

Section

Rept. Bk. No. 59/143
G.S. No. 3037
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DEPARTMENT OF MINES
SOUTH AUSTRALIA

GEOLOGICAL SURVEY
ENGINEERING & SOILS GEOLOGY SECTION

MIDDLE RIVER DAM - KANGAROO ISLAND
FEASIBILITY STAGE GEOLOGICAL INVESTIGATIONS
PRELIMINARY REPORT - SUMMARY OF RESULTS
~~DECEMBER~~ 1964

by
D. H. Stapledon
Senior Geologist

and
S. Robson
Geologist

64-14

DEPARTMENT OF MINES
SOUTH AUSTRALIA

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8th December, 1964.

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DEPARTMENT OF MINES
SOUTH AUSTRALIA

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INTRODUCTION

The site for the proposed Middle River Dam is located about 4 miles upstream from the mouth of Middle River, in Section 79, Hundred of Duncan, Kangaroo Island. It is about 30 miles east of Kingscote, as shown on Figure 1.

At the site it is proposed to build a dam about 70 feet high to store water for the Kingscote - Parndana district.

Field geological studies for site selection and feasibility have been completed, and a detailed report is in progress. The work has included detailed geological mapping at the dam site and at possible quarry sites, and five diamond drill holes put down along a tentative axis line at the dam site.

Design Branch of Engineering and Water Supply Department are preparing preliminary estimates for pre-stressed gravity, earth and rock fill, and faced rock fill dams at the site. Mr. R. Good, Designing Engineer, Dams, has requested a summary of the results of the geological studies, for use in preparation of the comparative estimates for the three alternative schemes. The foundation requirements for each of these types of structure have been discussed with Mr. Good.

This report summarises the main results and conclusions reached. The detailed factual data upon which the conclusions are based will be in the main detailed report.

DAM SITE GEOLOGY

Quality of Foundation Rock

The site is in massive quartzite or greywacke, which is a

compact, durable, rock, of very high strength and elastic modulus. There are several nearly vertical planar zones up to 2 ft. thick in which the rock is thinly bedded and fissile. It is considered that when confined, this rock would not be appreciably weaker than the massive rock.

Quality of Foundation Rock Mass

The rock mass is intersected by three sets of joints (Fig. 2) which tend to divide it into blocks of approximately rhomboidal shape. Near the surface the rock itself is partly weathered, and in addition the joints have opened up as a result of weathering and become partly filled with soft materials including weathered rock, clay, organic soil, and roots of vegetation. Below the red line on Figure 2, the majority of joints are tightly closed, in many cases cemented by strong material such as quartz and calcite.

Near-horizontal joints occur at intervals of 1 to 5 feet in the relatively steep right abutment, and together with joints dipping about 50° downslope, appear to have allowed mechanical loosening of the rock mass to depths of 15 to 20 feet below surface. On the right bank the red line on Figure 2 does not go beneath the limit of mechanical loosening as indicated by the exploration; it is however below the levels of the last soil or clay filled joints recorded, and it is considered that the material below can be consolidated by cement grouting. The stepped shape of the line is in part diagrammatic - it is intended to show the shape which can tend to form on excavation with the existing natural joint pattern.

No wide zones of sheared or crushed rock were located in the exploration. It is possible that some relatively narrow zones of such material occur in the areas between the exploratory holes.

A steeply dipping zone of weathered rock exposed in the trench on the left bank was intersected in the uppermost 10 to 15 feet of Hole 4. It appears to be a localised zone about 10

feet wide in which joints are spaced 0.5 feet or less, with the result that the effects of mechanical and chemical weathering probably extend locally deeper than in the average rock.

Permeability of Foundation

All of the holes were tested for permeability by the single packer method. Except in Hole 1 at the top of the right abutment, 10 and 20-foot sections of the rock below the main weathered zone were almost watertight when tested at pressures of up to 40 lbs./sq.in.

Slope Stability

There are no signs of ancient or recent slope instability in the dam site area.

The orientation of joints on the right abutment favours sliding of rhomboidal blocks during excavation, but with cutting depths of less than 20 feet this should not create serious problems.

NOTES ON GEOLOGICAL FEATURES AFFECTING THE ALTERNATIVE TYPES OF STRUCTURES

The following notes have been prepared after discussion of the engineering requirements with officers of Engineering and Water Supply Department.

Concrete Gravity Dam (Pre-stressed)

Foundation

The red line on Figure 2 is a suggested excavation depth for this type of structure, based on the results of the five diamond drill holes and permeability tests. Except for the uppermost part, above about R.L. 590 on both abutments, the rock below this line is assumed to be mainly fresh, in general tightly jointed, and to contain no seams or zones of weak material liable to consolidate under load or fail by sliding. On the right bank the rock below the line probably contains some partly open joints but it is considered that this rock can be consolidated by cement grouting. The red line is generally several feet

below the main weathered zone, to allow the structure to be keyed into suitably sound rock.

