDS.

HOLE NO CH 4.
SERIAL No.
R.L. Surface, 252.3 FT.
R.L. Collar FT.
Datum, M. S.L.



TYPE OF SAMPLE	CONSISTENCY	COMPACTNESS	RELATIVE DENSITY	MOISTURE CONTENT	ENGINEERING GEOLOGY SECTION
A shoe (SA) D = (SD) E = (SE) F = (SF)	VS - Very Soft S - Soft SF - Soft to Firm F - Firm F to Firm to Stiff	LS - Loose MC - Moderately Compact C - Compact VC - Very Compact	VL - Very Loose L - Loose MD - Medium Dense D - Dense D - Dense	H - Humid D - Damp M - Moist W - Wet S - Saturated SS - Super Saturated	DRILL No. 8 TYPE BULLWIP MILLER START 10 JAN 68 FINISH 15 JAN 68 LOGGED BY J. FRYAR DATE 28 JAN 68 INCHES 100