Drilling of Holes for Anchors

Drilling of the vertical holes for pre-stress cables may require a careful approach because of the very hard and abrasive nature of the quartzite, and the presence of several near-vertical zones of softer, fissile rock. Two possible problems are recognised:

--- slight defections of the holes from vertical.

It is believed that there is more chance of deflection with percussion drilling than diamond drilling, and further, that there is more chance of deflection with a solid (non-coring) diamond bit, than with a coring bit. In any case, if exact verticality of holes is critical, it may be advisable to locate holes away from the contacts between massive and fissile rock.

--- high diamond usage if diamond drilling is used.

During the exploratory diamond drilling at the site, bits usually became polished after several feet only of drilling. Once polished their penetration rate became very slow and it was not economical to use them further at this site. The following figures show diamond usage and costs for the exploratory drilling.

Total depth drilled	243 feet
Initial Diamond Cost	£1403
Diamond Loss (usage)	£551
Percentage Diamond Loss	39.3%
Diamond cost per foot	£2-5-4.

It is considered that the use of a soluble oil lubricant in the drilling water may increase penetration rates and reduce diamond usage in this type of rock.

Spillway

The rock in the stream bed downstream from the proposed structure is of sufficient quality that no protective measures should be required. It is also considered likely that very little protective work would be required if a channel

spillway was constructed on the left bank, as indicated on Figure 2.

Coarse Aggregate Sources

Quartzite which appears suitable for crushing for coarse aggregate is exposed in a 30 to 50 foot high face about $\frac{1}{2}$ mile downstream from the site (Fig. 3). The rock is generally massive or poorly bedded, and except in some localised thinly bedded zones, should provide coarse aggregate up to three inches in size.

Preliminary estimates on the basis of the surface geological mapping and sketch sections on Figure 5 indicate that at least 10,000 c. yds. of material could be obtained from this source.

The Bay of Shoals Quarry at Kingscote (Fig. 1) is in sheared columnar basalt. The rock is divided into thin platy fragments separated by limonite or soil-coated joints. A typical fragment would be 5 x 4 x $\frac{3}{4}$ inch. The rock in the core of each fragment is generally fresh, and adjacent to the joint faces it is slightly, and in some cases moderately weathered. It is understood that the rock is crushed and used for $\frac{3}{4}$ inch aggregate. However, it is considered unlikely that any larger size than $\frac{3}{4}$ inch could be obtained from this source.

Fine Aggregate Sources

Natural sand is worked intermittently for fine aggregate at the MacGillivray pit about 12 miles south-east of Kingscote (Fig. 1). The sand occurs in one of a series of elongated dunes. The depth of usable material appears to range from 4 to 6 feet and it is considered likely that the quantity required (approx. 4000 c.yds.) could be obtained from this source.

Quartzite from the proposed quarry site downstream from Middle River Dam may when crushed produce fine aggregate suitable for use in concrete, either alone, or else blended with natural sand.

Small alluvial terraces in the lower reaches of Middle River are underlain mostly by silts with some fine sandy silts rich in organic material. It is considered that these deposits contain no material suitable for use as concrete aggregate.

Earth and Rockfill Dam

Foundation

It is considered that the impervious core zone of such a dam could be founded in a trench excavated down to a little above the red line on Figure 2. On the left bank this could be 3 to 4 feet above the line, and on the right bank almost 6 feet above. The requirement assumed is that the rock at these levels will be sufficiently compact, or can be made sufficiently compact by local dental treatment, to ensure that fine material from the core cannot be carried by percolating water into open joints or voids in the foundation. It is also assumed that a grout curtain will have to be constructed from a line of holes located roughly in the centre of the trench.

The filter zone should require a shallower excavation than the impervious core, and a suitable foundation for the rockfill will probably be found in most places within a foot of the natural surface. It is assumed that only the vegetation and organic topsoil will need to be removed before placement of the rockfill.

Rockfill Sources

It is considered that adequate quantities of suitable quartzite exist along the sides of Middle River gorge, within $\frac{1}{2}$ mile of the dam site. An extension of proposed concrete aggregate quarry, shown in Figure 3, may prove to be best source.

Filter Material Sources

It is assumed that either crushed quartzite from the dam site area, or crushed basalt from the Bay of Shoals quarry near Kingscote, would prove suitable for use as filter material. No natural gravel deposit suitable for consideration as a filter

source has been located.

Impervious Core Material Sources

Completely weathered and lateritized bedrock has been proven to depths of up to 40 feet beneath the plateau surface about 2 miles from the site. This weathered rock consists of silty and sandy clays generally of low plasticity, often containing laterite gravel in the uppermost 5 feet. It is considered that this weathered rock would probably prove suitable for use as core material, and that deposits of similar material should be found within 1 mile of the site. The groundwater table often lies close (within 10 to 15 feet) to the ground surface in this area, and so it may be necessary to obtain the material from very shallow depths.

Spillway

It is assumed that a channel spillway would be constructed on the left bank. The results of drilling suggest that this should be in partly weathered quartzite for the first 4 to 5 feet below the surface, and mainly fresh rock, with localized weathered seams and zones, below that depth. Much of the material excavated should be usable as rockfill, and it should be possible to leave the channel largely unlined.

Faced Rockfill Dam

Foundation

It is assumed that this type of structure would require a cut-off trench at the upstream toe, excavated down to rock which can be rendered impervious by cement grouting. At this initial stage it could be assumed that the depths required would be 2 or 3 feet less than those for the gravity dam foundation, i.e., 2 or 3 feet above the red line on Figure 2.

The rockfill foundation requirements would be the same as for the earth and rockfill structure, and it is considered that a suitable foundation will probably be found within 1 foot of the natural surface in most places.

Rockfill Sources

These will be local quartzite, the same as for rockfill for an earth and rockfill structure, described above.

Spillway

A channel spillway located on the left bank, would be mainly in rock which could be used as rockfill, and it is considered likely that the channel could be largely unlined.

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ENGINEERING & SOILS GEOLOGY
SECTION

DHS&SR/AGK
8/12/64

DEPARTMENT OF MINES — SOUTH AUSTRALIA
GEOLOGICAL LOG OF DRILL HOLE

Hole No. 1

PROJECT MIDDLE RIVER DAM CO-ORDINATES

R.L. 598 FT

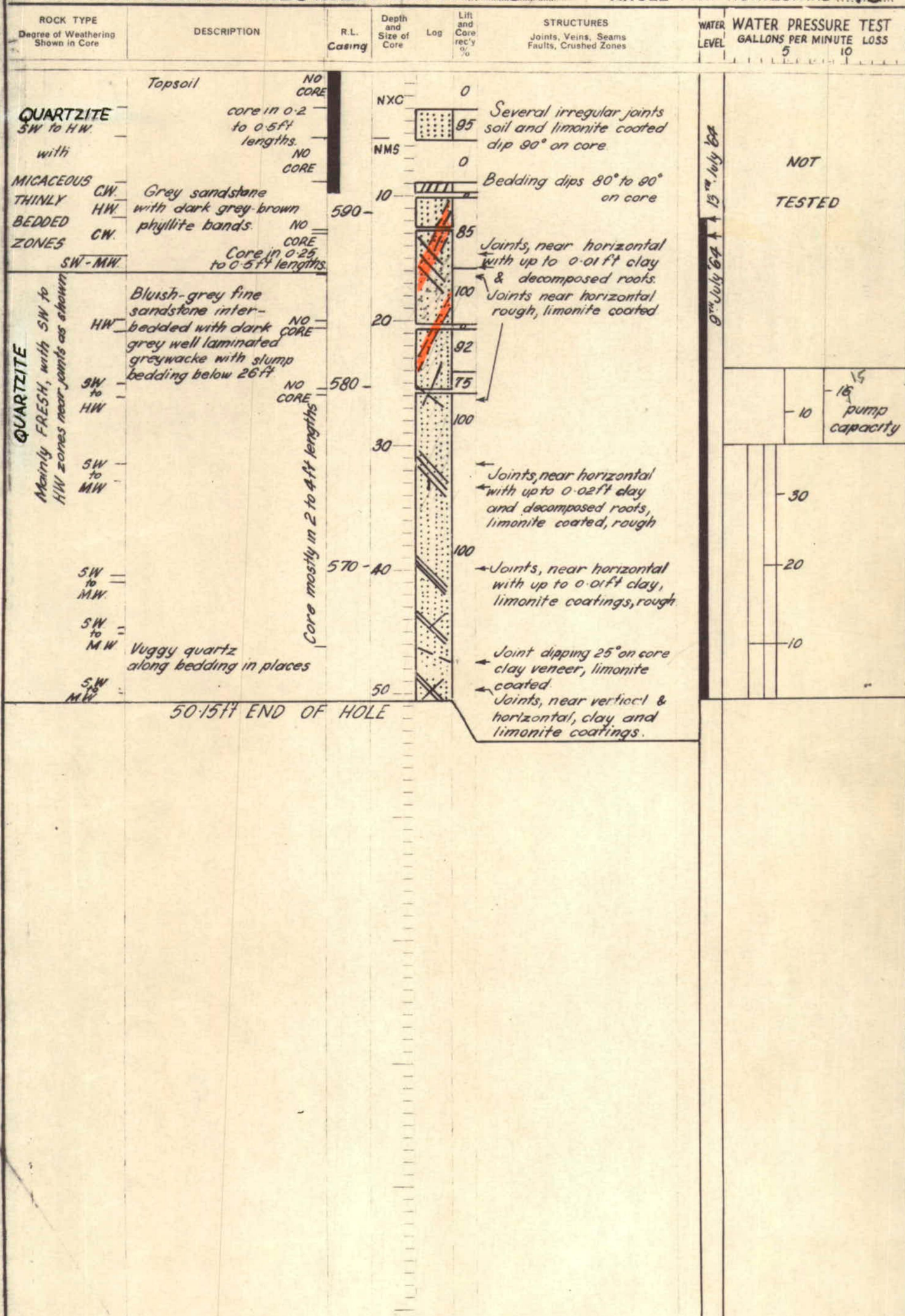
FEATURE FOUNDATION

HUNDRED DUNCAN

DIRECTION 267°

LOCATION UPPER RIGHT ABUTMENT SECTION 79

ANGLE FROM HORIZONTAL 45°



Drill No. 7
Type E 1000
Driller T. Briggs
Commenced 6th July '64
Completed 10th July '64

EXPLANATION
FR - Fresh
SW - Slightly weathered
MW - Moderately weathered
HW - Highly weathered

WATER LEVEL DATE

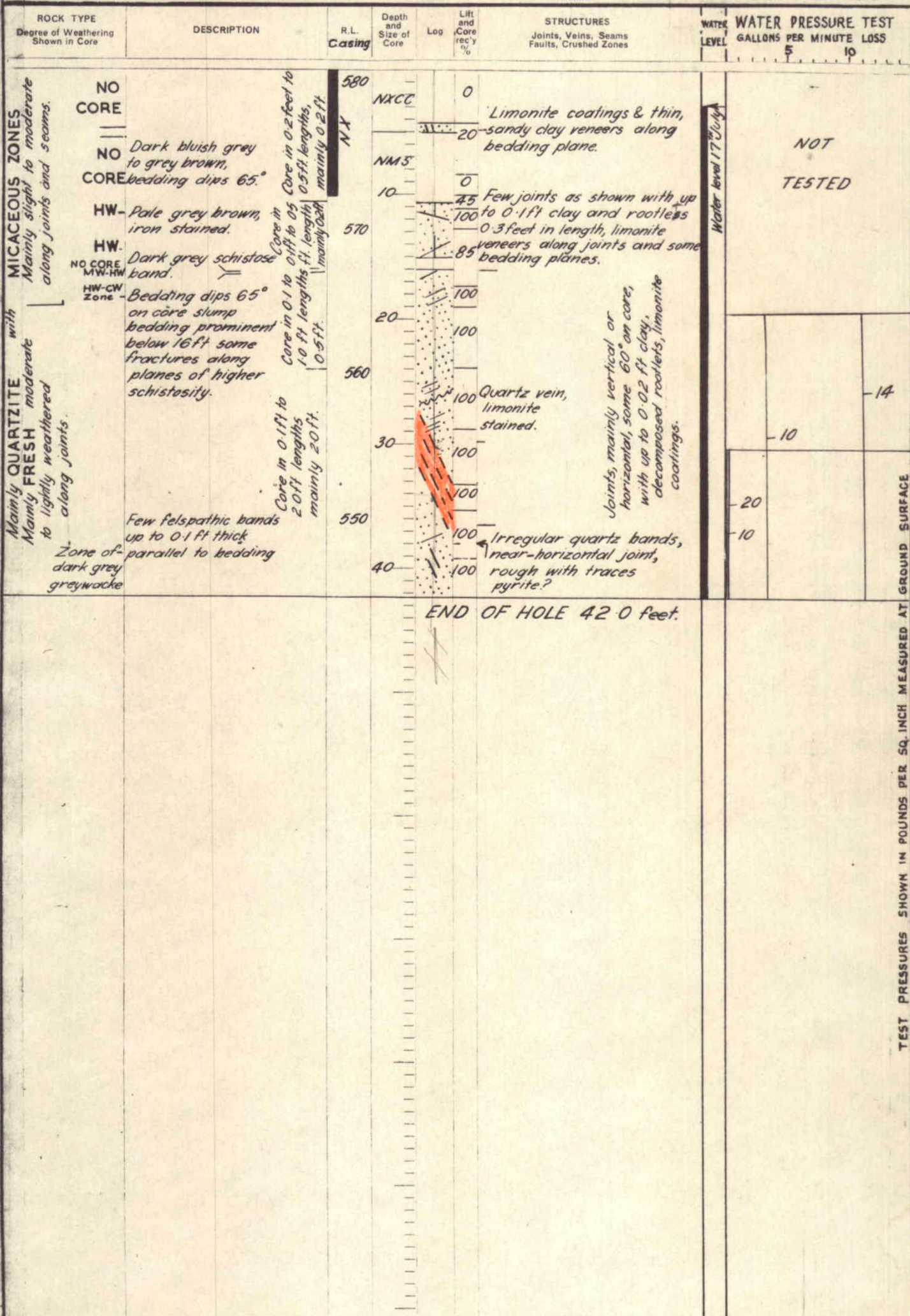
Logged S. Robson
Sheet No. 1
Drawn S. Robson
Checked
Submitted

Vert. Scale 1" = 10'
Sheet 1 of 1
53775
L1
250-7.60 8036

DEPARTMENT OF MINES — SOUTH AUSTRALIA
GEOLOGICAL LOG OF DRILL HOLE

Hole No. 2

PROJECT MIDDLE RIVER DAM CO-ORDINATES R.L. 581 FT
FEATURE FOUNDATION HUNDRED DUNCAN DIRECTION 215M °
LOCATION RIGHT ABUTMENT SECTION 179 ANGLE FROM HORIZONTAL 60 °



Drill No. 7
Type F100
Driller T. BRIGGS
Commenced 14 July '64
Completed 17 July '64

EXPLANATION

QUARTZITE
MICACEOUS THINLY BEDDED ZONES

WATER LEVEL DATE

Logged
Sheet No.
Drawn
Checked
Submitted

S.R.
S.R.

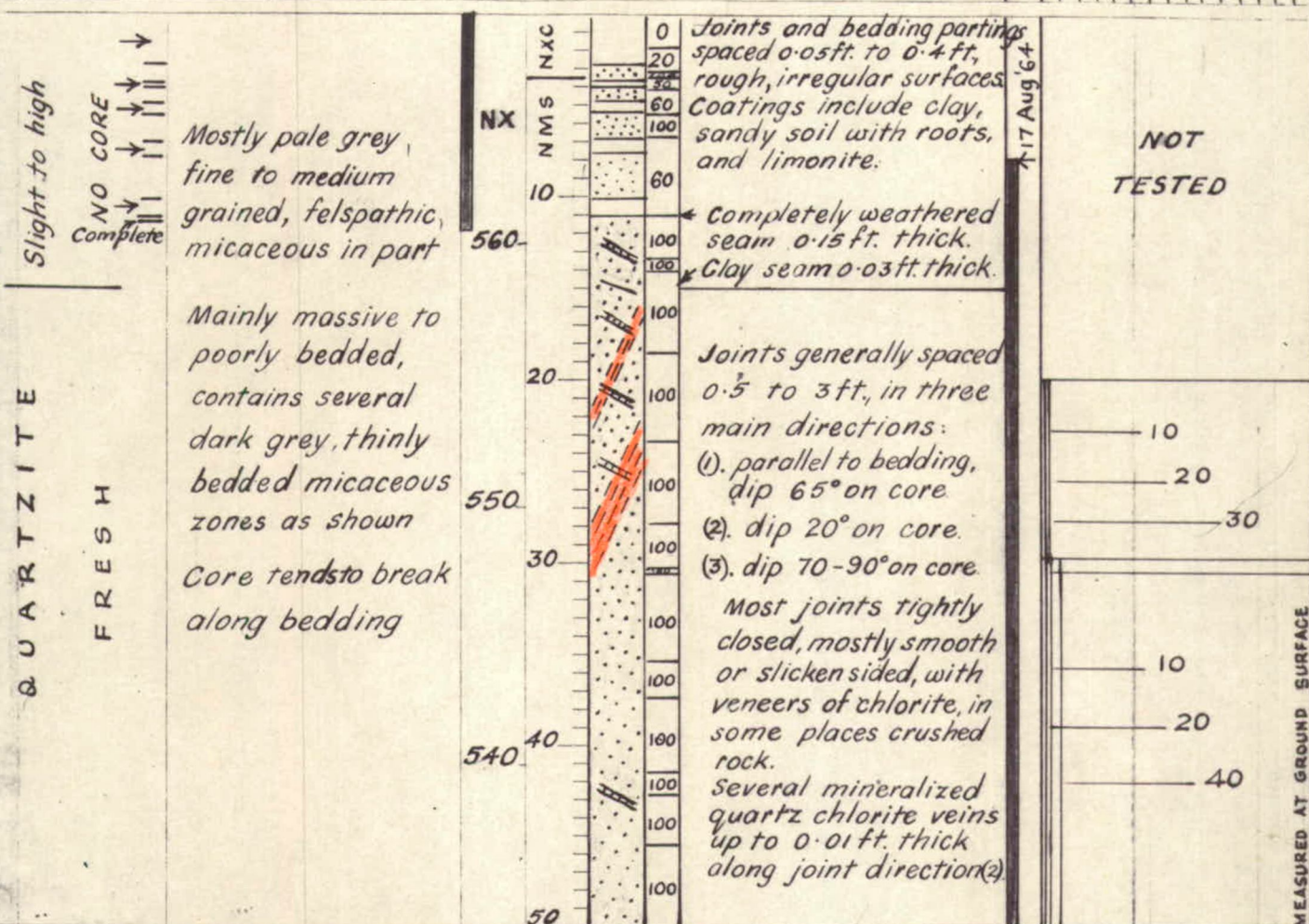
Vert. Scale 1"=10'
Sheet 1 of 1
53880
LI
250-7.60 8036

DEPARTMENT OF MINES — SOUTH AUSTRALIA
GEOLOGICAL LOG OF DRILL HOLE

Hole No. **4**

PROJECT **MIDDLE RIVER DAM** CO-ORDINATES **HUNDRED DUNCAN** R.L. **569** FT
 FEATURE **FOUNDATION** SECTION **79** DIRECTION **33M.** °
 LOCATION **LEFT ABUTMENT** ANGLE FROM HORIZONTAL **45** °

ROCK TYPE **QUARTZITE** DESCRIPTION **FRESH** R.L. **560** Depth and Size of Core **10** Log **100** Lift and Core rec'd **100** STRUCTURES **JOINTS AND BEDDING PARTINGS** WATER LEVEL **17** WATER PRESSURE TEST **NOT TESTED**
 Degree of Weathering Shown in Core **Complete** Gallons per minute loss **5**



Drill No. **D.M. 7** EXPLANATION **QUARTZITE** WATER LEVEL **1** DATE **26TH Aug '64**
 Type **E 1000** **QUARTZITE** **QUARTZ VEIN**
 Driller **T. Briggs** **MICACEOUS, THINLY BEDDED ZONE**
 Commenced **12TH Aug '64** **QUARTZ VEIN**
 Completed **18TH Aug '64**

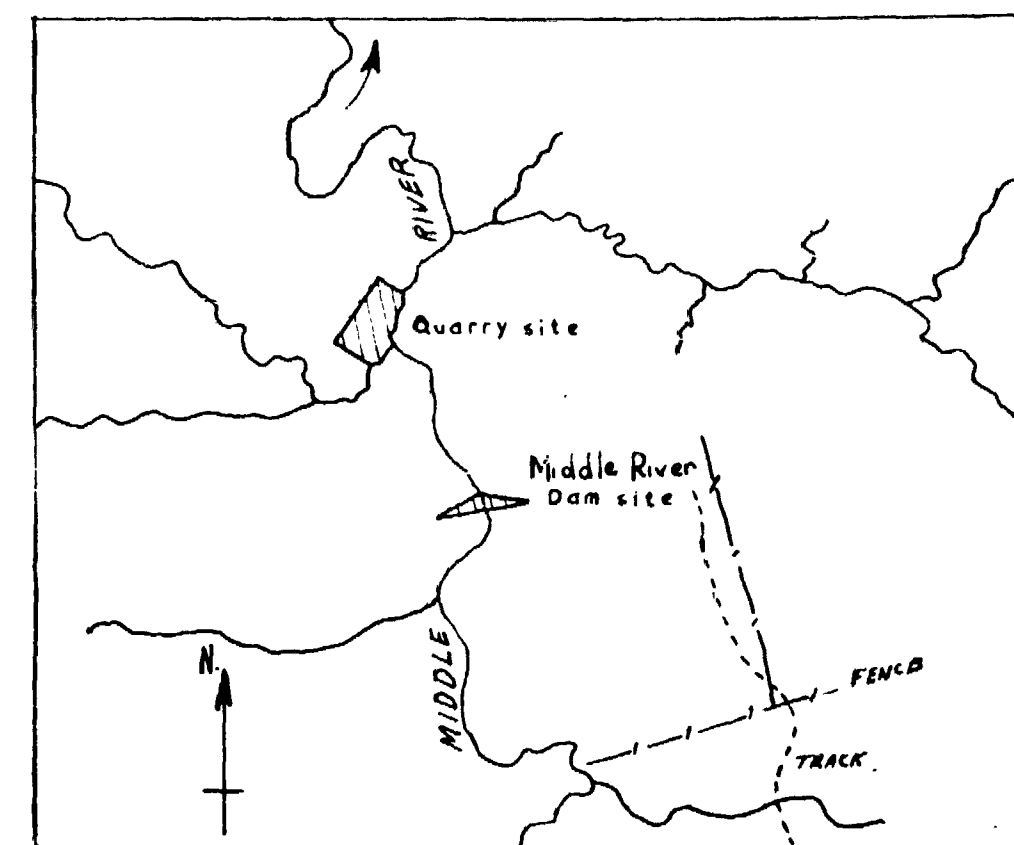
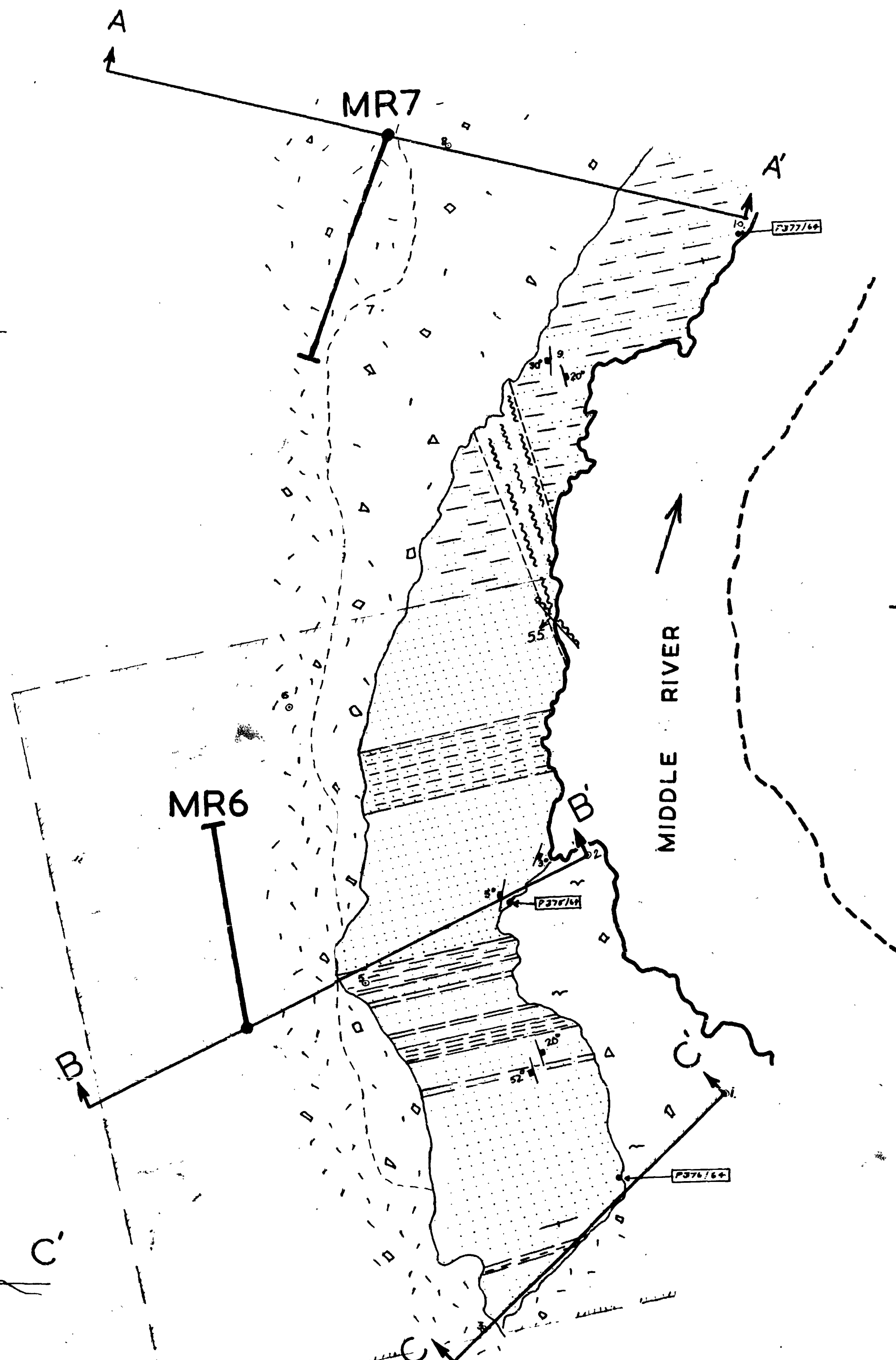
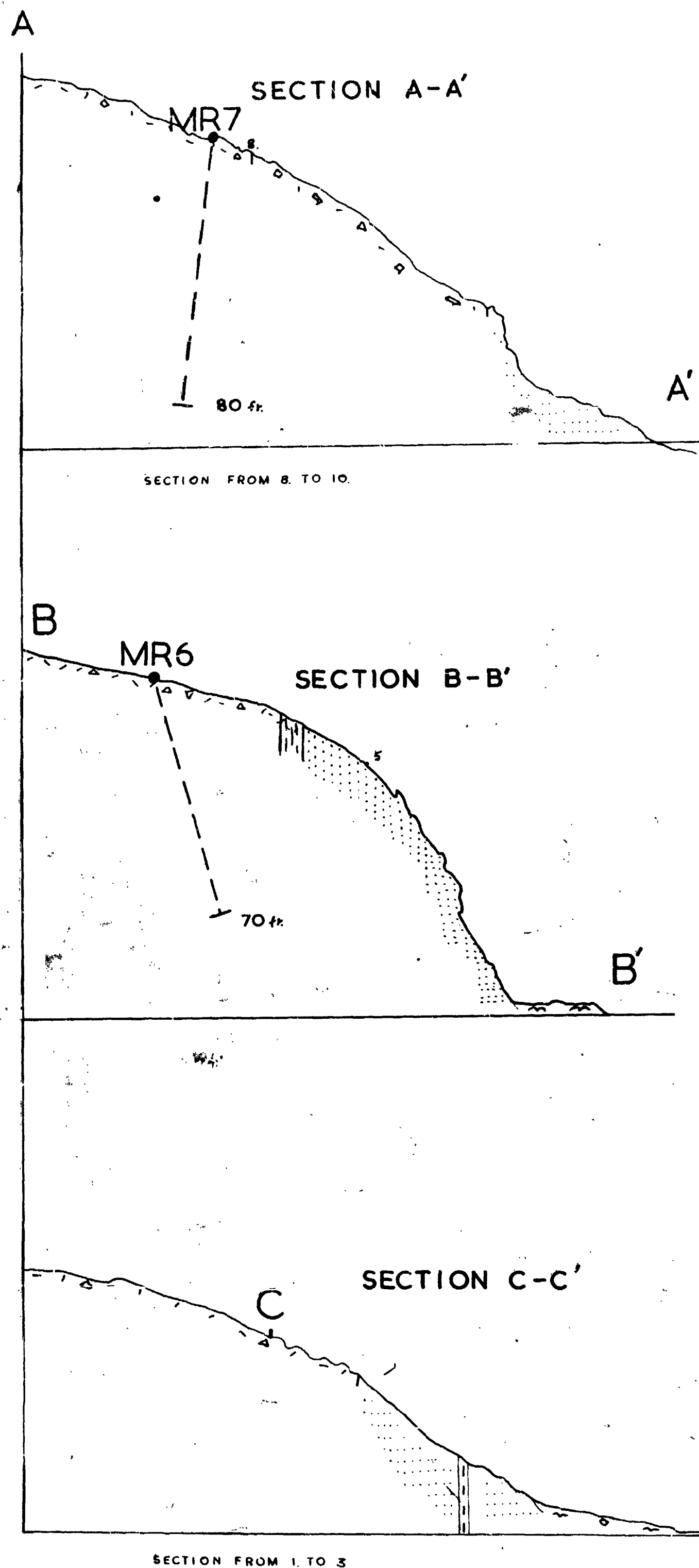
Logged **D.H.S.** S.R. / D.H.S. **26TH Aug '64**
 Drawn **S.R.**
 Checked **D.H.S.**
 Submitted

10 ft - 1 inch
 Vert. Scale
 Sheet **1** of **1**
 Drawing No. **5 3857**
 250-7.60 0036

PROJECT	MIDDLE RIVER DAM	CO-ORDINATES	R.L. 587 FT
FEATURE	FOUNDATION	HUNDRED DUNCAN	DIRECTION 216 M°
LOCATION	UPPER LEFT ABUTMENT	SECTION	ANGLE FROM HORIZONTAL 45°

TEST PRESSURES SHOWN IN POUNDS PER SQ. INCH MEASURED AT GROUND SURFACE

Vert. Scale $\frac{1}{2}'' = 10'$
Sheet 1 of 1
53881
L1



SCALE IN MILES

LEGEND

- STRIKE AND DIP OF BEDS
- QUARTZITE
- ▨ PLATY ZONE
- ◻ BOULDERS, LOOSE BLOCKS
- SOIL
- ◻ ALLUVIUM, LOOSE BLOCKS
- GEOLOGICAL BOUNDARY, DEFINITE; QUARTZITE OUTCROP
- GEOLOGICAL BOUNDARY, INDEFINITE; SOIL AND LOOSE BLOCKS
- TRAVERSE POINT
- DIAMOND DRILL HOLE INCLINED
- P650/64 SAMPLE
- 55° STRIKE AND DIP OF SHEARED ZONE
- 70° STRIKE AND DIP OF JOINTING

SCALE IN FEET

S.A. DEPT. OF MINES

MIDDLE RIVER DAM, KANGAROO ISLAND
QUARRY SITE NO. 1
 GEOLOGICAL SKETCH PLAN AND SECTIONS

Approved

Passed

Scale: 20 ft to 1 inch

Drn. S.R.
 Tcd. S.R.
 Ckd. D.H.S.
 Exd.

64-970
 L1

Date 7 Oct '64

Associated Drawing No. No. Amendment Exd. Date

Req. No. S.R. 29-1-65
 D.M.
 Compiled from